

THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED BY THE OWNER OR ITS REPRESENTATIVE. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

NOTICE: CONSTRUCTION SITE SAFETY IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. NEITHER THE OWNER NOR THE ENGINEER SHALL BE EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF THE WORK OF PERSONS ENGAGED IN THE WORK OF ANY NEARBY STRUCTURES, OR OF ANY OTHER PERSONS.

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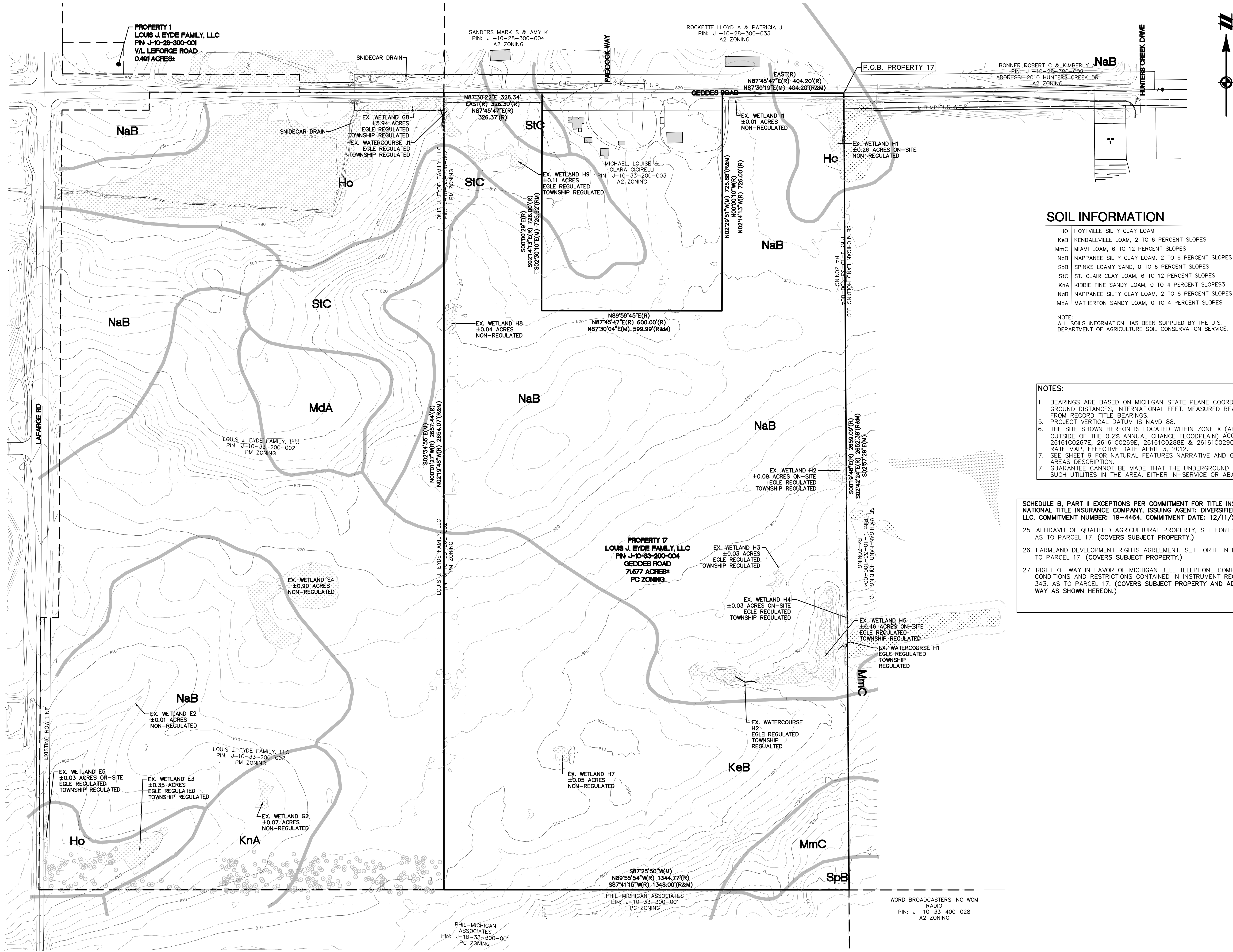
SECTION 33
TOWN 2 SOUTH, RANGE 7 EAST
SUPERIOR TOWNSHIP
WASHTENAW COUNTY, MICHIGAN

EYDE COMPANY
THE MEADOWS AT HAWTHORNE MILL
FINAL SITE PLANS - PHASE 1
OVERALL EXISTING CONDITIONS

DATE
OCT. 12, 2023

Table with 2 columns: REVISIONS, and empty rows for revision notes.

REVISIONS
0 75 150
SCALE: 1" = 150 FEET
DRAWN BY: KS
CHECKED BY: AK
P.M.: J. KIME
JOB #: 19004443
FILE CODE: -
SHEET NO. 2



SOIL INFORMATION

- HO HOYTVILLE SILTY CLAY LOAM
- KeB KENDALLVILLE LOAM, 2 TO 6 PERCENT SLOPES
- MmC MIAMI LOAM, 6 TO 12 PERCENT SLOPES
- NaB NAPPANEE SILTY CLAY LOAM, 2 TO 6 PERCENT SLOPES
- SpB SPINKS LOAMY SAND, 0 TO 6 PERCENT SLOPES
- StC ST. CLAIR CLAY LOAM, 6 TO 12 PERCENT SLOPES
- KnA KIBBIE FINE SANDY LOAM, 0 TO 4 PERCENT SLOPES
- NaB NAPPANEE SILTY CLAY LOAM, 2 TO 6 PERCENT SLOPES
- MdA MATHERTON SANDY LOAM, 0 TO 4 PERCENT SLOPES

NOTE: ALL SOILS INFORMATION HAS BEEN SUPPLIED BY THE U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE.

NOTES:

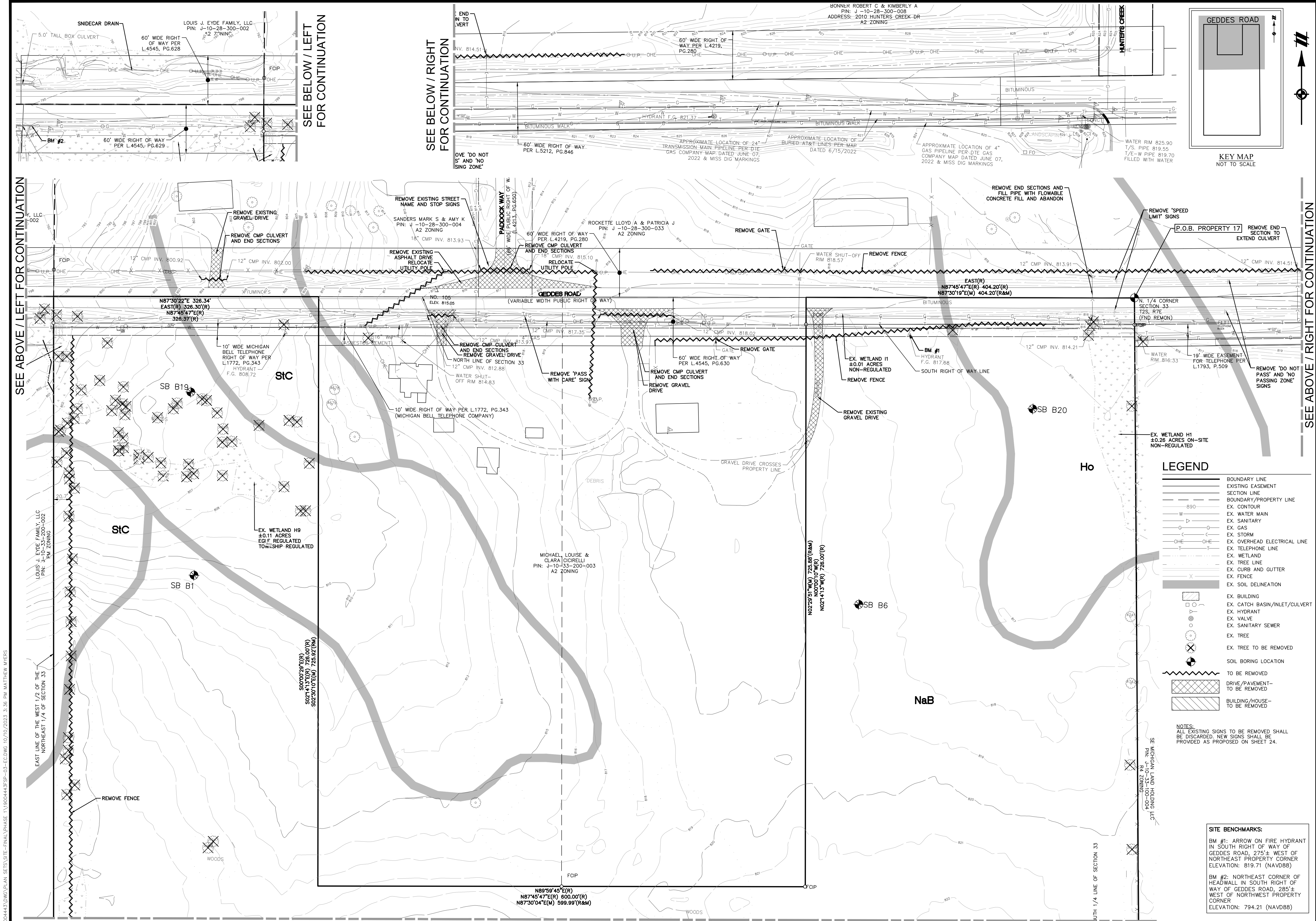
- BEARINGS ARE BASED ON MICHIGAN STATE PLANE COORDINATES (NAD83), SOUTH ZONE, GROUND DISTANCES, INTERNATIONAL FEET. MEASURED BEARINGS AS SHOWN DIFFER FROM RECORD TITLE BEARINGS.
- PROJECT VERTICAL DATUM IS NAVD 88.
- THE SITE SHOWN HEREON IS LOCATED WITHIN ZONE X (AREAS DETERMINED TO BE OUTSIDE OF THE 0.2% ANNUAL CHANCE FLOODPLAIN) ACCORDING TO MAP NUMBERS 26161C0267E, 26161C0269E, 26161C0288E & 26161C0290E OF THE FLOOD INSURANCE RATE MAP, EFFECTIVE DATE APRIL 3, 2012.
- SEE SHEET 9 FOR NATURAL FEATURES NARRATIVE AND GROUNDWATER RECHARGE AREAS DESCRIPTION.
- GUARANTEE CANNOT BE MADE THAT THE UNDERGROUND UTILITIES SHOWN COMPRISE ALL SUCH UTILITIES IN THE AREA, EITHER IN-SERVICE OR ABANDONED.

SCHEDULE B, PART II EXCEPTIONS PER COMMITMENT FOR TITLE INSURANCE ISSUED BY WFG NATIONAL TITLE INSURANCE COMPANY, ISSUING AGENT: DIVERSIFIED NATIONAL TITLE AGENCY, LLC, COMMITMENT NUMBER: 19-4464, COMMITMENT DATE: 12/11/2019:

- AFFIDAVIT OF QUALIFIED AGRICULTURAL PROPERTY, SET FORTH IN LIBER 4892, PAGE 194, AS TO PARCEL 17. (COVERS SUBJECT PROPERTY.)
- FARMLAND DEVELOPMENT RIGHTS AGREEMENT, SET FORTH IN LIBER 4960, PAGE 907, AS TO PARCEL 17. (COVERS SUBJECT PROPERTY.)
- RIGHT OF WAY IN FAVOR OF MICHIGAN BELL TELEPHONE COMPANY AND THE COVENANTS, CONDITIONS AND RESTRICTIONS CONTAINED IN INSTRUMENT RECORDED IN LIBER 1772, PAGE 343, AS TO PARCEL 17. (COVERS SUBJECT PROPERTY AND ADDITIONAL LAND, RIGHT OF WAY AS SHOWN HEREON.)

SITE BENCHMARKS:

BM #1: ARROW ON FIRE HYDRANT IN SOUTH RIGHT OF WAY OF GEDDES ROAD, 275'± WEST OF NORTHEAST PROPERTY CORNER ELEVATION: 819.71 (NAVD88)
BM #2: NORTHEAST CORNER OF HEADWALL IN SOUTH RIGHT OF WAY OF GEDDES ROAD, 285'± WEST OF NORTHWEST PROPERTY CORNER ELEVATION: 794.21 (NAVD88)



SEE ABOVE / LEFT FOR CONTINUATION

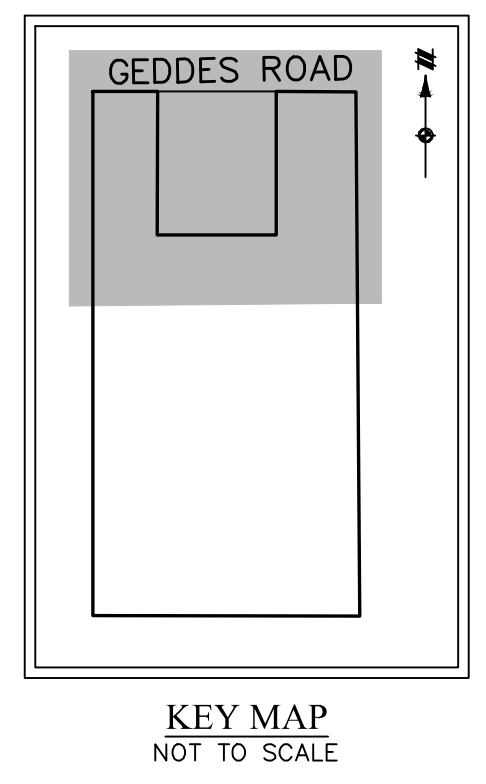
K:\19004443\DWG\PLAN SET\SITE-FINAL\PHASE 1\19004443\3P-03-EC.DWG 10/10/2023 3:36 PM MATTHEW MYERS

SEE BELOW / LEFT FOR CONTINUATION

SEE BELOW / RIGHT FOR CONTINUATION

SEE ABOVE / RIGHT FOR CONTINUATION

SEE SHEET 4 FOR CONTINUATION



811
Know what's below.
Call before you dig.
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LEGEND

- BOUNDARY LINE
- EXISTING EASEMENT
- SECTION LINE
- BOUNDARY/PROPERTY LINE
- EX. CONTOUR
- EX. WATER MAIN
- EX. SANITARY
- EX. GAS
- EX. STORM
- EX. OVERHEAD ELECTRICAL LINE
- EX. TELEPHONE LINE
- EX. WETLAND
- EX. TREE LINE
- EX. CURB AND GUTTER
- EX. FENCE
- EX. SOIL DELINEATION
- EX. BUILDING
- EX. CATCH BASIN/INLET/CULVERT
- EX. HYDRANT
- EX. VALVE
- EX. SANITARY SEWER
- EX. TREE
- EX. TREE TO BE REMOVED
- SOIL BORING LOCATION
- TO BE REMOVED
- DRIVE/PAVEMENT-- TO BE REMOVED
- BUILDING/HOUSE-- TO BE REMOVED

NOTES:
ALL EXISTING SIGNS TO BE REMOVED SHALL BE DISCARDED. NEW SIGNS SHALL BE PROVIDED AS PROPOSED ON SHEET 24.

SITE BENCHMARKS:

- BM #1: ARROW ON FIRE HYDRANT IN SOUTH RIGHT OF WAY OF GEDDES ROAD, 275'± WEST OF NORTHEAST PROPERTY CORNER ELEVATION: 819.71 (NAVD88)
- BM #2: NORTHEAST CORNER OF HEADWALL IN SOUTH RIGHT OF WAY OF GEDDES ROAD, 285'± WEST OF NORTHWEST PROPERTY CORNER ELEVATION: 794.21 (NAVD88)

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SECTION 33
TOWN 2 SOUTH, RANGE 7 EAST
SUPERIOR TOWNSHIP
WASHTENAW COUNTY, MICHIGAN

EYE COMPANY
THE MEADOWS AT HAWTHORNE MILL
FINAL SITE PLANS - PHASE 1
EXISTING CONDITIONS & DEMOLITION PLAN

DATE
OCT. 12, 2023

REVISIONS	
0	SCALE: 1" = 50 FEET
1	DRAWN BY: KS
2	CHECKED BY: AK
3	P.M.: J. KIME
4	JOB #: 19004443
5	FILE CODE: -
6	SHEET NO. 3

CAD FILE: 19004443\3P-03-EC.DWG

TAG #	COMMON NAME	SCIENTIFIC NAME	DBH (INCHES)	CONDITION	LANDMARK	INVASIVE	EXEMPT	REMOVE?	TAG #	COMMON NAME	SCIENTIFIC NAME	DBH (INCHES)	CONDITION	LANDMARK	INVASIVE	EXEMPT	REMOVE?	TAG #	COMMON NAME	SCIENTIFIC NAME	DBH (INCHES)	CONDITION	LANDMARK	INVASIVE	EXEMPT	REMOVE?	TAG #	COMMON NAME	SCIENTIFIC NAME	DBH (INCHES)	CONDITION	LANDMARK	INVASIVE	EXEMPT	REMOVE?	TAG #	COMMON NAME	SCIENTIFIC NAME	DBH (INCHES)	CONDITION	LANDMARK	INVASIVE	EXEMPT	REMOVE?	
260	Red Oak	Quercus rubra	12.5	Good				Y	388	Black Walnut	Juglans nigra	17	Good				Y	1712	Box Elder Maple	Acer negundo	8	Good		Y	Y	Y	4637	Box Elder Maple	Acer negundo	10	Good		Y	Y	Y	4796	Apple	Malus spp.	10	Good		Y	Y	Y	
261	American Basswood	Tilia americana	16.5	Good				Y	389	Black Walnut	Juglans nigra	19	Good	Y				1713	Box Elder Maple	Acer negundo	8	Fair		Y	Y	Y	4638	Box Elder Maple	Acer negundo	9	Poor		Y	Y	Y	4797	Eastern Cottonwood	Populus deltoides	32	Good	Y	Y	Y	Y	
262	Red Oak	Quercus rubra	22	Good	Y			Y	390	Red Oak	Quercus rubra	13	Good					1714	Box Elder Maple	Acer negundo	8	Fair		Y	Y	Y	4639	Box Elder Maple	Acer negundo	12	Good		Y	Y	Y	4798	Box Elder Maple	Acer negundo	17	Good		Y	Y	Y	Y
263	Bitternut Hickory	Carya cordiformis	10	Good				Y	391	Bitternut Hickory	Carya cordiformis	8	Good					1715	Box Elder Maple	Acer negundo	9	Good		Y	Y	Y	4640	Box Elder Maple	Acer negundo	10	Good		Y	Y	Y	4799	Box Elder Maple	Acer negundo	13	Good		Y	Y	Y	Y
264	Red Oak	Quercus rubra	13	Good				Y	392	Red Oak	Quercus rubra	10	Good					1716	Box Elder Maple	Acer negundo	9	Good		Y	Y	Y	4641	Box Elder Maple	Acer negundo	10	Good		Y	Y	Y	4800	Box Elder Maple	Acer negundo	9.5	Fair		Y	Y	Y	Y
265	American Basswood	Tilia americana	13	Good				Y	393	Bitternut Hickory	Carya cordiformis	8	Good					1717	Eastern Cottonwood	Populus deltoides	13.5	Good		Y	Y	Y	4642	Box Elder Maple	Acer negundo	11	Good		Y	Y	Y	4801	American Elm	Ulmus americana	9.5	Good		Y	Y	Y	Y
266	Bitternut Hickory	Carya cordiformis	13	Good				Y	394	Siberian Elm	Ulmus pumila	9	Good					1718	Eastern Cottonwood	Populus deltoides	24	Good	Y	Y	Y	4643	Box Elder Maple	Acer negundo	10.5	Good		Y	Y	Y	4802	American Elm	Ulmus americana	10	Good		Y	Y	Y	Y	
267	Red Oak	Quercus rubra	22.5	Good	Y			Y	395	Black Walnut	Juglans nigra	11.5	Good					1719	Eastern Cottonwood	Populus deltoides	17	Good		Y	Y	Y	4644	Green Ash	Fraxinus pensylvanica	8	Good		Y	Y	Y	4803	American Elm	Ulmus americana	8	Good		Y	Y	Y	Y
268	Bitternut Hickory	Carya cordiformis	9	Good				Y	396	Eastern Cottonwood	Populus deltoides	21	Good					1720	Red Oak	Quercus rubra	8.5	Poor				Y	4645	Box Elder Maple	Acer negundo	8	Good		Y	Y	Y	4804	American Elm	Ulmus americana	8	Poor		Y	Y	Y	Y
269	Bitternut Hickory	Carya cordiformis	10.5	Good				Y	397	Siberian Elm	Ulmus pumila	8	Good					1721	Black Willow	Salix nigra	9	Good					4646	Box Elder Maple	Acer negundo	8.5	Good		Y	Y	Y	4805	Box Elder Maple	Acer negundo	8	Good		Y	Y	Y	Y
270	Bitternut Hickory	Carya cordiformis	8.5	Good				Y	398	Box Elder Maple	Acer negundo	15	Good					1722	American Elm	Ulmus americana	8	Good					4647	Box Elder Maple	Acer negundo	20	Good		Y	Y	Y	4806	Box Elder Maple	Acer negundo	13	Good		Y	Y	Y	Y
271	Bitternut Hickory	Carya cordiformis	11	Good				Y	399	Common Buckthorn	Rhamnus cathartica	10	Good					1723	Red Cedar	Juniperus virginiana	5	Good		Y	Y	Y	4648	Box Elder Maple	Acer negundo	14	Poor		Y	Y	Y	4807	Box Elder Maple	Acer negundo	9.5	Good		Y	Y	Y	Y
272	Red Oak	Quercus rubra	14	Good				Y	400	Siberian Elm	Ulmus pumila	11	Good					1724	Apple	Malus spp.	9	Good					4649	Box Elder Maple	Acer negundo	10.5	Good		Y	Y	Y	4808	Box Elder Maple	Acer negundo	12.5	Good		Y	Y	Y	Y
273	Box Elder Maple	Acer negundo	10	Good		Y		Y	401	Eastern Cottonwood	Populus deltoides	22	Good					1725	Box Elder Maple	Acer negundo	8	Good		Y	Y	Y	4650	Box Elder Maple	Acer negundo	9	Good		Y	Y	Y	4809	Box Elder Maple	Acer negundo	10	Good		Y	Y	Y	Y
274	American Basswood	Tilia americana	9	Good				Y	402	Eastern Cottonwood	Populus deltoides	21	Poor					1726	Eastern Cottonwood	Populus deltoides	18.5	Good		Y	Y	Y	4651	Box Elder Maple	Acer negundo	9.5	Good		Y	Y	Y	4810	Box Elder Maple	Acer negundo	9	Good		Y	Y	Y	Y
275	Black Walnut	Juglans nigra	16	Good				Y	403	Box Elder Maple	Acer negundo	15	Good					1727	Box Elder Maple	Acer negundo	10.5	Good		Y	Y	Y	4652	White Mulberry	Morus alba	20	Good		Y	Y	Y	4811	Box Elder Maple	Acer negundo	11	Good		Y	Y	Y	Y
276	Black Walnut	Juglans nigra	19	Good	Y			Y	404	Box Elder Maple	Acer negundo	10.5	Fair					1728	Box Elder Maple	Acer negundo	11	Good		Y	Y	Y	4653	Box Elder Maple	Acer negundo	19	Good		Y	Y	Y	4812	Box Elder Maple	Acer negundo	15	Good		Y	Y	Y	Y
277	Bitternut Hickory	Carya cordiformis	8	Good				Y	405	Red Oak	Quercus rubra	9	Good					1729	Black Willow	Salix nigra	37	Poor	Y			Y	4654	Box Elder Maple	Acer negundo	13	Good		Y	Y	Y	4813	Box Elder Maple	Acer negundo	9.5	Good		Y	Y	Y	Y
278	American Basswood	Tilia americana	17.5	Good				Y	406	Red Oak	Quercus rubra	8	Good					1730	American Elm	Ulmus americana	11.5	Fair					4655	Box Elder Maple	Acer negundo	14	Good		Y	Y	Y	4814	Box Elder Maple	Acer negundo	11	Good		Y	Y	Y	Y
279	Bitternut Hickory	Carya cordiformis	10	Good				Y	407	Red Oak	Quercus rubra	21	Good	Y				1731	American Elm	Ulmus americana	8	Fair					4656	Black Cherry	Prunus serotina	10	Good		Y	Y	Y	4815	Box Elder Maple	Acer negundo	8	Good		Y	Y	Y	Y
280	Bitternut Hickory	Carya cordiformis	11	Good				Y	408	Eastern Cottonwood	Populus deltoides	17.5	Good					1732	Green Ash	Fraxinus pensylvanica	11.5	Fair					4657	Box Elder Maple	Acer negundo	10.5	Good		Y	Y	Y	4816	Box Elder Maple	Acer negundo	12	Good		Y	Y	Y	Y
281	Red Oak	Quercus rubra	12	Good				Y	409	Siberian Elm	Ulmus pumila	14	Good					1733	White Mulberry	Morus alba	10.5	Good					4658	White Mulberry	Morus alba	8	Good		Y	Y	Y	4817	Black Cherry	Prunus serotina	12	Good		Y	Y	Y	Y
282	Black Walnut	Juglans nigra	10	Good				Y	410	Eastern Cottonwood	Populus deltoides	22	Good	Y	Y	Y		1734	Box Elder Maple	Acer negundo	23	Good		Y	Y	Y	4659	Box Elder Maple	Acer negundo	10.5	Good		Y	Y	Y	4818	Black Cherry	Prunus serotina	12.5	Good		Y	Y	Y	Y
283	Bitternut Hickory	Carya cordiformis	14	Good				Y	411	Box Elder Maple	Acer negundo	22	Good					1735	Black Willow	Salix nigra	18	Good					4660	Box Elder Maple	Acer negundo	9	Good		Y	Y	Y	4819	American Elm	Ulmus americana	8	Good		Y	Y	Y	Y
284	American Basswood	Tilia americana	12	Good				Y	412	Siberian Elm	Ulmus pumila	10	Good					1736	Black Cherry	Prunus serotina	8.5	Good					4661	Box Elder Maple	Acer negundo	9	Good		Y	Y	Y	4820	Black Cherry	Prunus serotina	14	Good		Y	Y	Y	Y
285	Bitternut Hickory	Carya cordiformis	21	Good	Y			Y	413	Common Buckthorn	Rhamnus cathartica	8	Good					1737	Black Cherry	Prunus serotina	10	Good					4662	Box Elder Maple	Acer negundo	9	Good		Y	Y	Y	4821	Box Elder Maple	Acer negundo	17	Good		Y	Y	Y	Y
286	Bitternut Hickory	Carya cordiformis	10.5	Good				Y	414	Green Ash	Fraxinus pensylvanica	8.5	Poor					1738	Black Willow	Salix nigra	12	Good				Y	4663	Common Buckthorn	Rhamnus cathartica	8	Good		Y	Y	Y	4822	Black Cherry	Prunus serotina	10	Good		Y	Y	Y	Y
287	Black Cherry	Prunus serotina	10.5	Good				Y	415	Box Elder Maple	Acer negundo	13	Good					1739	Green Ash	Fraxinus pensylvanica	9	Good					4664	Box Elder Maple	Acer negundo	20	Good		Y	Y	Y	4823	American Elm	Ulmus americana	10.5	Good		Y	Y	Y	Y
288	Bitternut Hickory	Carya cordiformis	12.5	Good				Y	416	Red Cedar	Juniperus virginiana	6.5	Good					1740	Common Buckthorn	Rhamnus cathartica	10.5	Good		Y	Y	Y	4665	Box Elder Maple	Acer negundo	8	Good		Y	Y	Y	4824	Box Elder Maple	Acer negundo	10	Fair		Y	Y	Y	Y
289	Bitternut Hickory	Carya cordiformis	8	Good				Y	417	Green Ash	Fraxinus pensylvanica	12	Fair					1741	Black Walnut	Juglans nigra	9	Good					4666	Box Elder Maple	Acer negundo	9.5	Good		Y	Y	Y	4825	Box Elder Maple	Acer negundo	17	Good		Y	Y	Y	Y
290	American Elm	Ulmus americana	8.5	Good				Y	418	Apple	Malus spp.	8	Good					1742	Hawthorn	Crataegus spp.	13	Good					4667	Box Elder Maple	Acer negundo	9	Fair		Y	Y	Y	4826	Black Walnut	Juglans nigra	12	Good		Y	Y	Y	Y
291	Siberian Elm	Ulmus pumila	11	Good				Y	419	Red Cedar	Juniperus virginiana	8	Good					1743	White Mulberry	Morus alba	33.5	Good	Y				4668	Box Elder Maple	Acer negundo	10.5	Good		Y	Y	Y	4827	Box Elder Maple	Acer negundo	15	Fair		Y	Y	Y	Y
292	American Basswood	Tilia americana	21	Good	Y			Y	420	Red Cedar	Juniperus virginiana	5.5	Good					1744	Black Cherry	Prunus serotina	8	Fair					4669	American Elm	Ulmus americana	10	Good		Y	Y	Y	4828	Box Elder Maple	Acer negundo	18	Good		Y	Y	Y	Y
293	Bitternut Hickory	Carya cordiformis	9	Good				Y	421	Green Ash	Fraxinus pensylvanica	9	Good					1745	Bitternut Hickory	Carya cordiformis	10	Good					4670	Box Elder Maple	Acer negundo	11	Good		Y	Y	Y	4829	Box Elder Maple	Acer negundo	9	Good		Y	Y	Y	Y
294	Bitternut Hickory	Carya cordiformis	8	Good				Y	422	Green Ash																																			

K:\19004443\DWG\PLAN SET\SITE-FINAL\PHASE 1\19004443-SP-04-TL.DWG 10/10/2023 3:37 PM MATHEW MYERS

Table with 8 columns: TAG #, COMMON NAME, SCIENTIFIC NAME, DBH (INCHES), CONDITION, LANDMARK, INVASIVE, EXEMPT, REMOVE?. Contains 963 rows of tree data.

Table with 8 columns: TAG #, COMMON NAME, SCIENTIFIC NAME, DBH (INCHES), CONDITION, LANDMARK, INVASIVE, EXEMPT, REMOVE?. Contains 963 rows of tree data.

Table with 8 columns: TAG #, COMMON NAME, SCIENTIFIC NAME, DBH (INCHES), CONDITION, LANDMARK, INVASIVE, EXEMPT, REMOVE?. Contains 700 rows of tree data.

OFFSITE TREE REMOVAL/REPLACEMENT CALCULATION table with 4 columns: Category, Count, Replacement Count, and Requirement. Includes totals for regulated trees and landmark trees.

Vertical sidebar containing: 811 logo, 'Know what's below. Call before you dig.', disclaimer text, ATWELL logo and contact info (866.850.4200), SECTION 33, EYE COMPANY, THE MEADOWS AT HAWTHORNE MILL FINAL SITE PLANS - PHASE 1 OFFSITE TREE LIST, DATE OCT. 12, 2023, REVISIONS table, DRAWN BY: KS, CHECKED BY: AK, P.M.: J. KIME, JOB #: 19004443, FILE CODE: -, SHEET NO. 8.

CAD FILE: 19004443-SP-04-TL.DWG

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SUPERIOR TOWNSHIP
WASHTENAW COUNTY, MICHIGAN

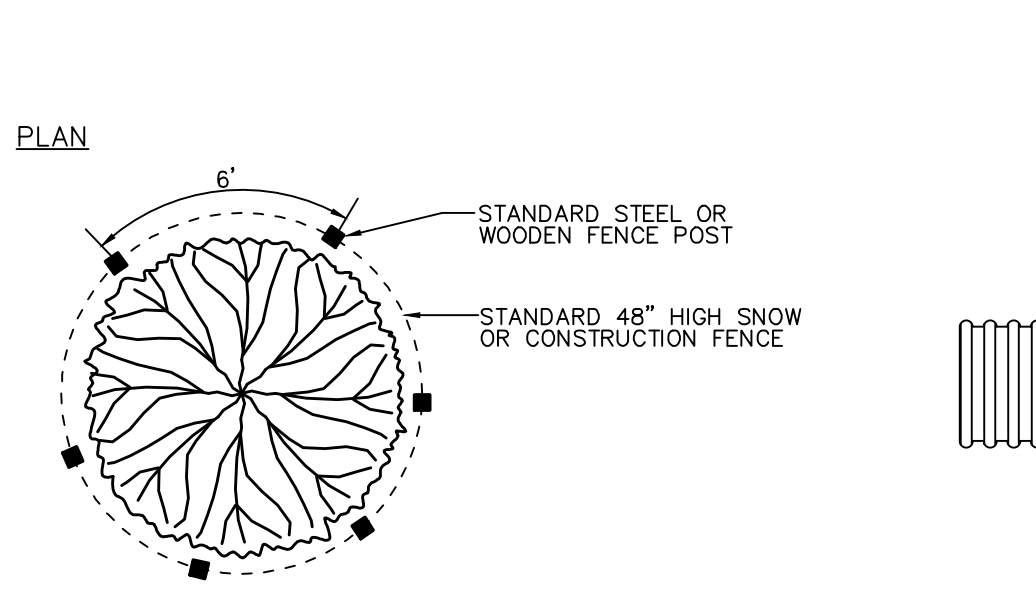
EYE COMPANY
THE MEADOWS AT HAWTHORNE MILL
FINAL SITE PLANS - PHASE 1
SOIL EROSION & SEDIMENTATION CONTROL DETAILS

DATE: OCT. 12, 2023

REVISIONS

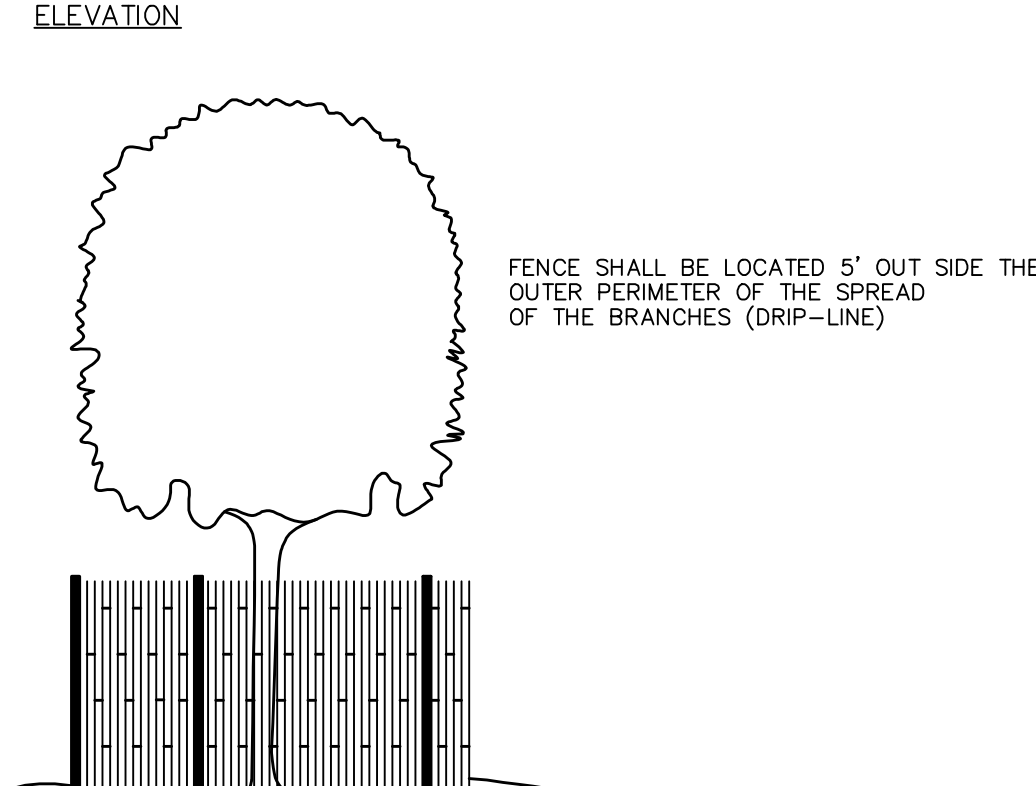
NA NA NA
N/A

DRAWN BY: KS
CHECKED BY: AK
P.M.: J. KIME
JOB #: 19004443
FILE CODE: -
SHEET NO. 14

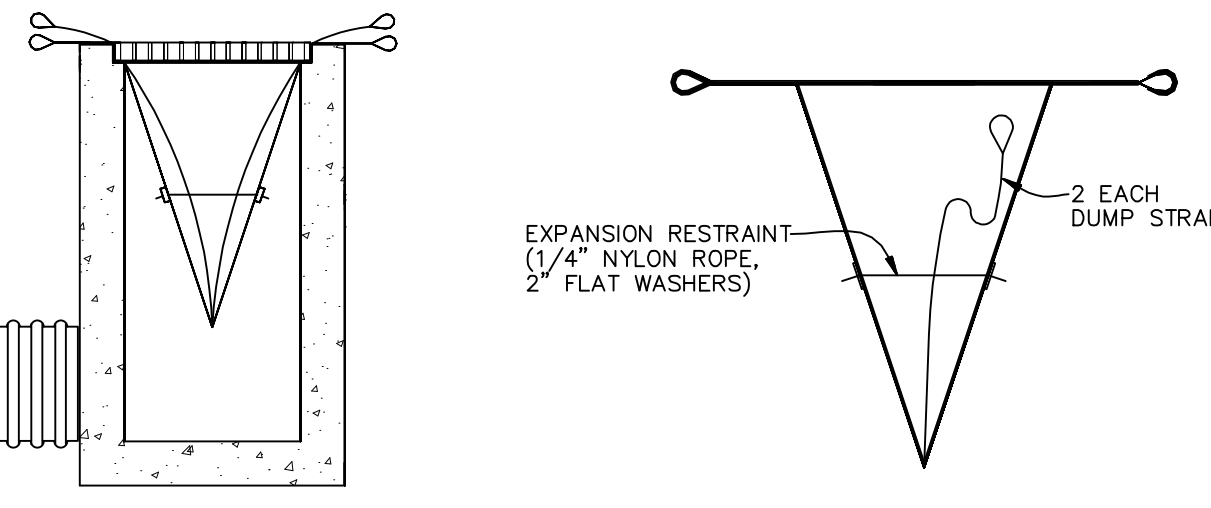


TREE PROTECTION NOTES:

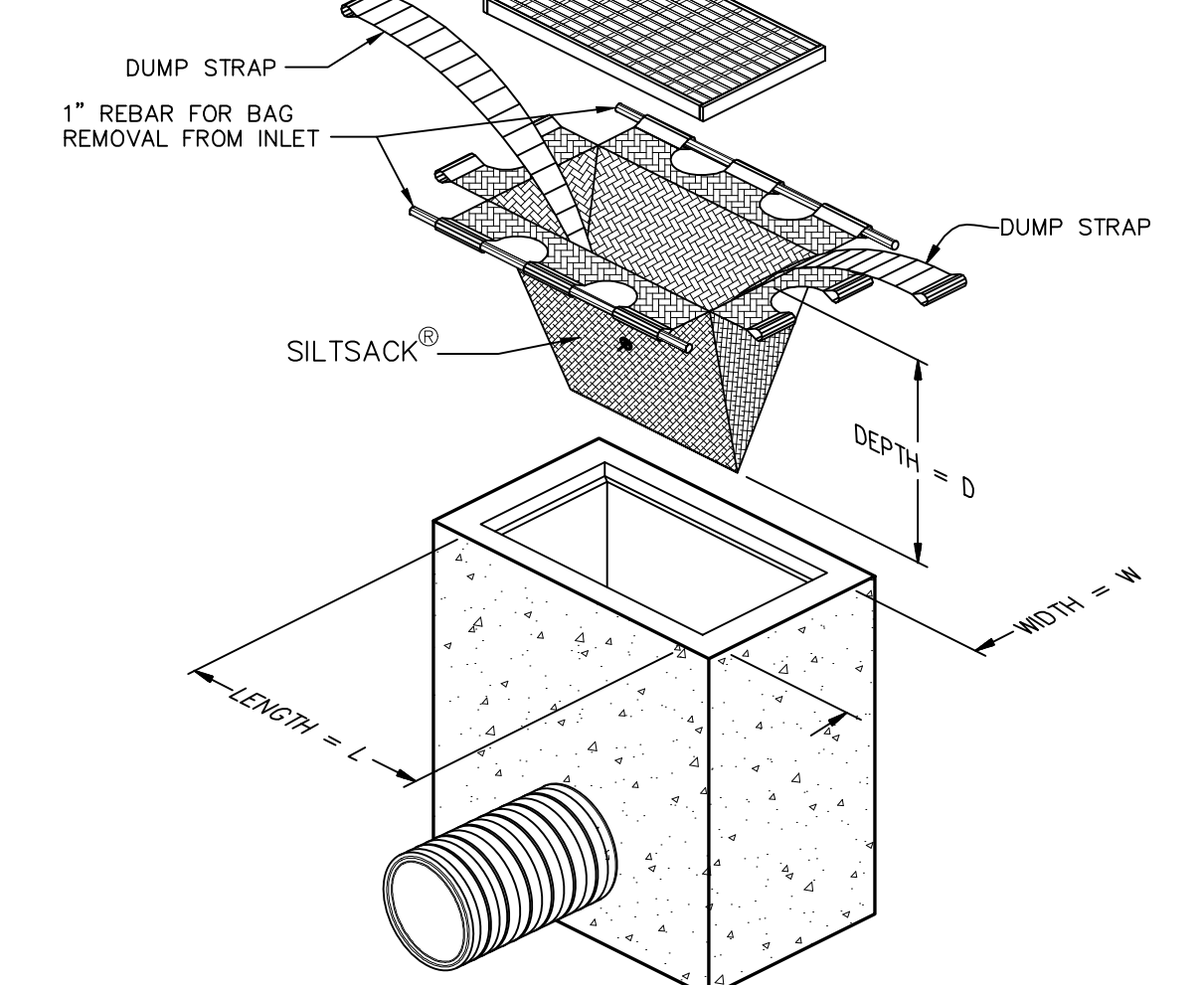
- ALL TREES TO BE REMOVED WILL BE IDENTIFIED BY RED FLAGGING.
- TREE PROTECTION FENCING IS TO BE ERRECTED PRIOR TO ANY EARTHWORK OR CONSTRUCTION AND IS TO REMAIN IN PLACE UNTIL CONSTRUCTION IS COMPLETE.
- ALL DEBRIS, FILL, EQUIPMENT OR MATERIAL IS TO BE KEPT CLEAR OF AREA WITHIN PROTECTIVE FENCE. NO CLEANING OF EQUIPMENT OR MATERIAL OR STORAGE OR DISPOSAL OF ANY MATERIAL WITHIN THE Drip LINE OF ANY TREES TO BE SAVED.



TREE PROTECTION FENCE DETAIL
NO SCALE



INSTALLATION DETAIL

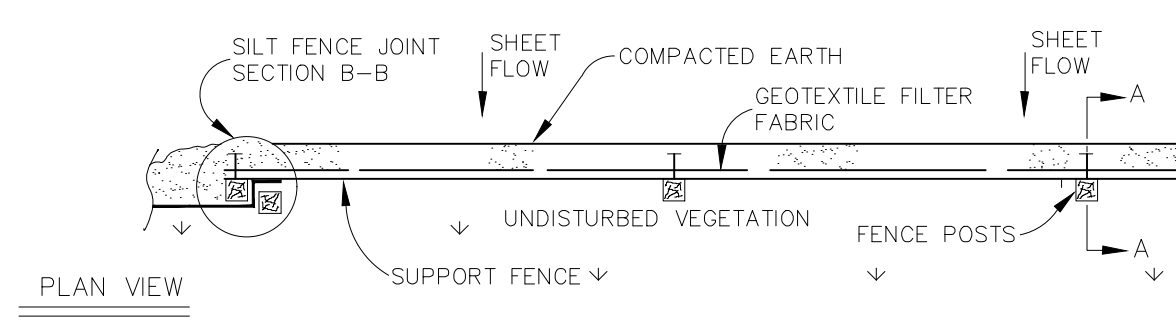


SILTSACK DETAIL
NO SCALE

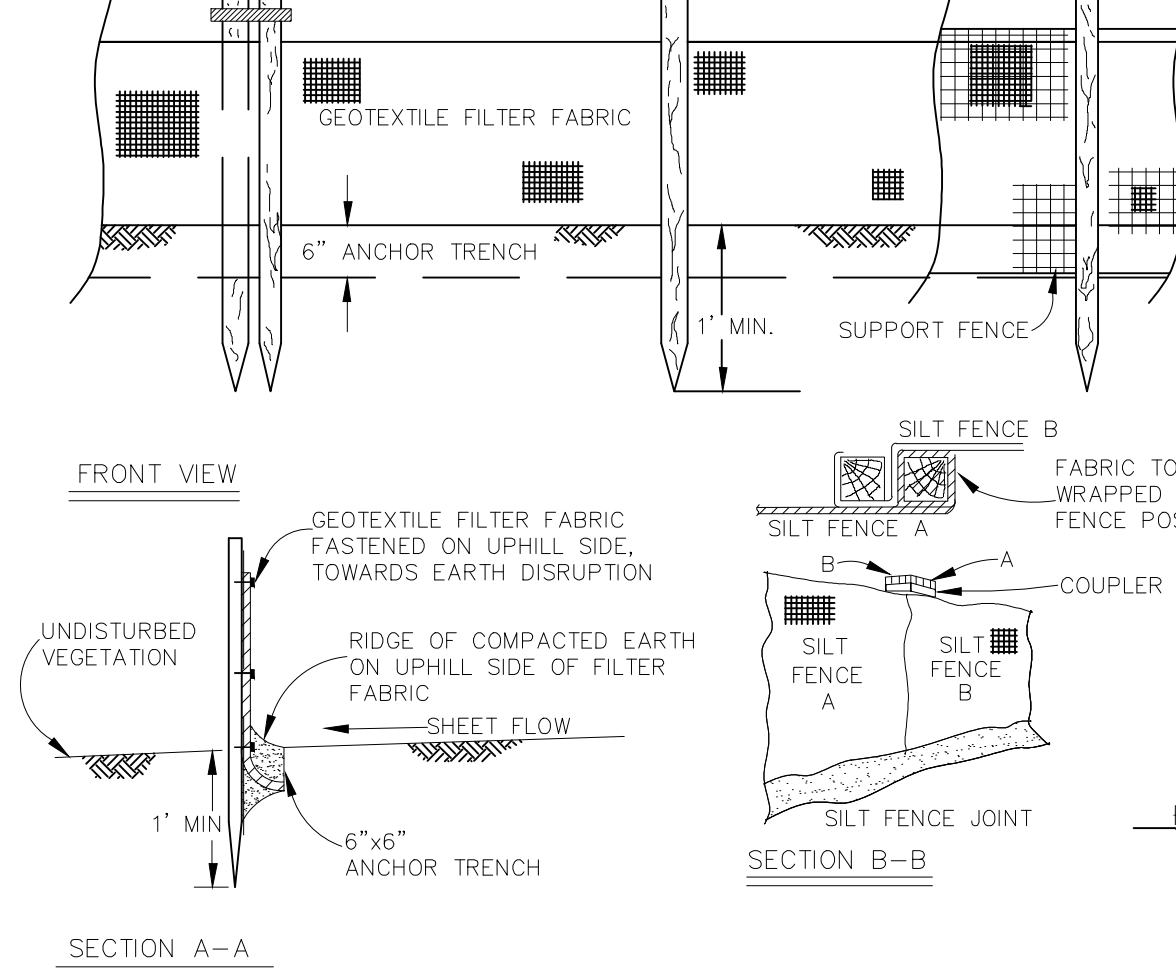
SITE INFORMATION

SITE LOCATION: NORTHEAST 1/4 OF SECTION 33, SUPERIOR TOWNSHIP, MICHIGAN.

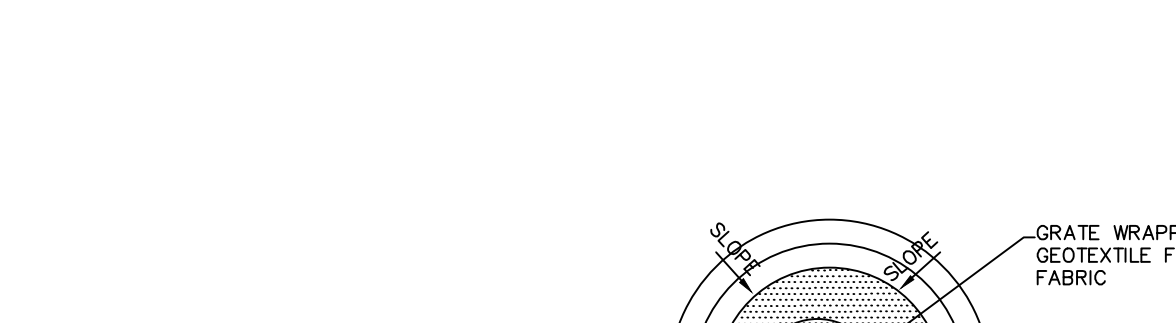
- ULTIMATE RECEIVING WATER: HURON RIVER, VIA SUPERIOR DRAIN NO. 1 AND SNIDEAR DRAIN.
- APPROXIMATE AREA OF DISTURBANCE: **74± ACRES**
- THIS PROJECT IS WITHIN 500 FEET OF A WATERBODY OR WATERCOURSE



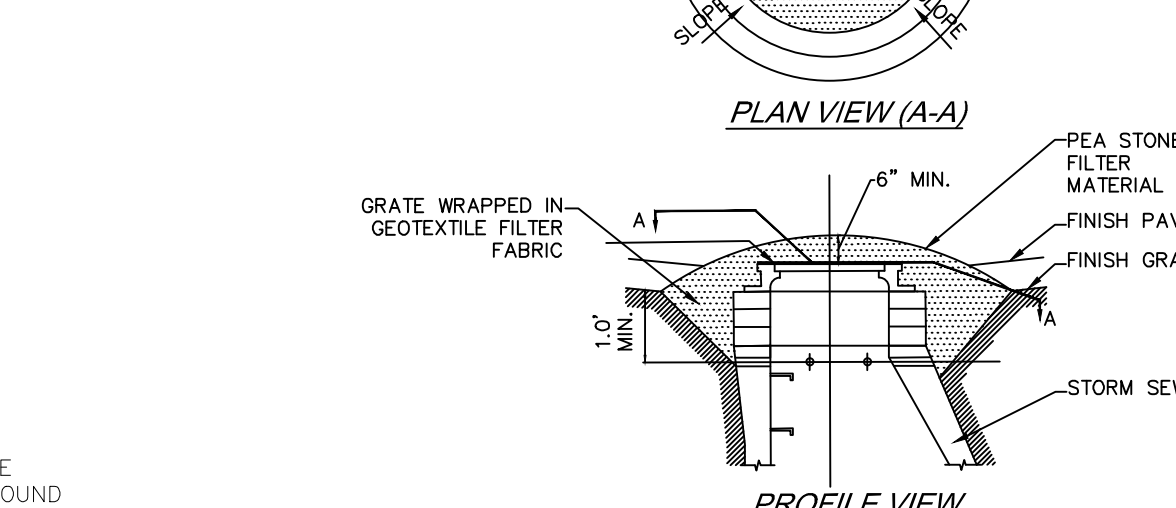
SILT FENCE DETAIL
NO SCALE



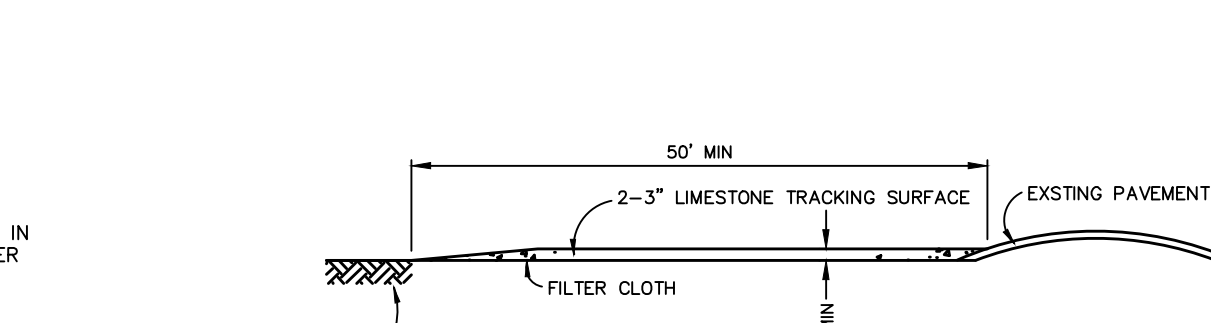
SILT FENCE DETAIL
NO SCALE



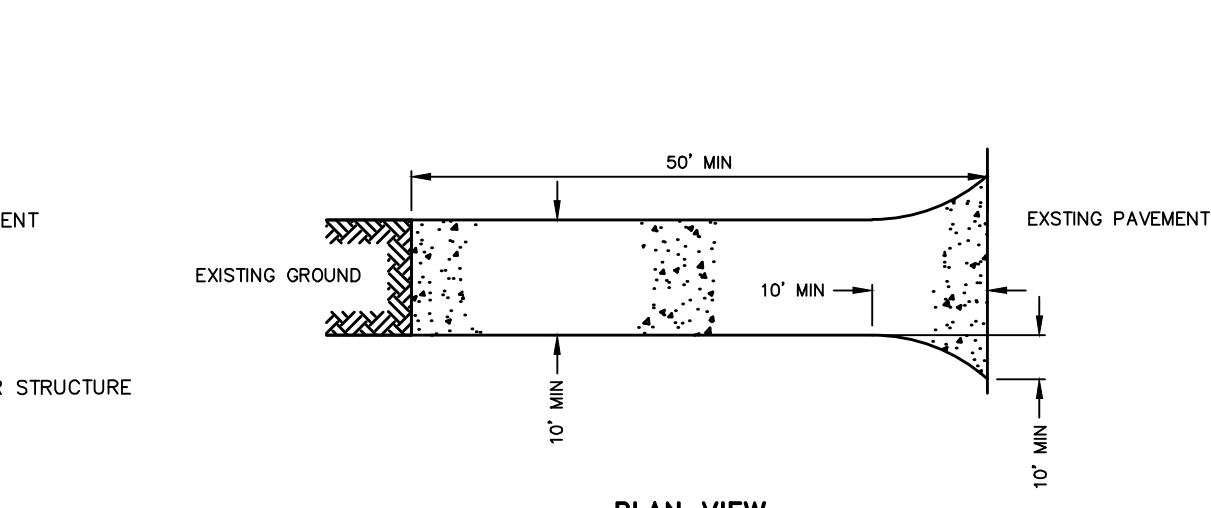
LOW POINT INLET FILTER
NO SCALE



LOW POINT INLET FILTER
NO SCALE



TRACKING SURFACE
NO SCALE



TRACKING SURFACE
NO SCALE

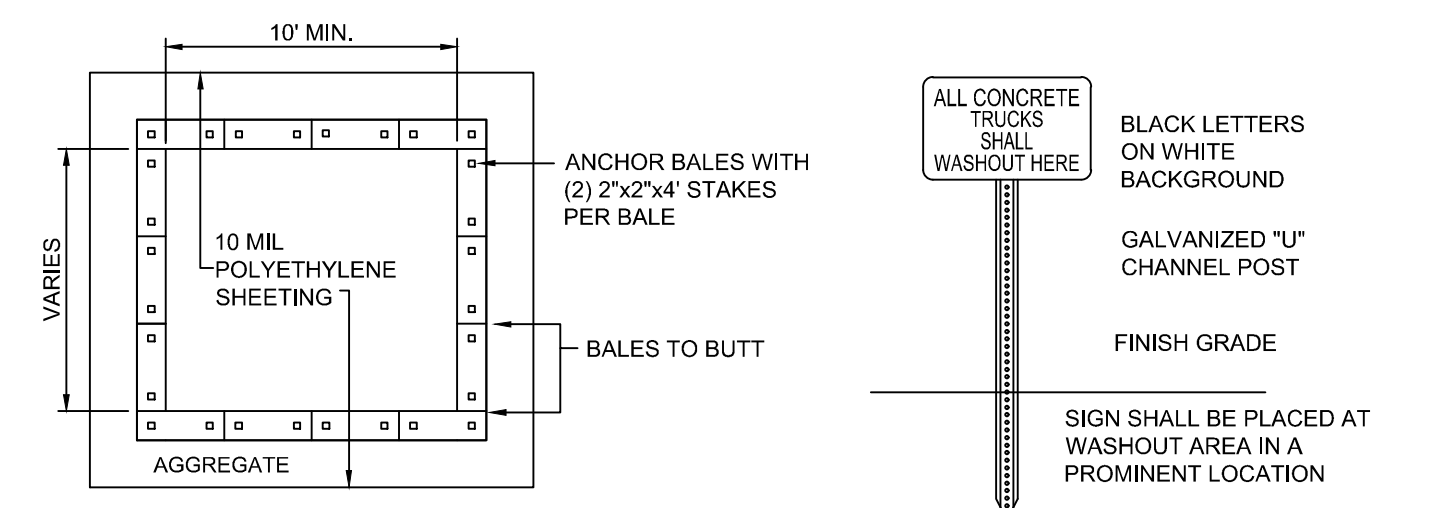
SOIL EROSION AND SEDIMENTATION CONTROL NOTES:

- THE SMALLEST PRACTICAL AREA OF LAND SHALL BE EXPOSED AT ANY ONE TIME DURING DEVELOPMENT.
- WHEN LAND IS EXPOSED DURING DEVELOPMENT, THE EXPOSURE SHALL BE KEPT TO THE SHORTEST PERIOD OF TIME.
- TEMPORARY VEGETATION AND/OR MULCHING SHALL BE USED TO PROTECT CRITICAL AREAS EXPOSED DURING DEVELOPMENT.
- THE PERMANENT FINAL VEGETATION AND STRUCTURES SHALL BE INSTALLED AS SOON AS PRACTICAL IN DEVELOPMENT.
- THE DEVELOPMENT PLAN SHOULD BE FITTED TO THE TOPOGRAPHY AND SOIL SO AS TO CREATE THE LEAST SOIL EROSION POTENTIAL.
- REFER TO WASHTENAW COUNTY STANDARD DETAILS OF THE SESC BMP MEASURES AS THEY CORRESPOND WITH THIS PLAN.
- ALL EROSION AND SEDIMENT CONTROL WORK SHALL CONFORM TO THE STANDARDS AND SPECIFICATIONS OF THE TOWNSHIP AND THE WASHTENAW COUNTY DRAIN COMMISSIONER.
- THE CONTRACTOR SHALL MAKE DAILY INSPECTIONS TO DETERMINE EFFECTIVENESS OF EROSION AND SEDIMENT CONTROL MEASURES, AND ANY NECESSARY REPAIRS SHALL BE PERFORMED WITHOUT DELAY.
- EROSION AND ANY SEDIMENTATION FROM WORK ON THIS SITE SHALL BE CONTAINED ON THE SITE AND NOT ALLOWED TO COLLECT ON ANY OFF-SITE AREAS OR IN WATERWAYS. WATERWAYS INCLUDE NATURAL AND MAN-MADE OPEN DITCHES, STREAMS, STORM DRAINS, LAKES AND PONDS.
- EROSION AND ANY SEDIMENTATION CONTROL MEASURES ARE TO BE PLACED PRIOR TO OR AS THE FIRST STEP IN CONSTRUCTION. SEDIMENT CONTROL PRACTICES WILL BE APPLIED AS A PERIMETER DEFENSE AGAINST ANY TRANSPORTING OF SILT OFF THE SITE.
- PERMANENT SOIL EROSION CONTROL MEASURES FOR ALL SLOPES, CHANNELS, DITCHES, OR ANY DISTURBED LAND AREA SHALL BE COMPLETED WITHIN 5 CALENDAR DAYS AFTER FINAL GRADING OR THE FINAL EARTH CHANGE HAS BEEN COMPLETED. WHEN IT IS NOT POSSIBLE TO PERMANENTLY STABILIZE A DISTURBED AREA AFTER AN EARTH CHANGE HAS BEEN COMPLETED OR WHERE SIGNIFICANT EARTH CHANGE ACTIVITY CEASES, TEMPORARY SOIL EROSION CONTROL MEASURES SHALL BE MAINTAINED UNTIL PERMANENT SOIL EROSION CONTROL MEASURES ARE IMPLEMENTED. ALL PERMANENT SOIL EROSION CONTROL MEASURES WILL BE IMPLEMENTED AND ESTABLISHED BEFORE A CERTIFICATE OF COMPLIANCE IS ISSUED.
- A WATER TRUCK SHALL BE AVAILABLE TO WATER DOWN THE SITE ON A DAILY BASIS FOR DUST CONTROL.
- ALL MUD/DIRT TRACKED ONTO EXISTING CITY/COUNTY ROADS FROM THIS SITE, DUE TO CONSTRUCTION, SHALL BE PROMPTLY REMOVED BY THE CONTRACTOR/BUILDER, AS DIRECTED BY THE TOWNSHIP. MUD MAT DAILY MAINTENANCE WILL INCLUDE THE ADDITION OF EXTRA LAYERS OF STONE AS NECESSARY.
- DURING CONSTRUCTION OF THE STORM SEWER SYSTEM, STRAW BALES, STONE FILTERS OR OTHER APPROVED MEANS, WILL PROTECT THE ENDS OF ALL OPEN PIPES.
- PROMPTLY UPON THE BACKFILLING OF STORM STRUCTURES, INLET FILTERS WILL BE PLACED AROUND THE STRUCTURE PER DETAILS.
- WITHIN FIVE (5) DAYS AFTER COMPLETION OF PAVING, A 16-FOOT STRIP AROUND PAVED AREAS SHALL BE PROTECTED FROM SOIL EROSION BY AN APPROVED METHOD CONSISTENT WITH THE GROWING SEASON.
- ANY REMAINING DENUEED AREA SHALL BE SEEDED AND MULCHED WITHIN 5 DAYS AFTER COMPLETION OF FINAL GRADING. SEED MIX AND APPLICATION RATES SHALL BE PER MDT CLASS A SEED.
- SOIL EROSION AND SEDIMENTATION CONTROL MEASURES SHALL BE MAINTAINED ON A DAILY BASIS TO ENSURE PROPER FUNCTIONING. SEDIMENT DEPOSIT MUST BE REMOVED WHEN ACCUMULATION REACHES 1/3 TO 1/2 OF THE HEIGHT OF THE SILT FENCE AND SHOULD BE REMOVED AFTER EACH STORM EVENT. FABRIC SHALL BE REPLACED PROMPTLY IF IT DECOMPOSES OR BECOMES INEFFECTIVE PRIOR TO THE END OF THE EXPECTED USEABLE LIFE.
- THAT ALL EROSION CONTROL MEASURES ARE INSTALLED IN ACCORDANCE WITH THE FOLLOWING SEQUENCE OF CONSTRUCTION
 - INSTALL SILT FENCE/STRAW BERMS AS SHOWN ON PLANS.
 - STRIP AND STOCKPILE TOPSOIL AND GRADE SITE.
 - INSTALL ON-SITE AND OFF-SITE STORM SEWER SYSTEMS COMPLETE, IMMEDIATELY INSTALL STONE FILTERS ON ALL PIPE INLETS AND CATCH BASINS AND ESTABLISH VEGETATION ON ALL DITCHES, SWALES, AND DISTURBED AREAS.
 - INSTALL ALL PUBLIC UTILITIES (GAS, ELECTRICITY, AND TELEPHONE)
 - INSTALL PAVEMENT COMPLETE REPAIR AND/OR REPLACE STONE FILTERS AS REQUIRED.
 - FINISH GRADE, REDISTRIBUTE TOPSOIL, ESTABLISH VEGETATION AND/OR LANDSCAPE ALL DISTURBED AREAS.
 - CLEAN PAVEMENT, WALKS, CULVERTS, WATERCOURSES, AND STORM SEWER SYSTEMS OF ALL SEDIMENT IN CONJUNCTION WITH THE REMOVAL OF ALL TEMPORARY EROSION CONTROL MEASURES. REESTABLISHED VEGETATION AS NECESSARY.
 - SHOULD DEWATERING BE NECESSARY, DISCHARGE SHALL BE ROUTED THROUGH A SEDIMENT FOREBAY, FILTER BAG OVER A WELL VEGETATED AREA OR OTHER APPROVED FILTERING MECHANISM PRIOR TO BEING DISCHARGED FROM THE SITE. DISCHARGE MUST BE LIMITED TO A NON-EROSIVE VELOCITY.
 - SOIL EROSION WILL BE CONTROLLED DURING AND AFTER CONSTRUCTION TO PROTECT ADJACENT PROPERTIES OR FACILITIES.
 - EROSION CONTROL BLANKET/MATTING SHALL BE INSTALLED ON SLOPES AT OR NEAR MAXIMUM ALLOWABLE GRADE AND AS NEEDED TO EFFECTIVELY ESTABLISH BOTH TEMPORARY AND PERMANENT VEGETATIVE COVER.

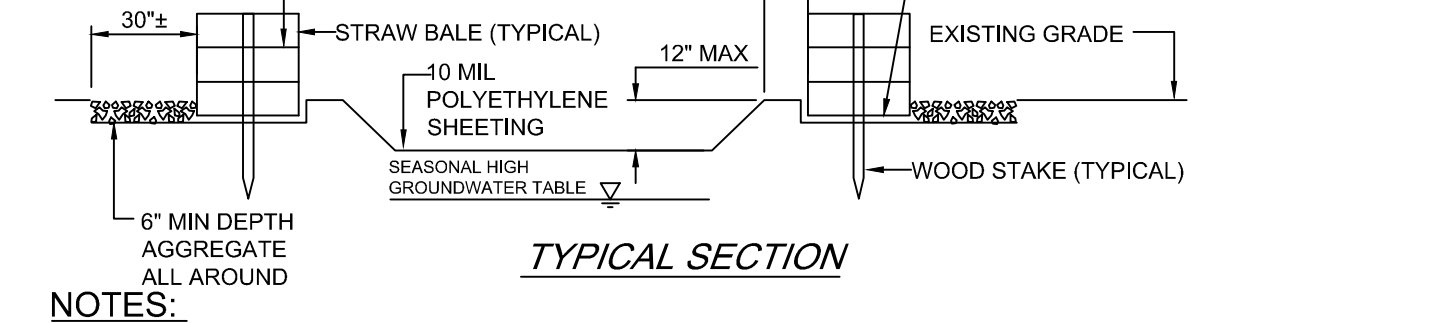
SOIL EROSION CONSTRUCTION SEQUENCE

- NOTIFY SOIL EROSION OFFICE 48 HOURS BEFORE WORK IS TO BEGIN.
- PRIOR TO CONSTRUCTION, INSTALL TEMPORARY STONE ACCESS DRIVES AT ENTRANCE, PERIMETER SILT FENCE, SNOW FENCE, AND EROSION CONTROL MEASURES ON EXISTING STORM INLETS AS DESIGNATED ON THE SESC PLAN.
- INSTALL UNDERGROUND UTILITIES (I.E. SANITARY, STORM, AND WATER MAIN.). INSTALL INLET FILTER PROTECTION ON PROPOSED STORM SEWER STRUCTURES.
- PAVING ACTIVITIES. REMOVE STONE ACCESS DRIVE WHEN COMPLETE.
- INSTALL PUBLIC UTILITIES (ELECTRIC, TELEPHONE, AND CABLE T.V.).
- FINAL GRADING AND INSTALLATION OF LANDSCAPING. ESTABLISH PERMANENT VEGETATION FOR REMAINING DISTURBED AREAS.
- CLEAN OUT STORM SEWER SYSTEM. CLEAN OUT SEDIMENT AND RESTORE SEDIMENT FOREBAY AND DETENTION POND TO DESIGN SPECIFICATIONS.
- CALL SOIL EROSION OFFICE FOR FINAL INSPECTION. REMOVE TEMPORARY SOIL EROSION MEASURES UPON APPROVAL.

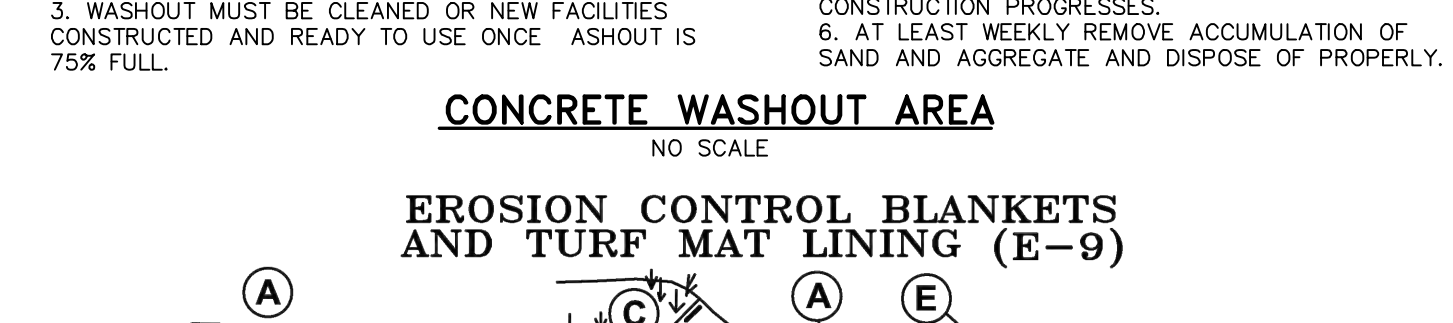
NOTE: NO WORK WITHIN REGULATED WETLANDS UNTIL EGLE PERMIT IS ISSUED.



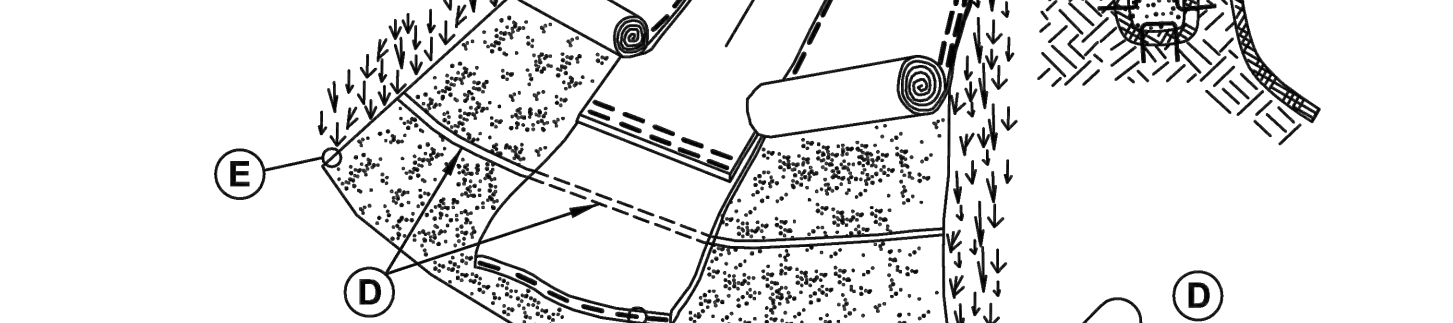
SEDIMENT FOREBAY MARKER DETAIL
NO SCALE



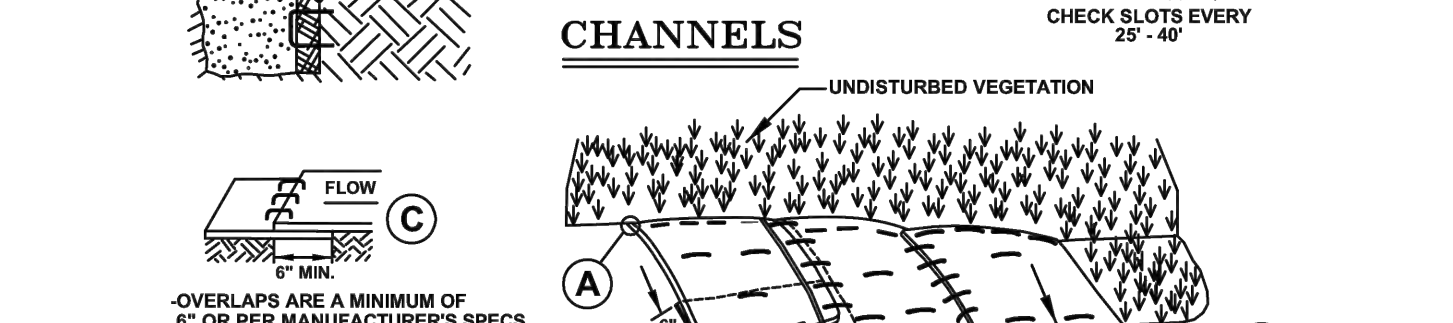
CONCRETE WASHOUT AREA
NO SCALE



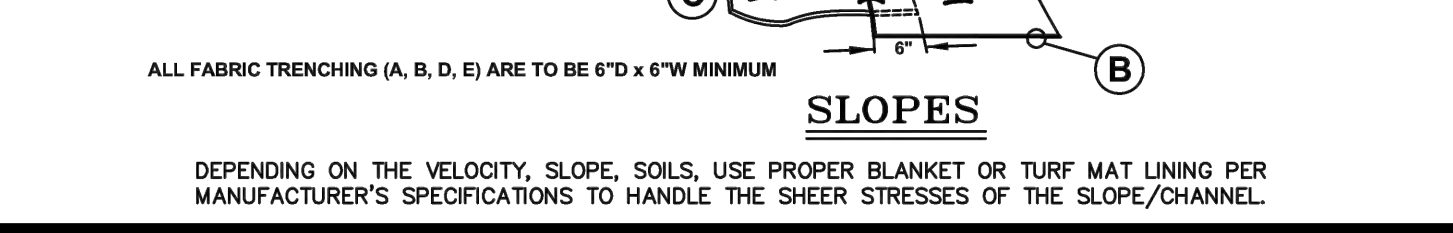
EROSION CONTROL BLANKETS AND TURF MAT LINING (E-9)



RIP-RAP, END SECTION AND BAR SCREEN DETAIL (E-7)



CHANNELS



SLOPES

Maintenance Tasks and Schedule (during construction) (To be performed by developer/contractor)

Components	Storm Sewer System	Catch Basin Stumps	Catch Basin Inlet Castings	Channels	Outflow Control Structures	Rip-Rap	Storm Detention Areas	Wetlands	Emergency Overflow	Schedule
Inspect for sediment accumulation	*	*	*	*	*	*	*	*	*	Weekly
Removal of sediment accumulation	*	*	*	*	*	*	*	*	*	As needed* and prior to turnover
Inspect for floatables and debris	*	*	*	*	*	*	*	*	*	Quarterly
Cleaning of floatables and debris	*	*	*	*	*	*	*	*	*	Quarterly and at turnover
Inspection for erosion	*	*	*	*	*	*	*	*	*	Weekly
Reestablish permanent vegetation on eroded slopes	*	*	*	*	*	*	*	*	*	As needed* and prior to turnover
Replacement of gravel jackets	*	*	*	*	*	*	*	*	*	As needed*
Mowing	*	*	*	*	*	*	*	*	*	0 to 2 times per yr
Inspect structural elements during wet weather and compare as-built plans (by a professional engineer reporting to the Developer)	*	*	*	*	*	*	*	*	*	Annually and at turnover
Make adjustments or replacements as determined by pre-turnover inspection	*	*	*	*	*	*	*	*	*	As needed*

Maintenance Tasks and Schedule (following construction) (To be performed by owner or owner's representative)

Components	Storm Sewer System	Catch Basin Stumps	Catch Basin Inlet Castings	Channels	Outflow Control Structures	Rip-Rap	Storm Detention Areas	Wetlands	Emergency Overflow	Schedule	Annual Cost
Inspect for sediment accumulation	*	*	*	*	*	*	*	*	*	Annually	\$300.00
Removal of sediment accumulation	*	*	*	*	*	*	*	*	*	Every 3-10 yrs as needed	\$1,000.00
Inspect for floatables and debris	*	*	*	*	*	*	*	*	*	Annually	\$500.00
Cleaning of floatables and debris	*	*	*	*	*	*	*	*	*	Annually	\$400.00
Inspection for erosion	*	*	*	*	*	*	*	*	*	Annually	\$500.00
Inspect wetlands and woodlands for invasive species. Remove or treat as needed	*	*	*	*	*	*	*	*	*	Annually (removal or treatment every 3-5 years as needed)	\$800.00
Reestablish permanent vegetation on eroded slopes	*	*	*	*	*	*	*	*	*	As needed	\$500.00
Replacement of gravel jackets	*	*	*	*	*	*	*	*	*	Every 3-5 yrs as needed	\$700.00
Clean streets	*	*	*	*	*	*	*	*	*	Annually	\$400.00
Mowing	*	*	*	*	*	*	*	*	*	0 to 2 times per yr	\$400.00
Inspect structural elements during wet weather and compare as-built plans (by a professional engineer reporting to the condominium association)	*	*	*	*	*	*	*	*	*	Annually	\$400.00
Make adjustments or replacements as determined by wet weather inspection	*	*	*	*	*	*	*	*	*	As needed	\$500.00
Keep records of all inspections and maintenance activities and report to condominium association	*	*	*	*	*	*	*	*	*	Annually	\$200.00
Keep records of all costs for inspections, maintenance and repairs. Report to condominium association	*	*	*	*	*	*	*	*	*	Annually	\$200.00
Condominium association reviews cost effectiveness of the preventative maintenance program and makes necessary adjustments	*	*	*	*	*	*	*	*	*	Annually	\$200.00
Condominium association to have a professional engineer carry out emergency inspections upon identification of severe problems	*	*	*	*	*	*	*	*	*	As needed	\$200.00

The Condominium Association will assess its members to pay for all maintenance activities on an annual basis

NOTE:
-BAR SCREEN DETAILS PER MDT ROAD & BRIDGE STANDARD PLANS, STEEL GRATES FOR END SECTIONS. BARS ARE TO BE CUT OFF AT EDGE OF END SECTION.
-FOR SLOPE AND/OR CHANNEL PROTECTION SEE OTHER BMP'S ON THIS SHEET OR IN THE WRC SOIL EROSION MANUAL.

DEPENDING ON THE VELOCITY, SLOPE, SOILS, USE PROPER BLANKET OR TURF MAT LINING PER MANUFACTURER'S SPECIFICATIONS TO HANDLE THE SHEAR STRESSES OF THE SLOPE/CHANNEL.

ASTILBE AVE. - HONEYSUCKLE DR. - BLUE BELL AVE.

ALIGNMENT LINE TABLE				
LINE #	LENGTH	DIRECTION	START POINT	END POINT
L2	179.52	S2° 29' 55.60"E	(N 283363.71, E 13326001.10)	(N 283184.35E 13326008.92)
L3	45.34	S32° 44' 26.17"E	(N 283069.99, E 13326045.24)	(N 283031.85E 13326069.77)
L4	734.62	S2° 29' 55.60"E	(N 282917.48, E 13326106.09)	(N 282183.56E 13326138.12)
L5	291.44	S2° 29' 55.60"E	(N 283416.00, E 13327199.33)	(N 283124.83E 13327212.04)
L6	33.96	S29° 13' 02.00"W	(N 283002.53, E 13327182.99)	(N 282972.90E 13327166.42)
L7	984.84	S2° 29' 55.60"E	(N 282850.60, E 13327137.38)	(N 281866.69E 13327180.31)
L9	1027.39	N87° 30' 04.40"E	(N 282489.27, E 13326124.78)	(N 282534.06E 13327151.19)

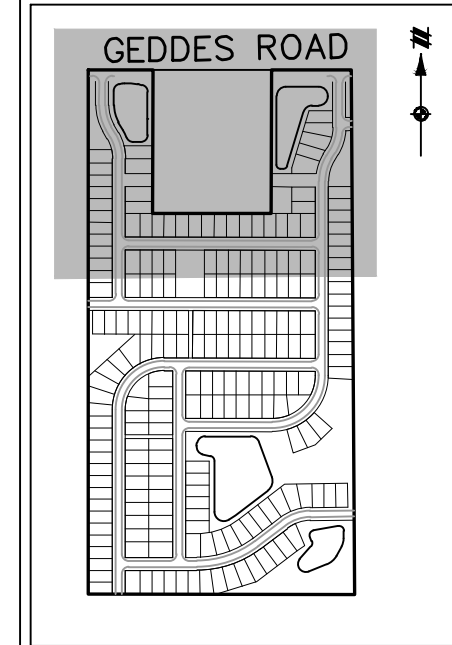
ALIGNMENT CURVE TABLE									
CURVE #	RADIUS	LENGTH	DELTA	CHD. BEARING	CHORD LENGTH	POINT OF CURVE	POINT OF AGENCY	POINT OF INTERSECTION	DEGREE OF CURVE
C2	230.00'	121.40'	30°14'31"	S17°37'11"E	119.99'	(N 283184.35, E 13326008.92)	(N 283069.99, E 13326045.24)	(N 283122.26, E 13326011.63)	24.91°
C3	230.00'	121.40'	30°14'31"	S17°37'11"E	119.99'	(N 283031.85, E 13326069.77)	(N 282917.48, E 13326106.09)	(N 282979.57, E 13326103.38)	24.91°
C5	230.00'	127.32'	31°42'58"	S13°21'33"W	125.70'	(N 282972.90, E 13327166.42)	(N 282850.60, E 13327137.38)	(N 282915.87, E 13327134.53)	24.91°
C4	230.00'	127.32'	31°42'58"	S13°21'33"W	125.70'	(N 283124.83, E 13327212.04)	(N 283002.53, E 13327182.99)	(N 283059.56, E 13327214.89)	24.91°

SANDERS MARK S & AMY K
PIN: J-10-28-300-004
A2 ZONING

ROCKETTE LLOYD A & PATRICIA J
PIN: J-10-28-300-033
A2 ZONING

LEGEND

- BOUNDARY LINE
- EX. EASEMENT
- SECTION LINE
- BOUNDARY/PROPERTY LINE
- EX. SETBACK
- EX. TREE LINE
- EX. CURB AND GUTTER
- EX. WETLAND BUFFER
- PR. SETBACK
- PR. LOT LINE
- PR. ROAD CENTERLINE
- PR. BACK OF CURB
- PR. PHASE LINE
- PR. CONCRETE WALK
- PR. ASPH.
- GENERAL COMMON ELEMENT
- NATURE PATH
- EXISTING WETLAND
- GARAGE/DRIVEWAY LOCATION



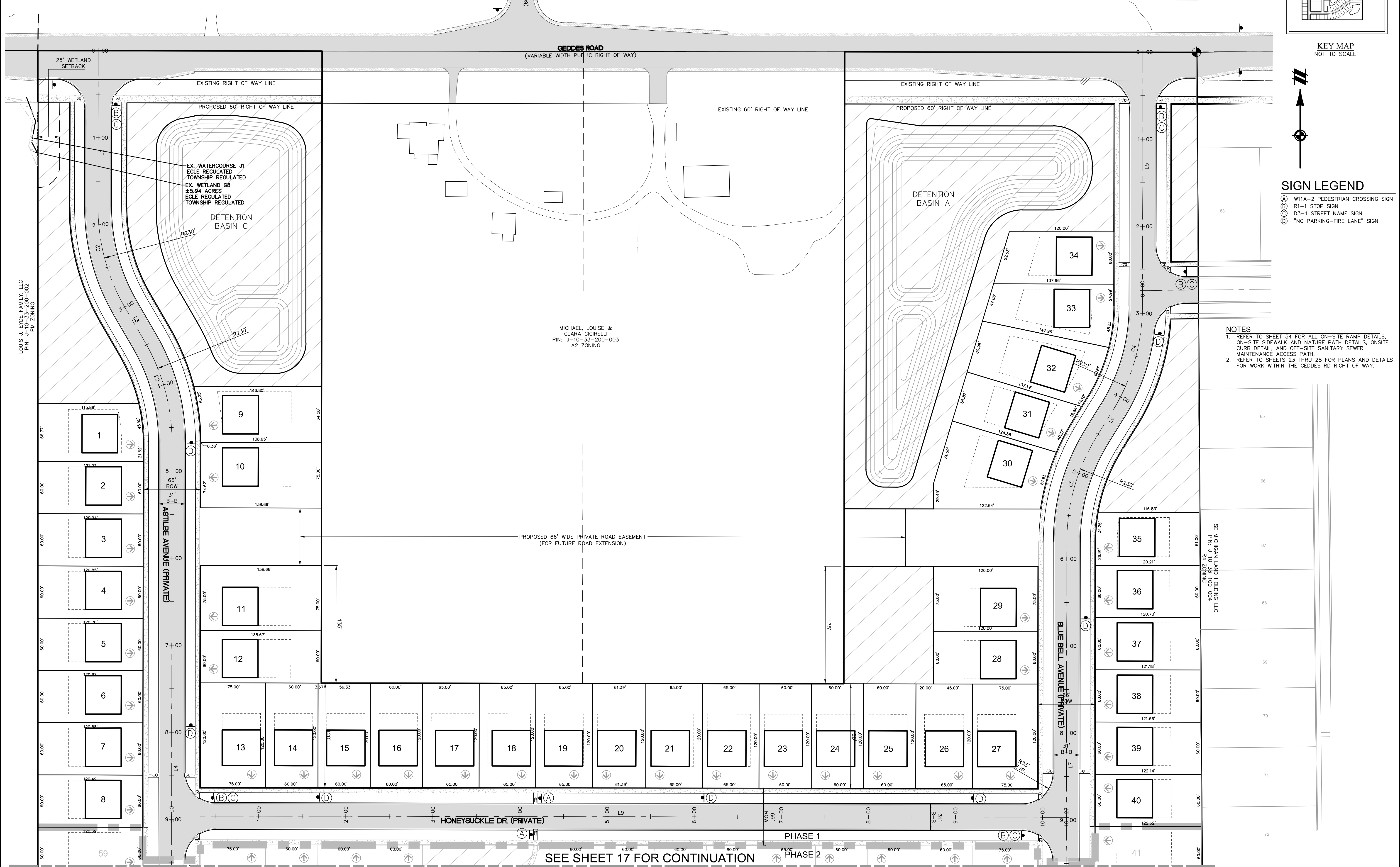
KEY MAP
NOT TO SCALE

SIGN LEGEND

- (A) W11A-2 PEDESTRIAN CROSSING SIGN
- (B) R1-1 STOP SIGN
- (C) D3-1 STREET NAME SIGN
- (D) "NO PARKING-FIRE LANE" SIGN

NOTES

- REFER TO SHEET 54 FOR ALL ON-SITE RAMP DETAILS, ON-SITE SIDEWALK AND NATURE PATH DETAILS, ON-SITE CURB DETAIL, AND OFF-SITE SANITARY SEWER MAINTENANCE ACCESS PATH.
- REFER TO SHEETS 23 THRU 28 FOR PLANS AND DETAILS FOR WORK WITHIN THE GEDDES RD RIGHT OF WAY.



SEE SHEET 17 FOR CONTINUATION

811
Know what's below.
Call before you dig.
THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED BY THE OWNER OR ITS REPRESENTATIVE. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

NOTICE:
CONSTRUCTION SITE SAFETY IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. NEITHER THE OWNER NOR THE ENGINEER SHALL BE EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF THE WORK OF PERSONS ENGAGED IN THE WORK, OF ANY NEARBY STRUCTURES, OR OF ANY OTHER PERSONS.

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SOUTHFIELD, MI 48076
248.447.2606

SECTION 33
TOWN 2 SOUTH, RANGE 7 EAST
SUPERIOR TOWNSHIP
WASHTENAW COUNTY, MICHIGAN

EYE COMPANY
THE MEADOWS AT HAWTHORNE MILL
FINAL SITE PLANS - PHASE 1
LAYOUT PLAN

DATE
OCT. 12, 2023

REVISIONS

SCALE: 1" = 50 FEET

DRAWN BY: KS
CHECKED BY: AK
P.M.: J. KIME
JOB #: 19004443
FILE CODE: -
SHEET NO. 16

K:\19004443\DWG\PLAN SET\SITE-FINAL\PHASE 1\19004443\SP-09-LDWG-10/10/2023 3:43 PM MATTHEW MYERS

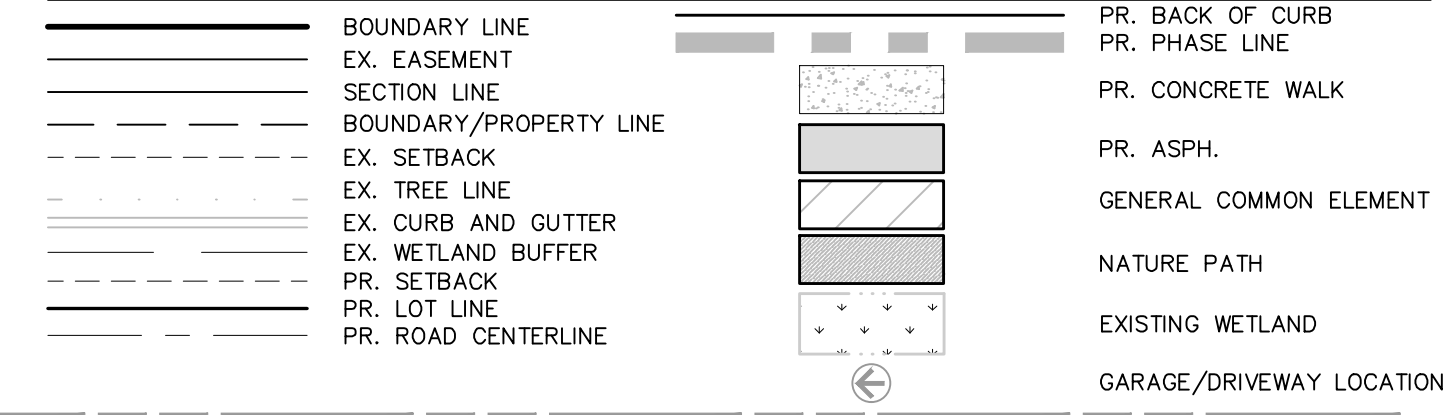
CAD FILE: 19004443\SP-09-LDWG

ASTILBE AVE. - HONEYSUCKLE DR. - BLUE BELL AVE. GOLDENROD WAY - N. DAYLILY DR.

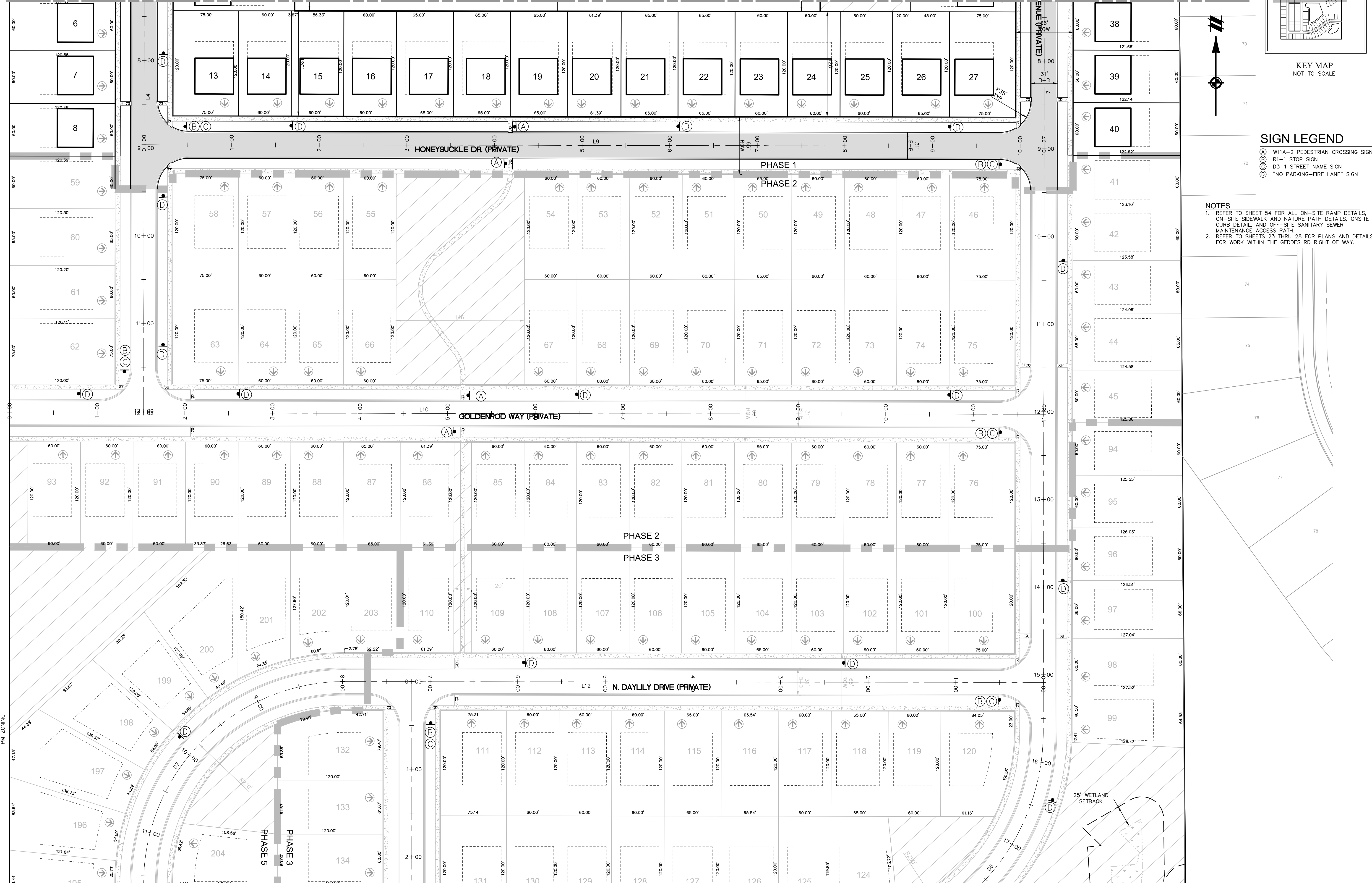
ALIGNMENT LINE TABLE				
LINE #	LENGTH	DIRECTION	START POINT	END POINT
L4	734.62	S2° 29' 55.60"E	(N 282917.48, E 13326106.09)	(N 282183.56, E 13326138.12)
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L10	1180.39	N87° 30' 04.40"E	(N 282176.89, E 13325985.26)	(N 282228.35, E 13327164.53)
L12	796.61	S87° 30' 04.40"W	(N 281922.64, E 13327177.87)	(N 281887.91, E 13326382.02)

ALIGNMENT CURVE TABLE										
CURVE #	RADIUS	LENGTH	DELTA	CHD. BEARING	CHORD LENGTH	POINT OF CURVE	POINT OF AGENCY	POINT OF INTERSECTION	DEGREE OF CURVE	
C6	250.00'	392.70'	90°00'00"	S42°30'04"W	353.55'	(N 281866.69, E 13327180.31)	(N 281606.03, E 13326941.45)	(N 281616.93, E 13327191.21)	22.92°	
C7	230.00'	361.62'	90°04'59"	S42°27'35"W	325.51'	(N 281887.91, E 13326382.02)	(N 281647.77, E 13326162.28)	(N 281877.87, E 13326151.90)	24.91°	

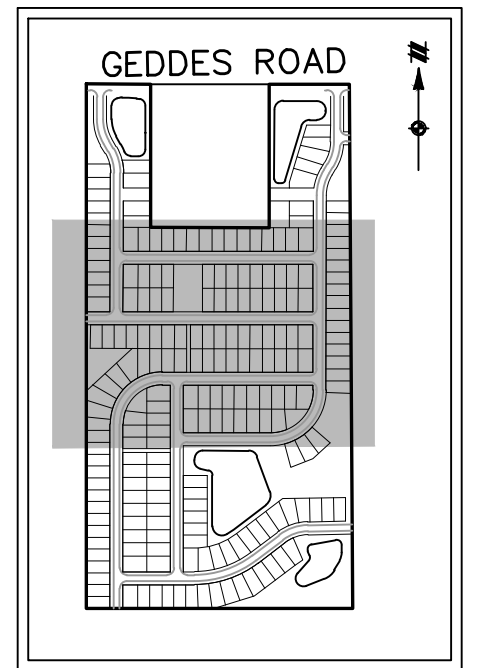
LEGEND



SEE SHEET 16 FOR CONTINUATION



- NOTES**
- REFER TO SHEET 54 FOR ALL ON-SITE RAMP DETAILS, ON-SITE SIDEWALK AND NATURE PATH DETAILS, ON-SITE CURB DETAIL, AND OFF-SITE SANITARY SEWER MAINTENANCE ACCESS PATH.
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811
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SECTION 33
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K:\19004443\DWG\PLAN SET\SITE-FINAL\PHASE 1\19004443.PLT - 10/10/2023 3:43 PM MATTHEW MYERS
 LOUIS J. EYDE FAMILY, LLC
 P.M. J. KIME
 P.M. ZONING

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SECTION 33

TOWN 2 SOUTH, RANGE 7 EAST
SUPERIOR TOWNSHIP
WASHTENAW COUNTY, MICHIGAN

EYE COMPANY
THE MEADOWS AT HAWTHORNE MILL
FINAL SITE PLANS - PHASE 1
GRADING PLAN

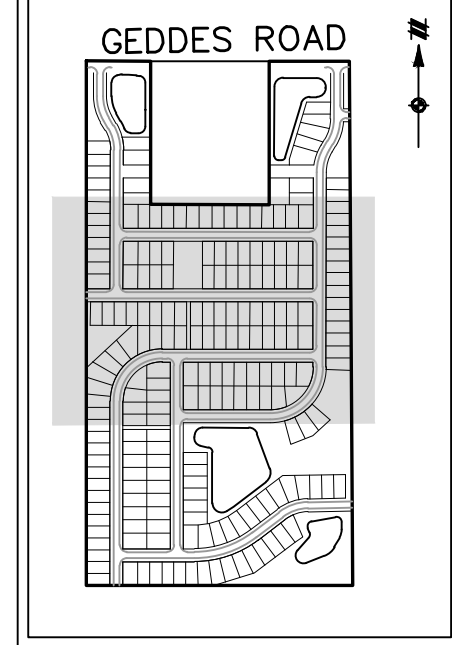
DATE
OCT. 12, 2023

REVISIONS
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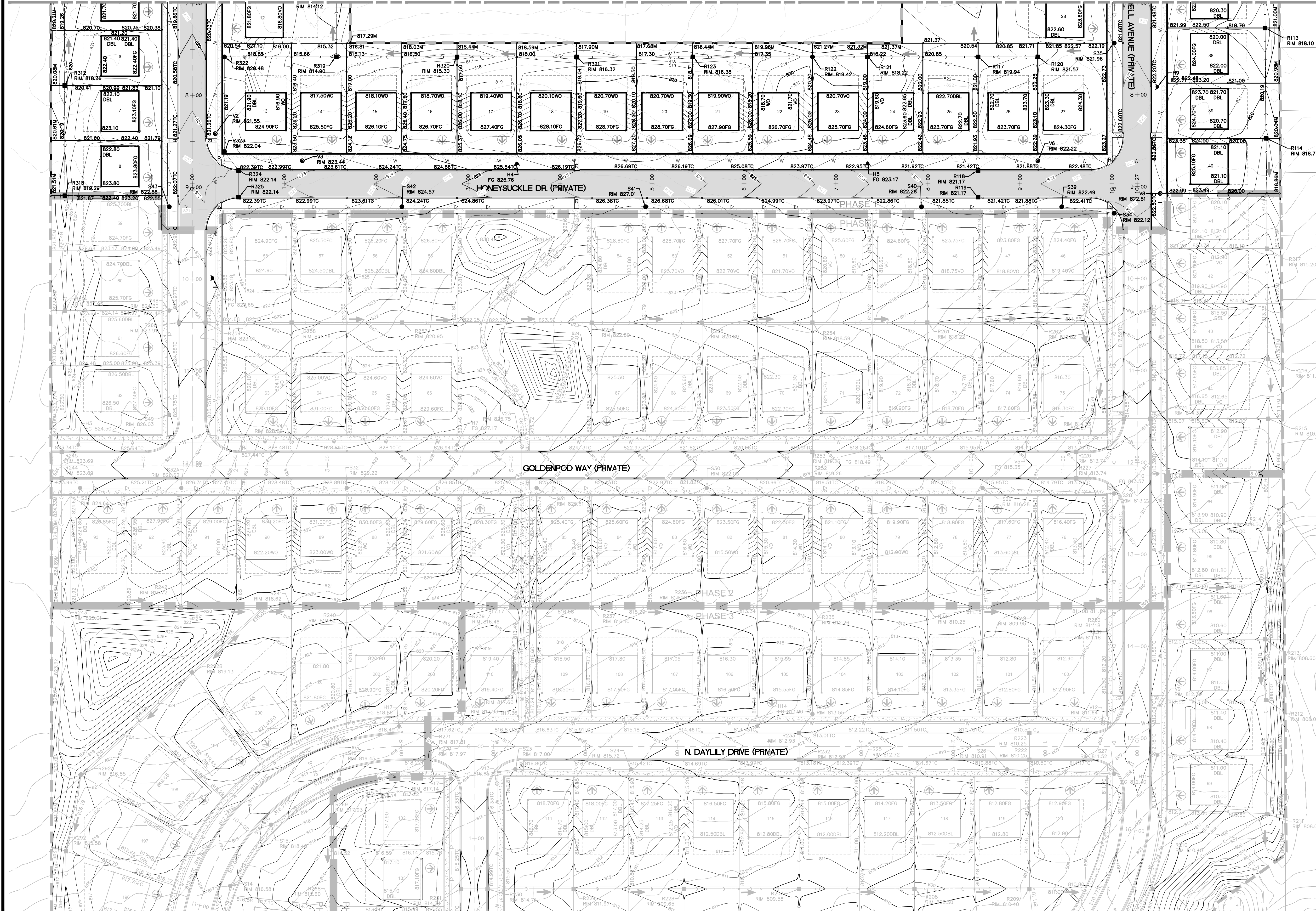
LEGEND

- 5.88 PR. 1' CONTOUR
- 5.90 PR. 5' CONTOUR
- 5.90 EX. 5' CONTOUR
- PROPERTY LINE
- GRADING LIMITS
- EX. MANHOLE / CATCH BASIN
- PR. MANHOLE / CATCH BASIN
- EX. HYDRANT
- PR. HYDRANT
- EX. GATE VALVE & WELL
- PR. GATE VALVE & WELL
- EX. WATER MAIN MANHOLE
- PR. SANITARY SEWER MANHOLE
- EX. SANITARY SEWER MANHOLE
- EX. WETLAND
- PR. ASPHALT
- PR. CONCRETE
- PR. EMERGENCY OVERFLOW ROUTE
- PR. DRIVEWAY

SPOT GRADE DESIGNATIONS
FG = FINISH GRADE
DBL = DROP BRICK LEDGE
WV = WETLAND
WO = WALKOUT
RM = RIM GRADE / FLOW LINE
HP = HIGH POINT
TC = TOP OF CURB
TW = TOP OF WALK
TW = TOP OF WALL
B/WALL = BOTTOM OF WALL



SEE SHEET 19 FOR CONTINUATION

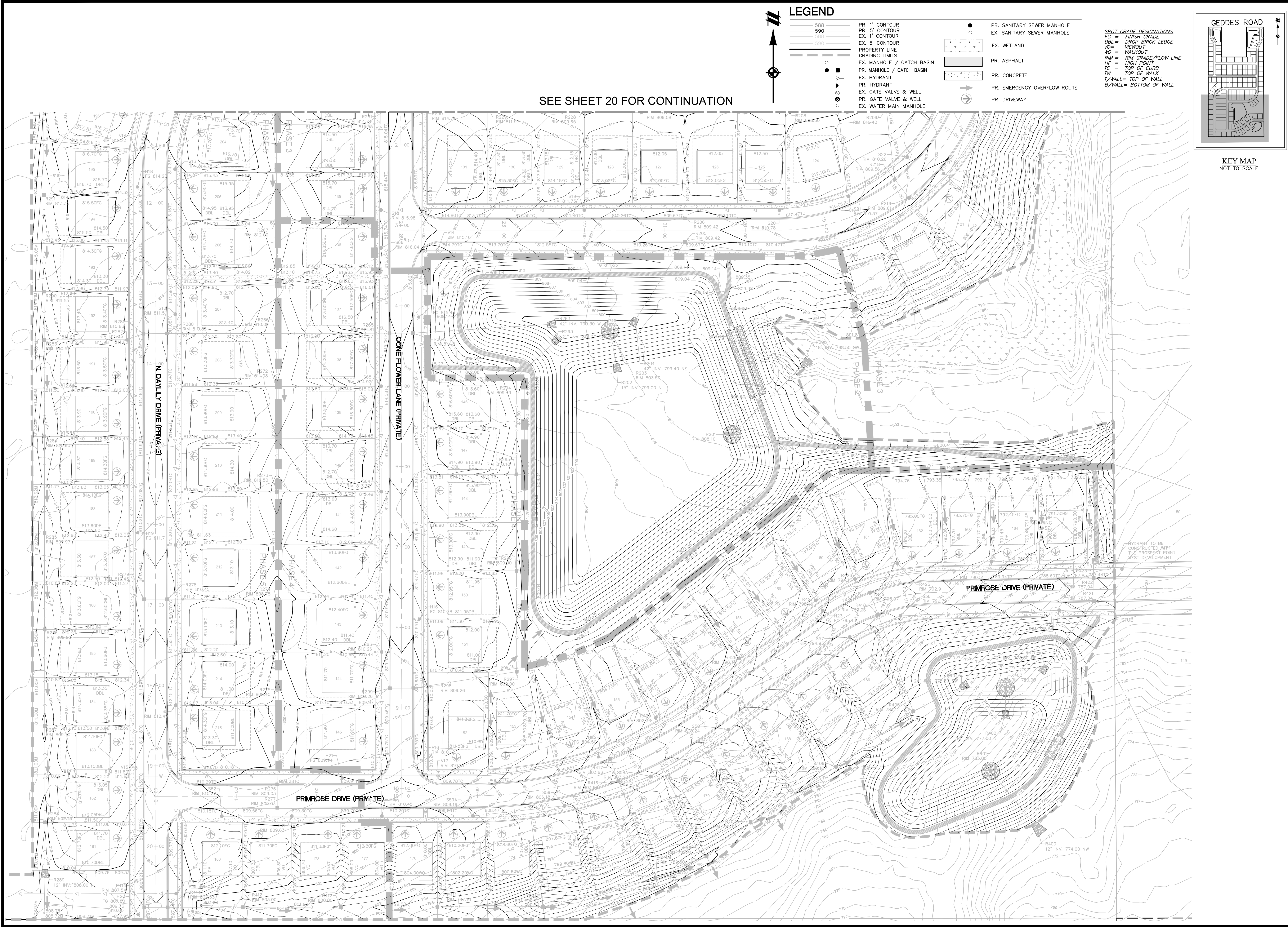


SEE SHEET 21 FOR CONTINUATION

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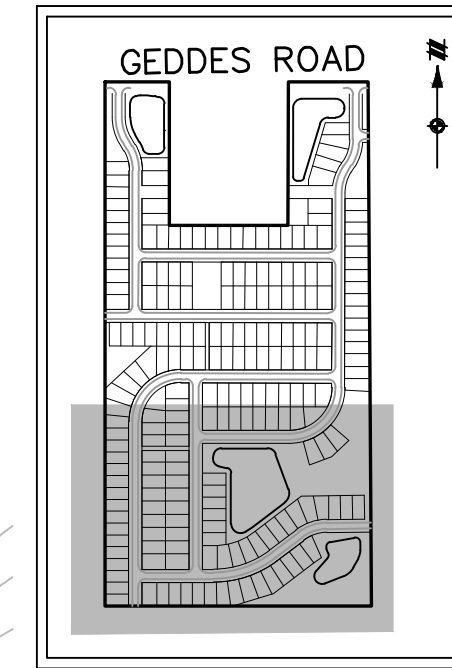
SEE SHEET 20 FOR CONTINUATION



LEGEND

- 588 PR. 1" CONTOUR
- 590 PR. 5" CONTOUR
- 588 EX. 1" CONTOUR
- 590 EX. 5" CONTOUR
- PROPERTY LINE
- GRADING LIMITS
- EX. MANHOLE / CATCH BASIN
- PR. MANHOLE / CATCH BASIN
- EX. HYDRANT
- PR. HYDRANT
- EX. GATE VALVE & WELL
- PR. GATE VALVE & WELL
- EX. WATER MAIN MANHOLE
- PR. SANITARY SEWER MANHOLE
- EX. SANITARY SEWER MANHOLE
- EX. WETLAND
- PR. ASPHALT
- PR. CONCRETE
- PR. EMERGENCY OVERFLOW ROUTE
- PR. DRIVEWAY

- SPOT GRADE DESIGNATIONS
- FG = FINISH GRADE
 - DBL = DROP BRICK LEDGE
 - WO = WALKOUT
 - W = WALKOUT
 - RM = RIM GRADE/FLOW LINE
 - HP = HIGH POINT
 - TC = TOP OF CURB
 - TW = TOP OF WALK
 - T/WALL = TOP OF WALL
 - B/WALL = BOTTOM OF WALL



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SECTION 33
TOWN 2 SOUTH, RANGE 7 EAST
SUPERIOR TOWNSHIP
WASHTENAW COUNTY, MICHIGAN

EYE COMPANY
THE MEADOWS AT HAWTHORNE MILL
FINAL SITE PLANS - PHASE 1
GRADING PLAN

DATE: OCT. 12, 2023

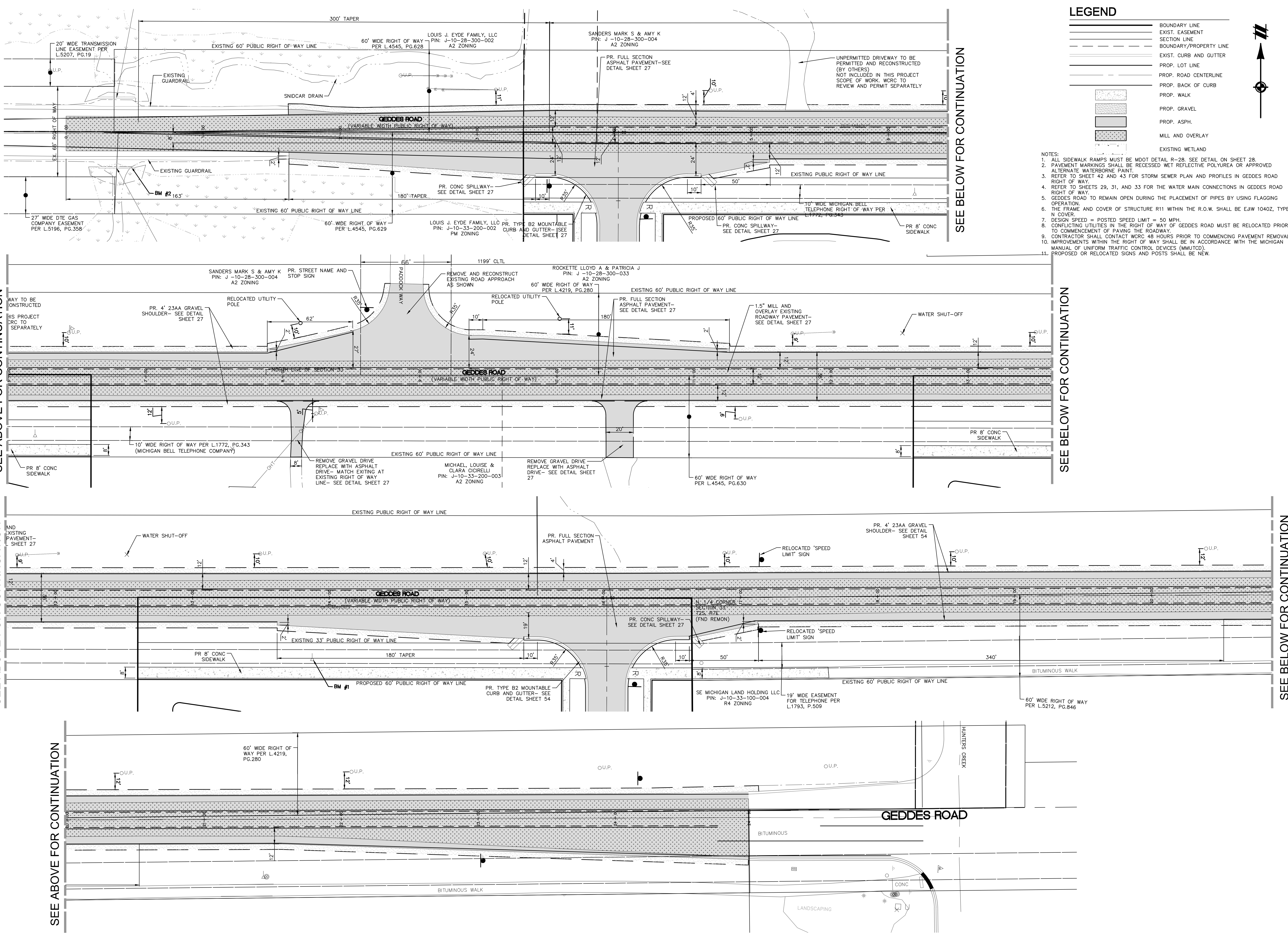
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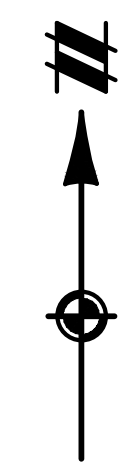
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LEGEND

- BOUNDARY LINE
EXIST. EASEMENT
SECTION LINE
BOUNDARY/PROPERTY LINE
EXIST. CURB AND GUTTER
PROP. LOT LINE
PROP. ROAD CENTERLINE
PROP. BACK OF CURB
PROP. WALK
PROP. GRAVEL
PROP. ASPH.
MILL AND OVERLAY
EXISTING WETLAND



- NOTES:
1. ALL SIDEWALK RAMP MUST BE MDOT DETAIL R-28. SEE DETAIL ON SHEET 28.
2. PAVEMENT MARKINGS SHALL BE RECESSED WET REFLECTIVE POLYUREA OR APPROVED ALTERNATE WATERBORNE PAINT.
3. REFER TO SHEET 42 AND 43 FOR STORM SEWER PLAN AND PROFILES IN GEDDES ROAD RIGHT OF WAY.
4. REFER TO SHEETS 29, 31, AND 33 FOR THE WATER MAIN CONNECTIONS IN GEDDES ROAD RIGHT OF WAY.
5. GEDDES ROAD TO REMAIN OPEN DURING THE PLACEMENT OF PIPES BY USING FLAGGING OPERATION.
6. THE FRAME AND COVER OF STRUCTURE R11 WITHIN THE R.O.W. SHALL BE EJIW 1040Z, TYPE N COVER.
7. DESIGN SPEED = POSTED SPEED LIMIT = 50 MPH.
8. CONFLICTING UTILITIES IN THE RIGHT OF WAY OF GEDDES ROAD MUST BE RELOCATED PRIOR TO COMMENCEMENT OF PAVING THE ROADWAY.
9. CONTRACTOR SHALL CONTACT WRCG 48 HOURS PRIOR TO COMMENCING PAVEMENT REMOVAL. IMPROVEMENTS WITHIN THE RIGHT OF WAY SHALL BE IN ACCORDANCE WITH THE MICHIGAN MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MMUTCD).
11. PROPOSED OR RELOCATED SIGNS AND POSTS SHALL BE NEW.

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SECTION 33 TOWN 2 SOUTH, RANGE 7 EAST SUPERIOR TOWNSHIP WASHTEENAW COUNTY, MICHIGAN

EYE COMPANY THE MEADOWS AT HAWTHORNE MILL FINAL SITE PLANS - PHASE 1 GEDDES RD. R.O.W. IMPROVEMENTS LAYOUT PLAN

DATE OCT. 12, 2023

REVISIONS table with columns for description and date. SCALE: 1" = 30 FEET. DRAWN BY: KS CHECKED BY: AK P.M.: J. KIME JOB #: 19004443 FILE CODE: - SHEET NO. 23

SEE ABOVE FOR CONTINUATION

SEE ABOVE FOR CONTINUATION

SEE ABOVE FOR CONTINUATION

SEE BELOW FOR CONTINUATION

SEE BELOW FOR CONTINUATION

SEE BELOW FOR CONTINUATION

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WASHTENAW COUNTY, MICHIGAN

EYE COMPANY
THE MEADOWS AT HAWTHORNE MILL
FINAL SITE PLANS - PHASE 1
GEDDES RD. R.O.W. IMPROVEMENTS
GRADING PLAN

DATE
OCT. 12, 2023

Table with 2 columns: REVISIONS, and empty rows for revision notes.

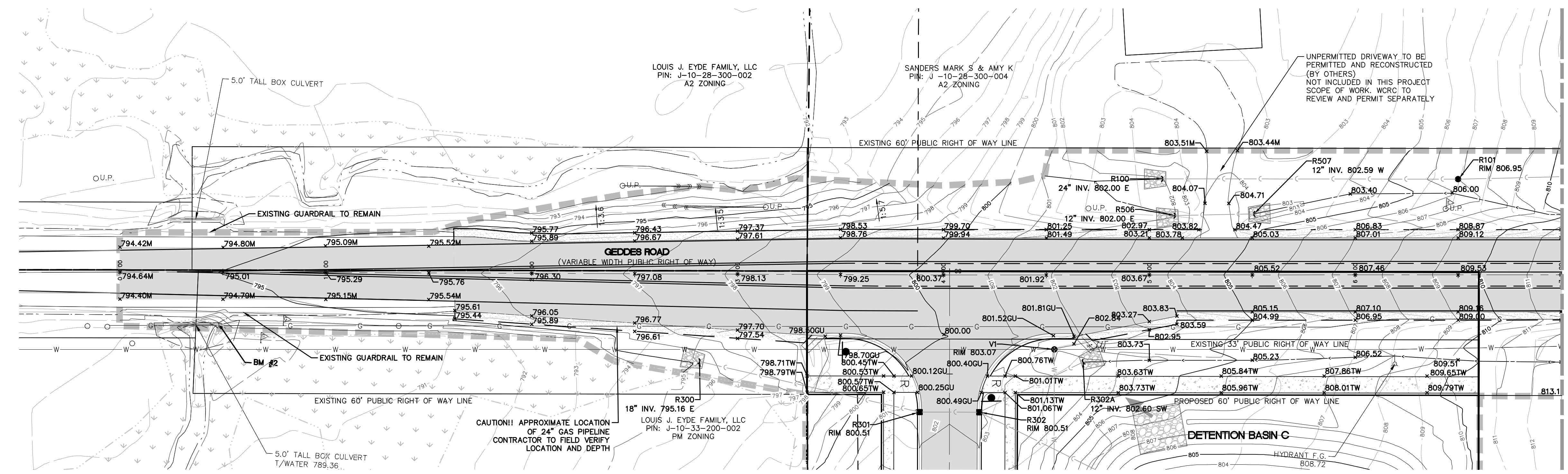
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JOB #: 19004443
FILE NO.: -
SHEET NO.: 25

LEGEND

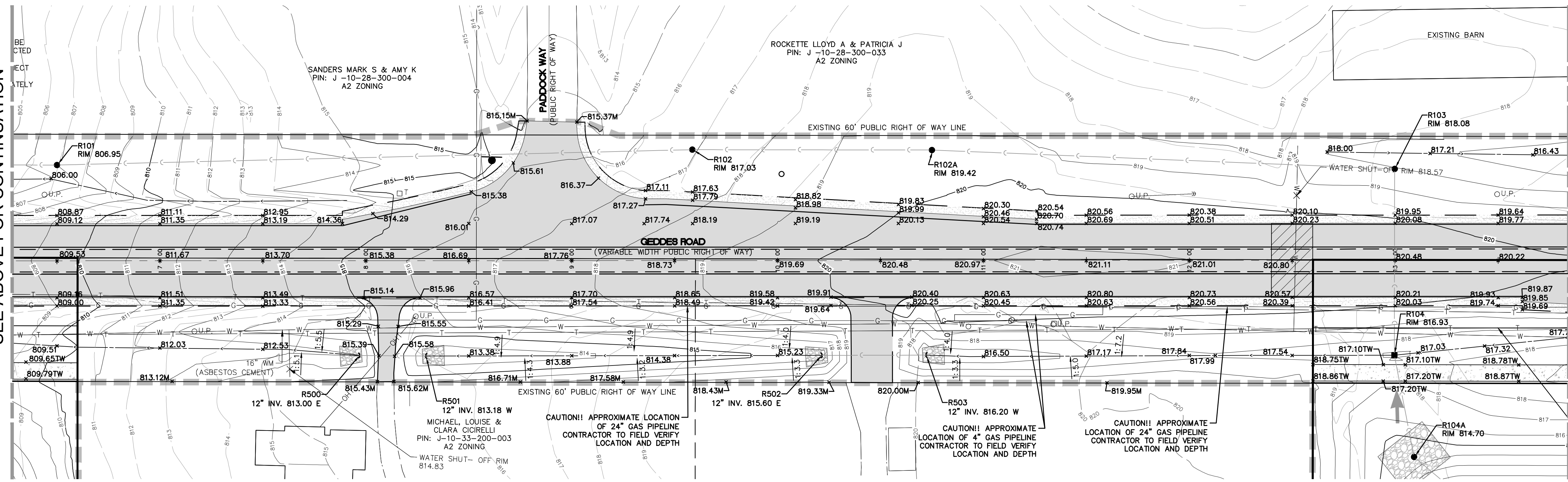
- PR. 1" CONTOUR, PR. 5" CONTOUR, EX. 1" CONTOUR, EX. 5" CONTOUR, PROPERTY LINE, GRADING LIMITS, EX. MANHOLE / CATCH BASIN, PR. MANHOLE / CATCH BASIN, PR. HYDRANT, EX. HYDRANT, EX. GATE VALVE & WELL, PR. GATE VALVE & WELL, EX. WATER MAIN MANHOLE, PR. SANITARY SEWER MANHOL, EX. SANITARY SEWER MANHOL, EX. WETLAND, PR. ASPHALT, PR. CONCRETE, PR. GRAVEL, PR. TRENCHING LIMITS

- SPOT GRADE DESIGNATIONS: FG = FINISH GRADE, DBL = DROP BRICK LEDGE, VO = VIEWOUT, WO = WALKOUT, RM = RIM GRADE/FLOW LINE, HP = HIGH POINT, TC = TOP OF CURB, TW = TOP OF WALK, T/WALL = TOP OF WALL, B/WALL = BOTTOM OF WALL

- NOTES: 1. REFER TO SHEET 27 FOR GEDDES ROAD CROSS SECTION & TYPE 'M' APPROACH DETAIL. 2. ALL SIDEWALK RAMPS MUST BE MOOT DETAIL R-28. SEE DETAIL ON SHEET 28. 3. PAVEMENT STRIPING SHALL BE WATERBORNE PAINT. 4. REFER TO SHEET 42 AND 43 FOR STORM SEWER PLAN AND PROFILES IN GEDDES ROAD RIGHT OF WAY. 5. REFER TO SHEET 29, 31, AND 33 FOR THE WATER MAIN CONNECTIONS IN GEDDES ROAD RIGHT OF WAY. 6. GEDDES ROAD TO REMAIN OPEN DURING THE PLACEMENT OF PIPES BY USING FLAGGING OPERATION. 7. THE FRAME AND COVER OF STRUCTURE R11 WITHIN THE R.O.W. SHALL BE E.I.W. 1040Z, TYPE N COVER. 8. CONTRACTOR TO FIELD VERIFY LOCATION AND DEPTH OF THE GAS LINES AND THE AT&T LINES PRIOR TO COMMENCEMENT OF CONSTRUCTION.



SEE BELOW FOR CONTINUATION

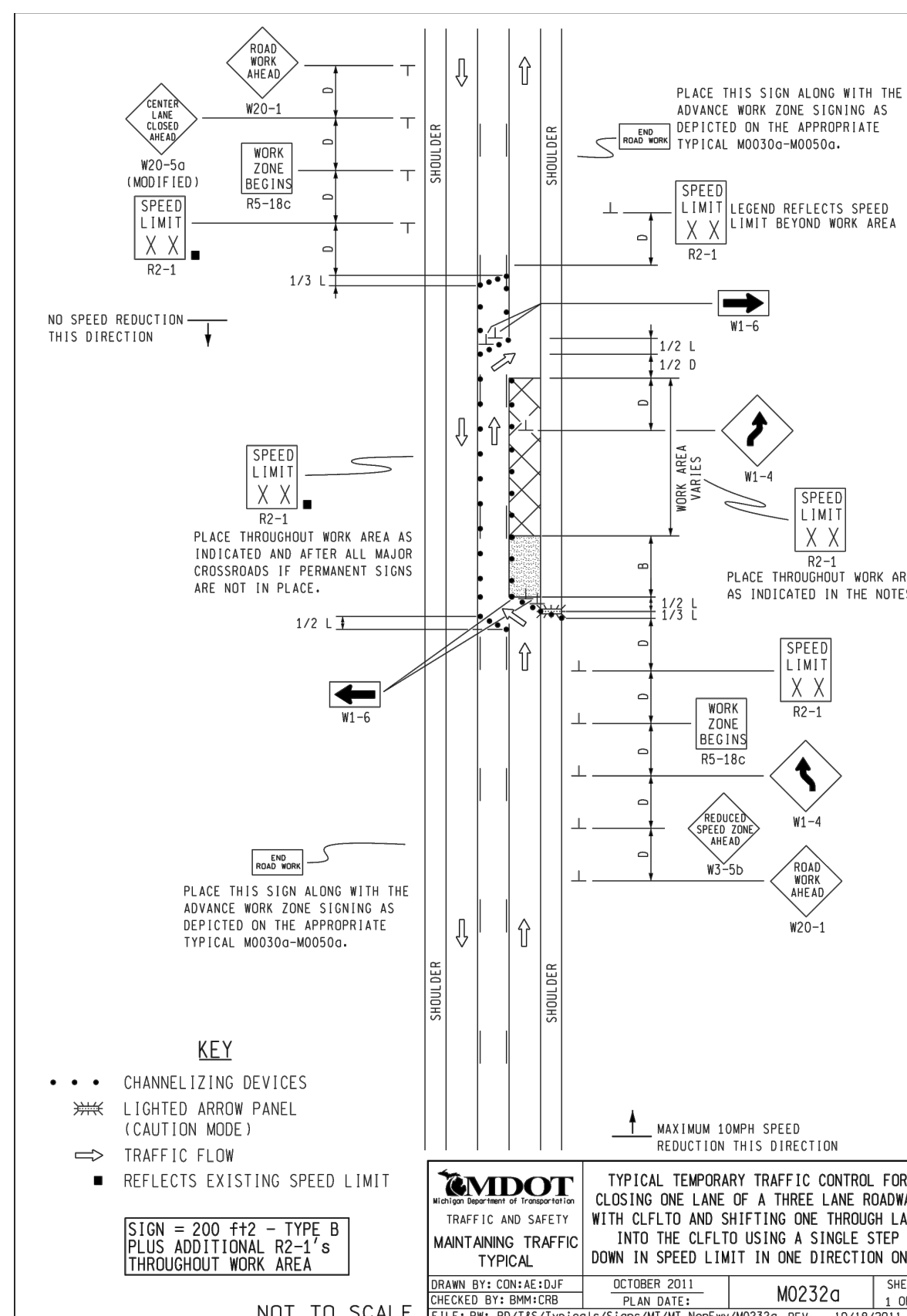


SEE ABOVE FOR CONTINUATION

SEE SH. 26 FOR CONTINUATION

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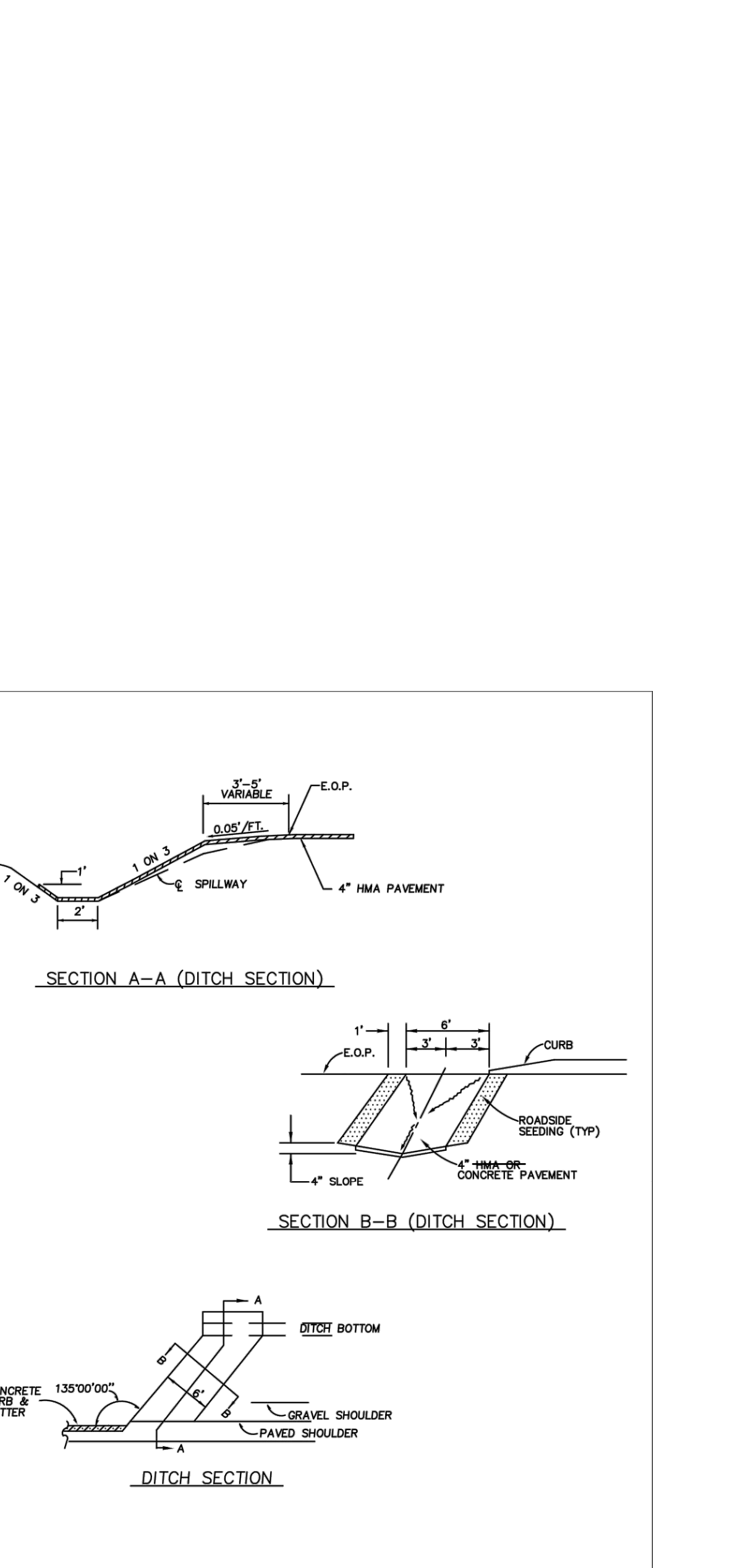
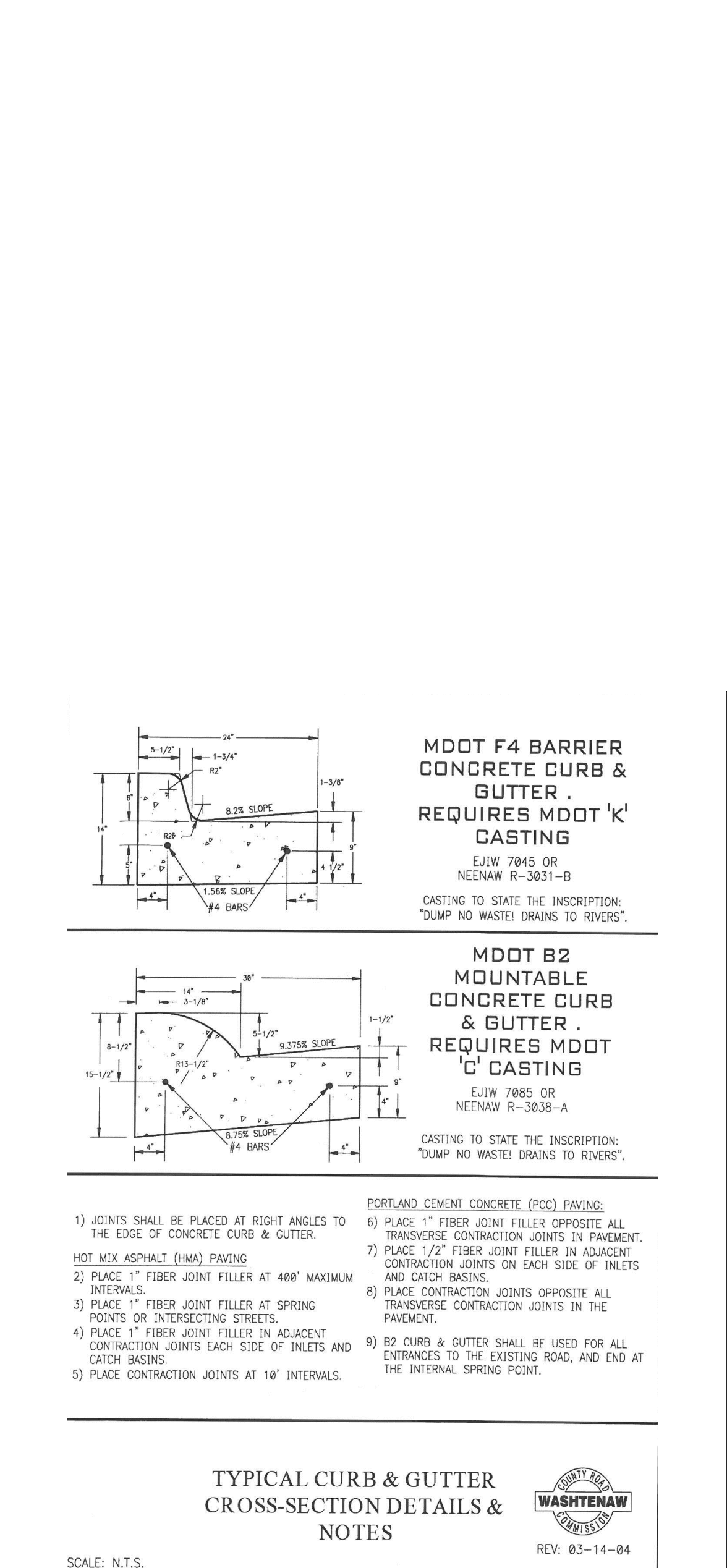
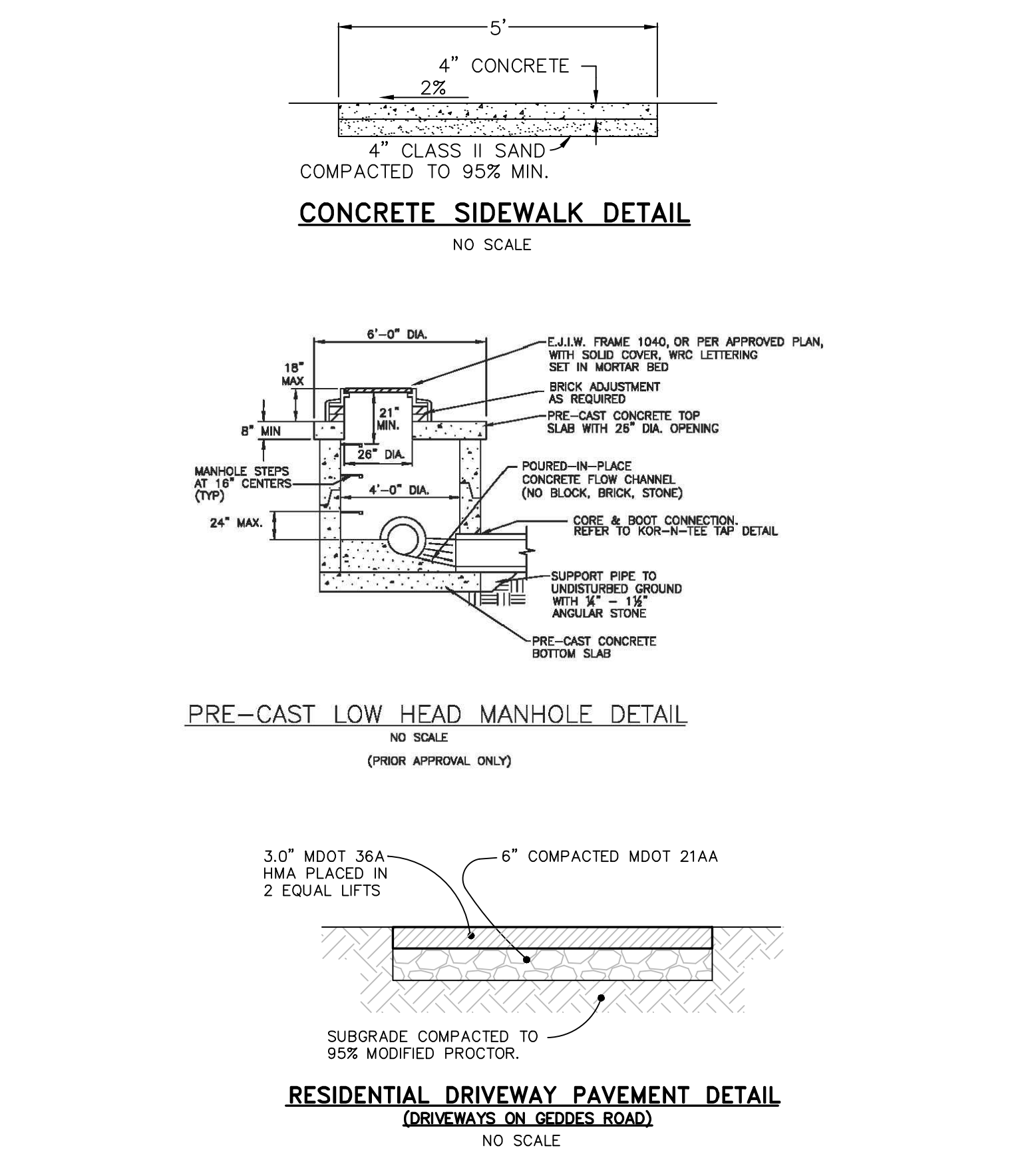
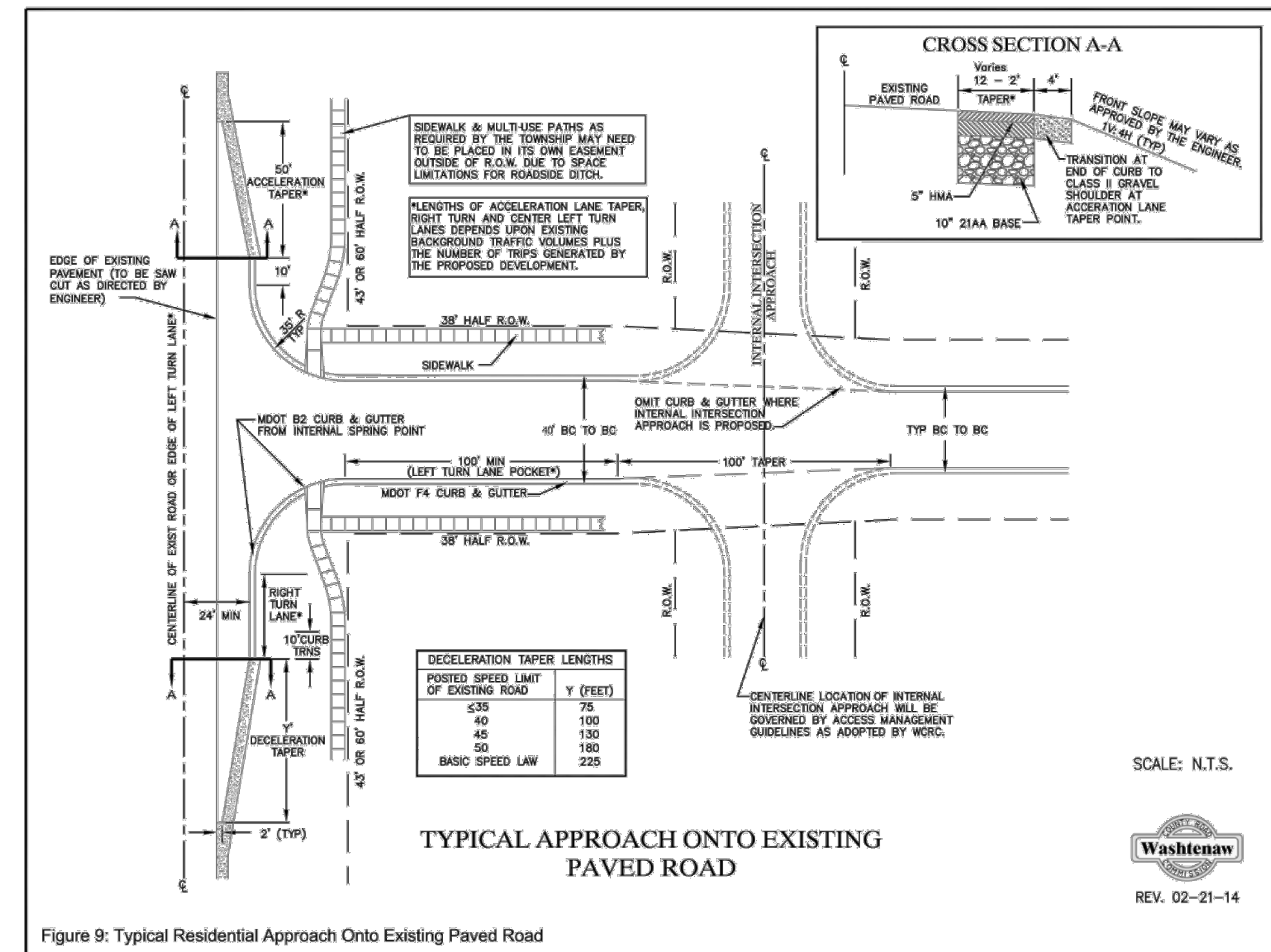
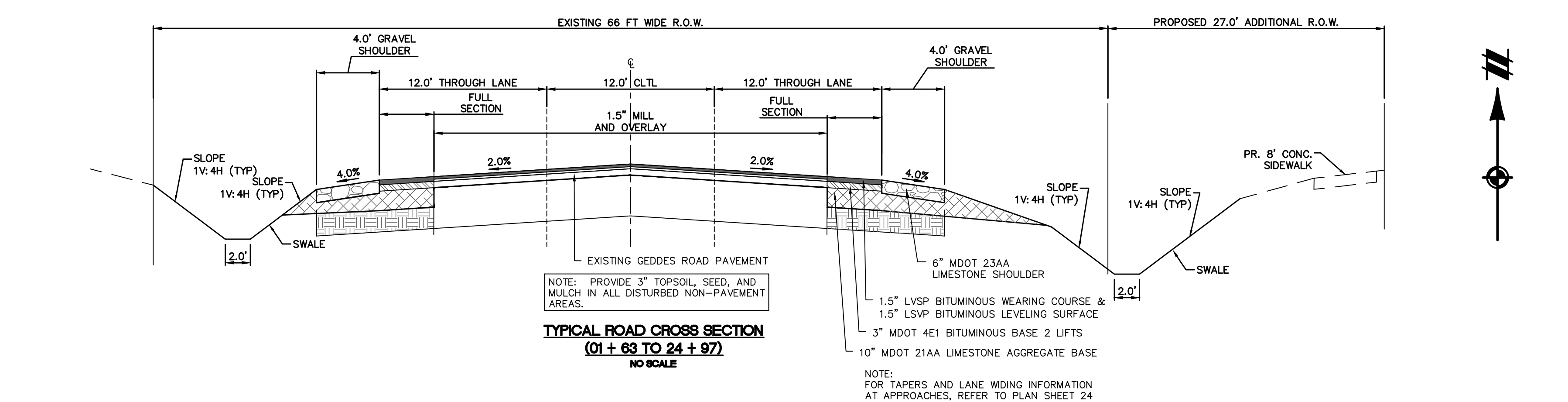
NOTES

- D = DISTANCE BETWEEN TRAFFIC CONTROL DEVICES
 $1/2 L + 1/3 L =$ MINIMAL LENGTH OF TAPER
 B = LENGTH OF LONGITUDINAL BUFFER
 SEE MD020g FOR "D," "L," AND "B" VALUES
- ALL NON-APPLICABLE SIGNING WITHIN THE CIA SHALL BE MODIFIED TO FIT CONDITIONS, COVERED OR REMOVED.
- DISTANCES BETWEEN SIGNS, THE VALUES FOR WHICH ARE SHOWN IN TABLE D, ARE APPROXIMATE AND MAY NEED ADJUSTING AS DIRECTED BY THE ENGINEER.
- THE "WORK ZONE BEGINS" (RS-18c) SIGN SHALL BE USED ONLY IN THE INITIAL SIGNING SEQUENCE IN THE WORK ZONE. SUBSEQUENT SIGNING IN THE SAME WORK ZONE SHALL OMIT THIS SIGN AND THE QUANTITIES SHALL BE ADJUSTED APPROPRIATELY.
- FOR OVERNIGHT CLOSURES, TYPE III BARRICADES SHALL BE LIGHTED.
- THE TYPE A WARNING FLASHER SHOWN ON THE WARNING SIGNS SHALL BE POSITIONED ON THE SIDE OF THE SIGN NEAREST THE ROADWAY.
- ALL TEMPORARY SIGNS, TYPE III BARRICADES, THEIR SUPPORT SYSTEMS AND LIGHTING REQUIREMENTS SHALL MEET MICHIGAN CRASHWORTHY REQUIREMENTS STIPULATED IN THE CURRENT EDITION OF THE MICHIGAN MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES, THE CURRENT EDITION OF THE STANDARD SPECIFICATIONS FOR CONSTRUCTION, THE STANDARD PLANS AND APPLICABLE SPECIAL PROVISIONS. ONLY DESIGNS AND MATERIALS APPROVED BY MDOT WILL BE ALLOWED.
- WHEN BUFFER AREAS ARE ESTABLISHED, THERE SHALL BE NO EQUIPMENT OR MATERIALS STORED OR WORK CONDUCTED IN THE BUFFER AREA.
- ADDITIONAL SPEED LIMIT SIGNS REFLECTING THE REDUCED SPEED SHALL BE PLACED AFTER EACH MAJOR CROSSROAD THAT INTERSECTS THE WORK AREA WHERE THE REDUCED SPEED IS IN EFFECT, AND AT INTERVALS ALONG THE ROADWAY SUCH THAT NO SPEED LIMIT SIGNS REFLECTING THE REDUCED SPEED ARE MORE THAN TWO MILES APART.
- WHEN REDUCED SPEED LIMITS ARE UTILIZED IN THE WORK AREA, ADDITIONAL SPEED LIMIT SIGNS RETURNING TRAFFIC TO ITS NORMAL SPEED SHALL BE PLACED BEYOND THE LIMITS OF THE REDUCED SPEED AS INDICATED.
- WHEN EXISTING SPEED LIMITS ARE REDUCED MORE THAN 10 MPH, THE SPEED LIMIT SHALL BE STEPPED DOWN IN NO MORE THAN 10 MPH INCREMENTS.
- ALL EXISTING PAVEMENT MARKINGS WHICH ARE IN CONFLICT WITH EITHER PROPOSED CHANGES IN TRAFFIC PATTERNS OR PROPOSED TEMPORARY TRAFFIC MARKINGS, SHALL BE REMOVED BEFORE ANY CHANGE IS MADE IN THE TRAFFIC PATTERN. EXCEPTION WILL BE MADE FOR DAYTIME-ONLY TRAFFIC PATTERNS THAT ARE ADEQUATELY DELINEATED BY OTHER TRAFFIC CONTROL DEVICES.

SIGN SIZES

MDOT TRAFFIC AND SAFETY
MDOT TRAFFIC AND SAFETY
MDOT TRAFFIC AND SAFETY

NOT TO SCALE



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 FINAL SITE PLANS - PHASE 1
 GEDDES RD. R.O.W. IMPROVEMENTS
 DETAILS

DATE
 OCT. 12, 2023

REVISIONS

NA NA NA
 N/A

DRAWN BY: KS
 CHECKED BY: AK
 P.M.: J. KIME
 JOB #: 19004443
 FILE CODE: -
 SHEET NO. 27

CAD FILE: 19004443SP-26-EXT.DWG

* MAXIMUM LANDING SLOPE IS 2.0% IN EACH DIRECTION OF TRAVEL. LANDING MINIMUM DIMENSIONS 5' x 5'. SEE NOTES.
 ** MAXIMUM RAMP CROSS SLOPE IS 2.0%. RUNNING SLOPE 5% - 7% (8.3% MAXIMUM). SEE NOTES.

CURB RAMP TYPE R
(ROLLED SIDES)

CURB RAMP TYPE F
(FLARED SIDES, TWO RAMPS SHOWN)

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BUREAU OF DEVELOPMENT STANDARD PLAN FOR
CURB RAMP AND DETECTABLE WARNING DETAILS
 5-8-2020 PLAN DATE
R-28-J SHEET 1 OF 1

* MAXIMUM LANDING SLOPE IS 2.0% IN EACH DIRECTION OF TRAVEL. LANDING MINIMUM DIMENSIONS 5' x 5'. SEE NOTES.
 ** MAXIMUM RAMP CROSS SLOPE IS 2.0%. RUNNING SLOPE 5% - 7% (8.3% MAXIMUM). SEE NOTES.

CURB RAMP TYPE RF
(ROLLED / FLARED SIDES)

SECTION A-A

CURB TYPE	MAXIMUM RISE (INCHES)	A	B
B1	1/4	1	1
B2	1/2	1	1
B3	3/4	1	1
D1	1/4	1	1
D2	1/2	1	1
D3	3/4	1	1
C1	1/2	1/2	1/2
C2	1/2	1/2	1/2
C3	1/2	1/2	1/2
C4	1/2	1/2	1/2
C5	1	1/2	1/2
C6	1	1/2	1/2
F1	1/2	1/2	1/2
F2	1/2	1/2	1/2
F3	1/2	1/2	1/2
F4	1/2	1/2	1/2
F5	1	1/2	1/2
F6	1	1/2	1/2

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CURB RAMP AND DETECTABLE WARNING DETAILS
 5-8-2020 PLAN DATE
R-28-J SHEET 2 OF 1

* MAXIMUM LANDING SLOPE IS 2.0% IN EACH DIRECTION OF TRAVEL. LANDING MINIMUM DIMENSIONS 5' x 5'. SEE NOTES.
 ** MAXIMUM RAMP CROSS SLOPE IS 2.0%. RUNNING SLOPE 5% - 7% (8.3% MAXIMUM). SEE NOTES.

CURB RAMP TYPE P
(PARALLEL RAMP)

CURB RAMP TYPE C
(COMBINATION RAMP)

CURB RAMP TYPE M
(MEDIAN ISLAND)

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CURB RAMP AND DETECTABLE WARNING DETAILS
 5-8-2020 PLAN DATE
R-28-J SHEET 3 OF 1

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 ** MAXIMUM RAMP CROSS SLOPE IS 2.0%. RUNNING SLOPE 5% - 7% (8.3% MAXIMUM). SEE NOTES.

CURB RAMP TYPE D
(DEPRESSED CORNER)

CURB RAMP TYPE D
(TANGENT DETECTABLE WARNING SHOWN)

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CURB RAMP AND DETECTABLE WARNING DETAILS
 5-8-2020 PLAN DATE
R-28-J SHEET 4 OF 1

* THE DETECTABLE WARNING SURFACE SHALL BE LOCATED SO THAT THE EDGE NEAREST THE RAIL CROSSING IS 6" MINIMUM AND 15" MAXIMUM FROM THE CENTERLINE OF THE NEAREST RAIL. DO NOT PLACE DETECTABLE WARNING ON RAILROAD CROSSING MATERIAL.

DETECTABLE WARNING AT RAILROAD CROSSING

DETECTABLE WARNING AT FLUSH SHOULDER OR ROADWAY

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CURB RAMP AND DETECTABLE WARNING DETAILS
 5-8-2020 PLAN DATE
R-28-J SHEET 5 OF 1

* GRADE BREAKS AT THE TOP AND BOTTOM OF CURB RAMPS SHALL BE PERPENDICULAR TO THE DIRECTION OF TRAVEL.
 ** TRANSITION ADJACENT GUTTER PAN CROSS SECTION TO PROVIDE 5.0% MAXIMUM COUNTER SLOPE ACROSS THE RAMP OPENING. SEE SHEET 2 FOR CURB RAMP OPENING DETAILS.

SECTION B-B
CURB RAMP ORIENTATION

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CURB RAMP AND DETECTABLE WARNING DETAILS
 5-8-2020 PLAN DATE
R-28-J SHEET 6 OF 1

50% TO 65% OF BASE
 0.3" TO 1.4"

DETECTABLE WARNING DETAILS

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SECTION 33
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 SUPERIOR TOWNSHIP
 WASHTEENAW COUNTY, MICHIGAN

EYE COMPANY
 THE MEADOWS AT HAWTHORNE MILL
 FINAL SITE PLANS - PHASE 1
 GEDDES RD. MDOT DETAILS

DATE
 OCT. 12, 2023

REVISIONS
 NA NA NA
 N/A

DRAWN BY: KS
 CHECKED BY: AK
 P.M.: J. KIME
 JOB #: 19004443
 FILE CODE: -
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CAD FILE: 19004443SP-28-EXT.DWG



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WASHTENAW COUNTY, MICHIGAN

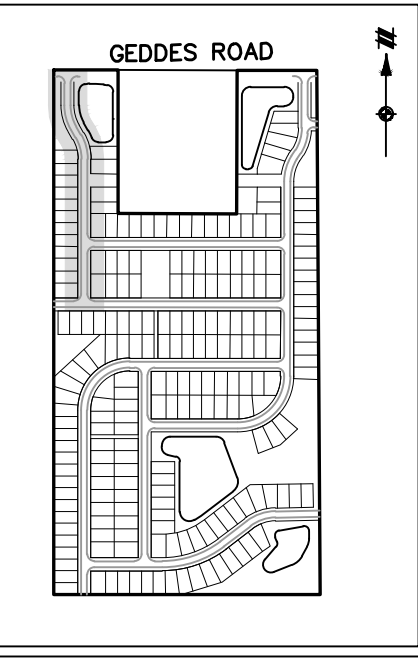
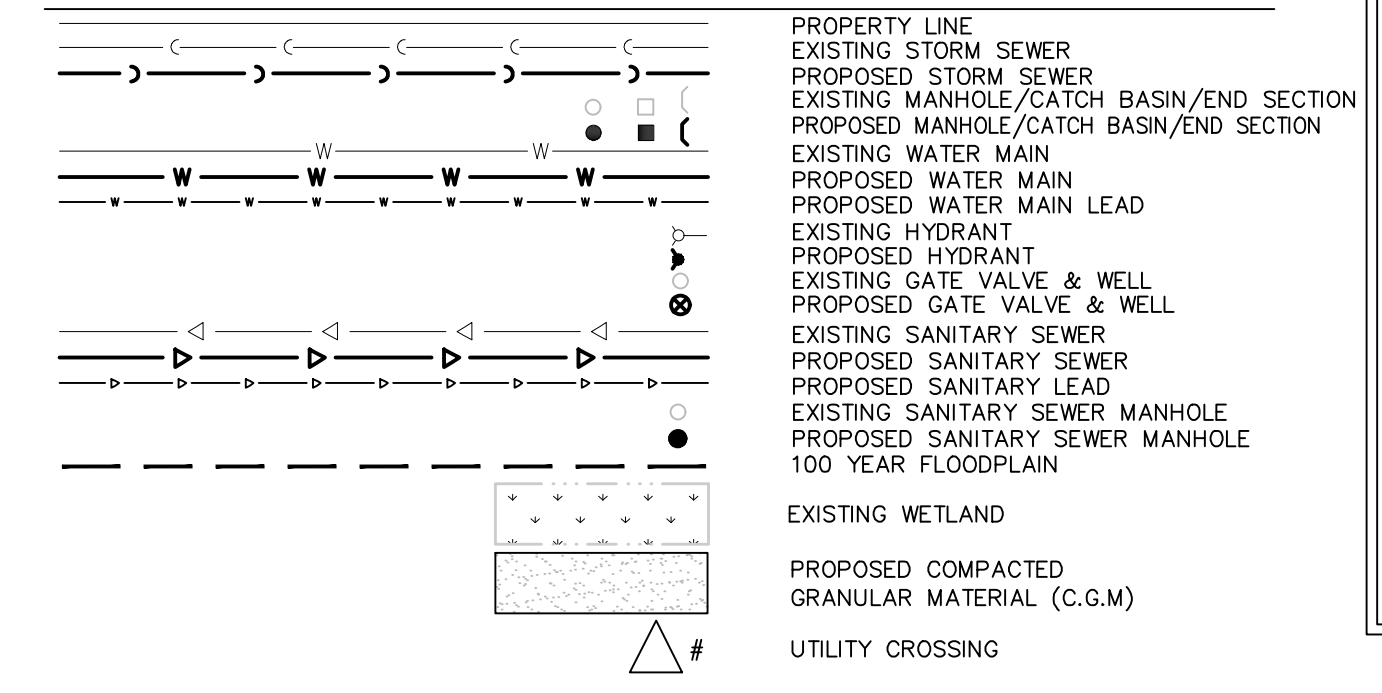
EYE COMPANY
THE MEADOWS AT HAWTHORNE MILL
FINAL SITE PLANS - PHASE 1
ROAD & WATERMAIN
PLAN & PROFILE -
ASTILBE AVE.

DATE
OCT. 12, 2023

REVISIONS table with columns for description and date.

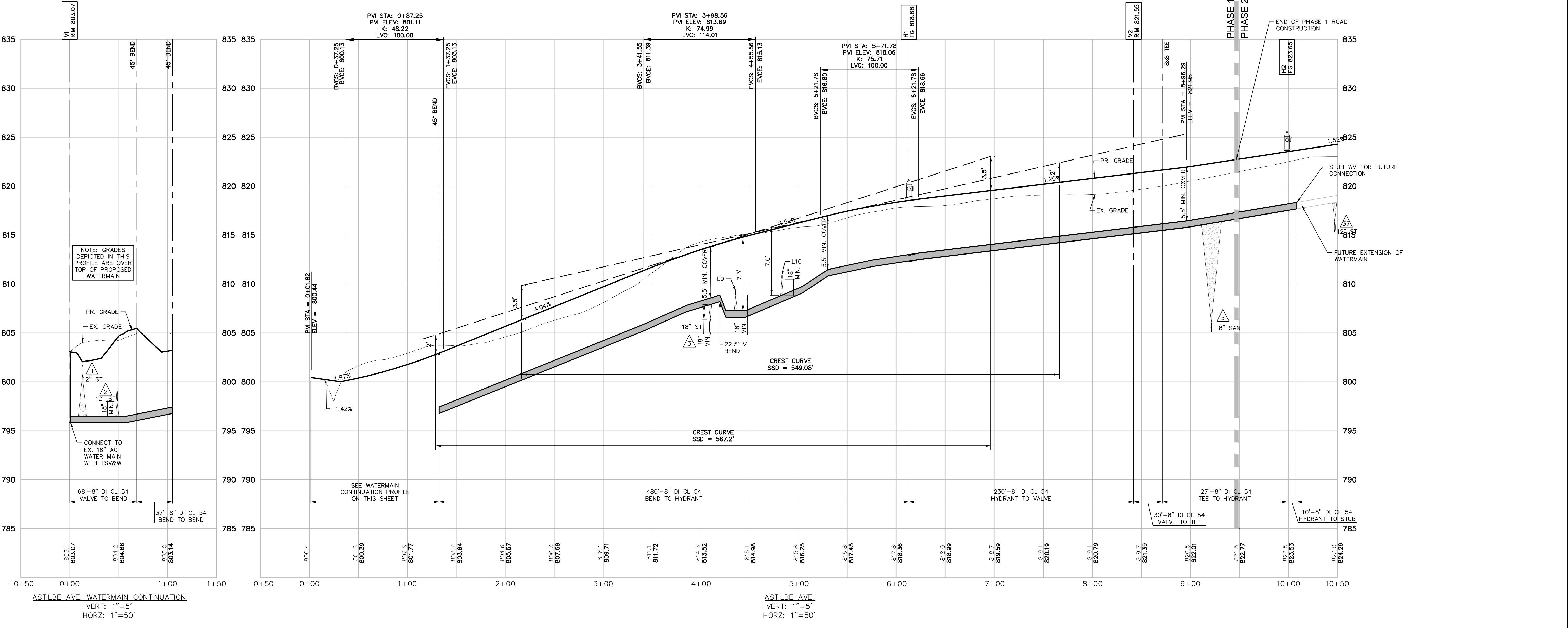
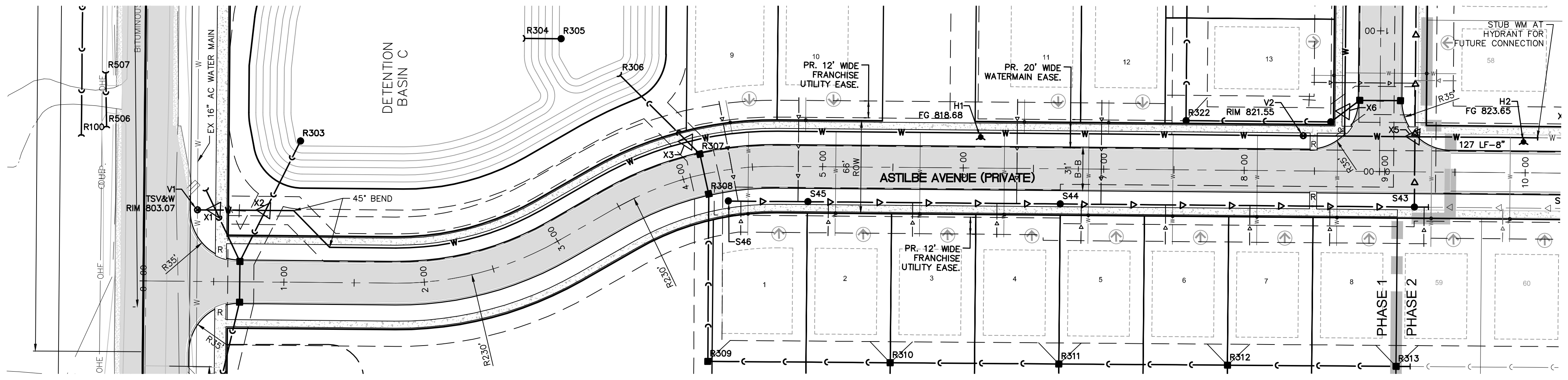
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P.M.: J. KIME
JOB #: 19004443
FILE CODE: -
SHEET NO. 29

LEGEND



NOTE: WHERE THE WATER MAIN PASSES A STORM STRUCTURE WITH LESS THAN 10' OF HORIZONTAL SEPARATION, A FULL STICK OF PIPE SHALL BE INSTALLED (CENTERED AT THE STRUCTURE) TO LOCATE THE WATER MAIN JOINTS 10' AWAY FROM THE STRUCTURE.

NOTE: ACCORDING TO UTILITIES DEPARTMENT RECORDS THE EXISTING GEDDES ROAD WATER MAIN IS A 16" AC (ASBESTOS CEMENT) PIPE.



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SECTION 33 TOWN 2 SOUTH, RANGE 7 EAST SUPERIOR TOWNSHIP WASHTEENAW COUNTY, MICHIGAN

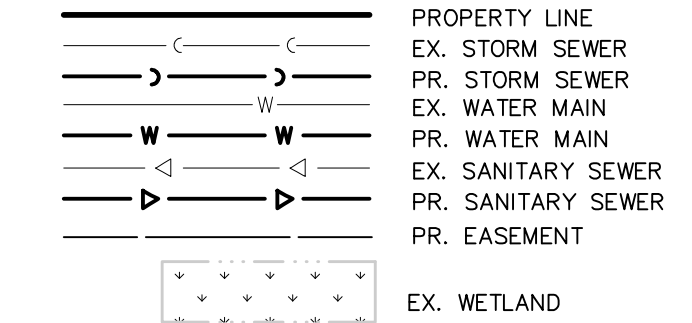
EYE COMPANY THE MEADOWS AT HAWTHORNE MILL FINAL SITE PLANS - PHASE 1 OVERALL UTILITY PLAN

DATE OCT. 12, 2023

REVISIONS table with columns for date, description, and initials.

SCALE: 1" = 150 FEET DRAWN BY: KS CHECKED BY: AK P.M.: J. KIME JOB #: 19004443 FILE CODE: SHEET NO. 32

LEGEND



NOTES

- 1. ALL UTILITIES SHALL BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AUTHORITY HAVING JURISDICTION.
2. CONTRACTOR SHALL COORDINATE SCHEDULING AND PROCEDURES WITH THE AUTHORITY HAVING JURISDICTION PRIOR TO PERFORMING THE PROPOSED CONNECTIONS TO THE EXISTING WATER MAIN.
3. 10' HORIZONTAL SEPARATION SHALL BE MAINTAINED BETWEEN WATER MAINS AND SEWERS.
4. A MINIMUM VERTICAL SEPARATION OF 18" SHALL BE MAINTAINED BETWEEN CROSSING UTILITIES, UNLESS OTHERWISE APPROVED BY THE ENGINEER.
5. ALL PROPOSED STORM SEWER SHALL BE RCP CL-IV UNLESS OTHERWISE NOTED.
6. ALL STORM STRUCTURES RECEIVING STORM RUNOFF, EXCEPT FOR 2 FOOT DIAMETER INLETS, ARE TO HAVE 2-FOOT SLOPES PER SUPERIOR TOWNSHIP STANDARDS.
7. LOW HEAD (FLAT TOP) STRUCTURES SHALL BE PROVIDED FOR THE STORM STRUCTURES WITH LESS THAN FOUR FEET OF COVER.
8. ALL WATER MAIN SHALL BE DUCTILE IRON CL-54 AS APPROVED. WATER LEADS SHALL BE 1" TYPE K COPPER, UNLESS OTHERWISE APPROVED. ALL GATE VALVES EXCEPT HYDRANT SHUTOFFS SHALL BE INSTALLED IN WELLS.
9. ALL SANITARY LEADS SHALL BE 6" PVC SDR 23.5 LAID AT 1% MINIMUM SLOPE.
10. ALL SANITARY SEWER PIPE SHALL BE PVC SDR 26 TO 20 FEET DEPTH PVC TRUSS PIPE FOR SANITARY SEWER DEEPER THAN 20 FEET FROM FINISH GRADE.
11. THE MINIMUM COVER OF SANITARY SEWERS 4'.
12. TRAFFIC CONTROL TO BE PROVIDED IN ACCORDANCE WITH THE MMUTCD & COUNTY ROAD COMMISSION.
13. MDOT CLASS II SAND BACKFILL IS REQUIRED FOR ALL UTILITY BACKFILL UNDER OR WITHIN A 1:1 INFLUENCE OF PROPOSED ROADWAYS.
14. ALL STORM SEWER SHALL BE PLACED IN 20' WIDE DRAINAGE EASEMENTS. CROSS LOT AND REAR LOT SWALES WILL BE PLACED IN 30' WIDE DRAINAGE EASEMENTS.
15. ALL SANITARY SEWER WILL BE PLACED IN EASEMENTS. WIDER EASEMENTS HAVE BEEN PROVIDED FOR MAINTENANCE ACCESS ON DEEP SANITARY SEWERS.
16. ALL WATER MAIN WILL BE PLACED IN A MINIMUM 20' WIDE EASEMENTS.
17. AT THIS TIME, LOTS 1, 9, AND 10 ARE ANTICIPATED TO BE SERVICED BY HUNG PLUMBING.
18. STOP BOXES SHALL BE INSTALLED AT THE R.O.W. LINE (AT 1 FT. BEHIND THE SIDEWALK).
19. WATER LEADS TO ROW LINE SHALL BE INSTALLED BY THE CONTRACTOR FROM THE WATER MAIN AND AFTER WATER MAIN TESTING ACCEPTANCE.
20. SUMP LEADS SHALL BE PVC PIPE AT 1% MINIMUM SLOPE.
21. GATE WELL AND FRAME SHALL BE EJ 1040 WITH TYPE 'C' COVER OR APPROVED EQUAL COVERS SHALL BE SAT WITH THE SUPERIOR TOWNSHIP LOGO AND THE WORDS "SUPERIOR TOWNSHIP-WATER" IN RAISED LETTERS SPACED IN FROM THE PERIPHERY OF THE COVER.
22. STORM MANHOLE COVERS AND FRAMES SHALL BE EJ 1040 WITH TYPE 'C' COVER OR APPROVED EQUAL.
23. STORM YARD INLETS COVERS AND FRAMES SHALL BE EJ M.D.O.T. BEHIVE WITH TYPE 'A' COVER OR APPROVED EQUAL.
24. CATCH BASIN COVERS AND FRAMES SHALL BE EJ 7065 OR EQUAL WHEN IN PAVEMENT EDGE OR GUTTER LINE.
25. SANITARY SEWER MANHOLE COVERS AND FRAMES SHALL BE EJ 1040 WITH TYPE 'A' COVER OR APPROVED EQUAL COVERS SHALL BE CAST WITH THE SUPERIOR TOWNSHIP LOGO AND THE WORDS "SUPERIOR TOWNSHIP-SANITARY" IN RAISED LETTERS SPACED IN FROM THE PERIPHERY OF THE COVER.

UTILITY NARRATIVE

SEWER: THE SITE WILL BE SERVICED WITH PUBLIC SANITARY SEWER VIA CONNECTION POINTS TO TWO SEPARATE SEWER DISTRICTS. SOME CAPACITY REMAINS IN THE PROSPECT POINT PUMP STATION FOR SANITARY SEWER. THE SOUTHERN-MOST UNITS WILL BE SERVICED BY A CONNECTION THROUGH PROSPECT POINT WEST TO MAKE USE OF THIS REMAINING CAPACITY. IT IS ANTICIPATED THAT UP TO 49 UNITS CAN BE ACCOMMODATED IN THE AVAILABLE CAPACITY. ALL UNITS NOT SERVICED THROUGH PROSPECT POINT WILL BE SERVICED BY CONNECTION TO EXISTING SEWER IN LEFORGE ROAD.

WATER: THE SITE WILL BE SERVICED WITH PUBLIC WATER BY CONNECTING TO AN EXISTING 12" MAIN IN GEDDES ROAD ROW AS WELL AS CONNECTING INTO TWO 8" MAINS FROM PROSPECT POINT WEST.

STORMWATER: ATWELL HAS COORDINATED WITH THE WASHTENAW COUNTY WATER RESOURCE COMMISSION RELATED TO PRELIMINARY DESIGN REQUIREMENTS AND SOILS ANALYSIS. THIS IS DISCUSSED IN MORE DETAIL ON SHEET 46.

UTILITY PHASING: DURING THE INITIAL PHASE, ALL UTILITIES NEEDED TO SERVICE LOTS 1-40 SHALL BE INSTALLED ALONG WITH THE TWO NORTH DETENTION BASINS AND OUTLETS TO THE SNIDEAR DRAIN. THE SANITARY SEWER WILL BE EXTENDED THROUGH THE ADJACENT PARCEL OUT TO LEFORGE ROAD AND WATER MAIN WILL CONNECT TO THE EXISTING 16" MAIN ALONG GEDDES AS WELL AS THE NORTHERN CONNECTION TO THE 8" MAIN IN PROSPECT POINT TO ENSURE A LOOPING SYSTEM. THE LATER PHASES OF THE DEVELOPMENT WILL ADD ADDITIONAL STORMWATER BASINS THAT FOLLOW EXISTING DRAINAGE PATTERNS TO THE SUPERIOR NO. 1 DRAIN. WATER AND SANITARY SEWER SERVICE WILL BE EXTENDED AS REQUIRED FROM THE PREVIOUS PHASES.

NOTE: THE STORM SEWER AND SURFACE DRAINAGE EASEMENTS AND THE DETENTION BASIN EASEMENT/BUFFER WILL BE GRANTED TO THE WASHTENAW COUNTY WATER RESOURCES COMMISSION (WCWR) AND THE MEADOWS AT HAWTHORNE MILL ASSOCIATION.

NOTE: THE PATH FOR THE OFFSITE SANITARY SEWER NEEDS TO SUPPORT A FULL LOADED VECTOR TRUCK. THE TRENCH BACKFILL EITHER NATIVE MATERIAL OR IMPORTED GRANULAR FILL NEEDS TO BE COMPACTED TO 95% OF ITS MAXIMUM UNIT WEIGHT TO SUPPORT THIS EQUIPMENT

SANITARY SEWER BASIS OF DESIGN - PROSPECT POINTE SERVICE AREA

Table with columns for Total No. of Single Family Lots (45), No. of Users per Lot (3.5), Total Expected Population Served (158), Average Daily Flow (per capita) (100 G.P.D.), Peaking Factor (4.18), Average Flow (15,800 G.P.D.), Peak Flow (66,101 G.P.D.), Pipe Capacity (8 in. diameter), Manning's Capacity (0.766 C.F.S.), Velocity Flowing Full (2.20 F.P.S.).

Prospect Point Pump Station Sewer Flows Summary

Table with columns for Existing Flows to date (Acres, REUs, Population, GPD, CFS, GPM, GPD, CFS, GPM, Peak Factor) and Proposed Future Flows (Acres, REUs, Population, GPD, CFS, GPM, GPD, CFS, GPM, Peak Factor).

Table with columns for Existing Pump Sta. capacity (382 GPM or 0.85 cfs) and Proposed Future Flows (Acres, REUs, Population, GPD, CFS, GPM, GPD, CFS, GPM, Peak Factor).

NOTE: THE OFF-SITE SANITARY SEWER ROUTING SHOWN TO THE LEFORGE SANITARY SEWER CONNECTION LOCATION IS FINAL. NOTE THAT THIS ADJACENT DEVELOPMENT PROPERTY IS UNDER COMMON OWNERSHIP WITH THE PROPOSED MEADOWS AT HAWTHORNE MILL DEVELOPMENT.

12' MDOT 22A SANITARY ACCESS DRIVE SEE DETAIL SHEET 54

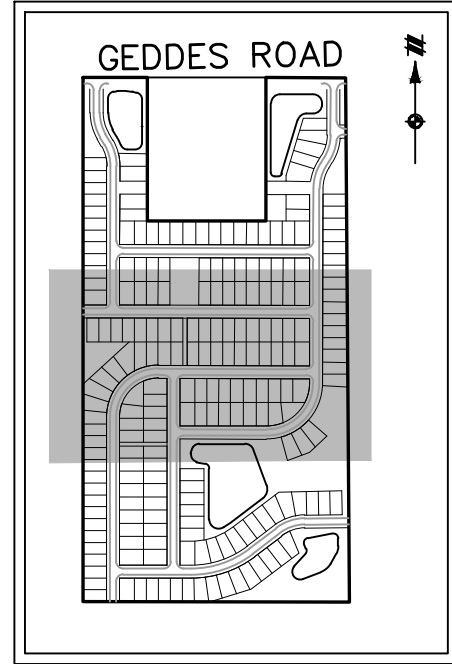
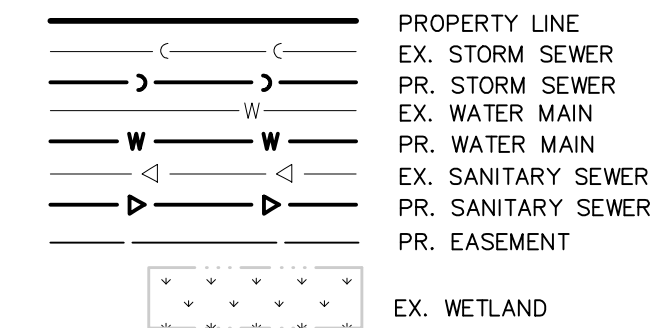
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NOTE:
 1. SANITARY LEADS FOR LOTS ON THE SOUTH SIDE OF HONEYSUCKLE DRIVE SHOULD BE INSTALLED IN PHASE 1. ALL OTHER LOTS OUTSIDE OF PHASE 1 SHALL HAVE ONLY THEIR WYES AND RISERS INSTALLED DURING PHASE 1.
 2. WATER LEADS TO ROW LINE SHALL BE INSTALLED BY THE CONTRACTOR FROM THE WATER MAIN AND AFTER WATER MAIN TESTING ACCEPTANCE.

SEE SHEET 33 FOR CONTINUATION

LEGEND



KEY MAP
NOT TO SCALE

811
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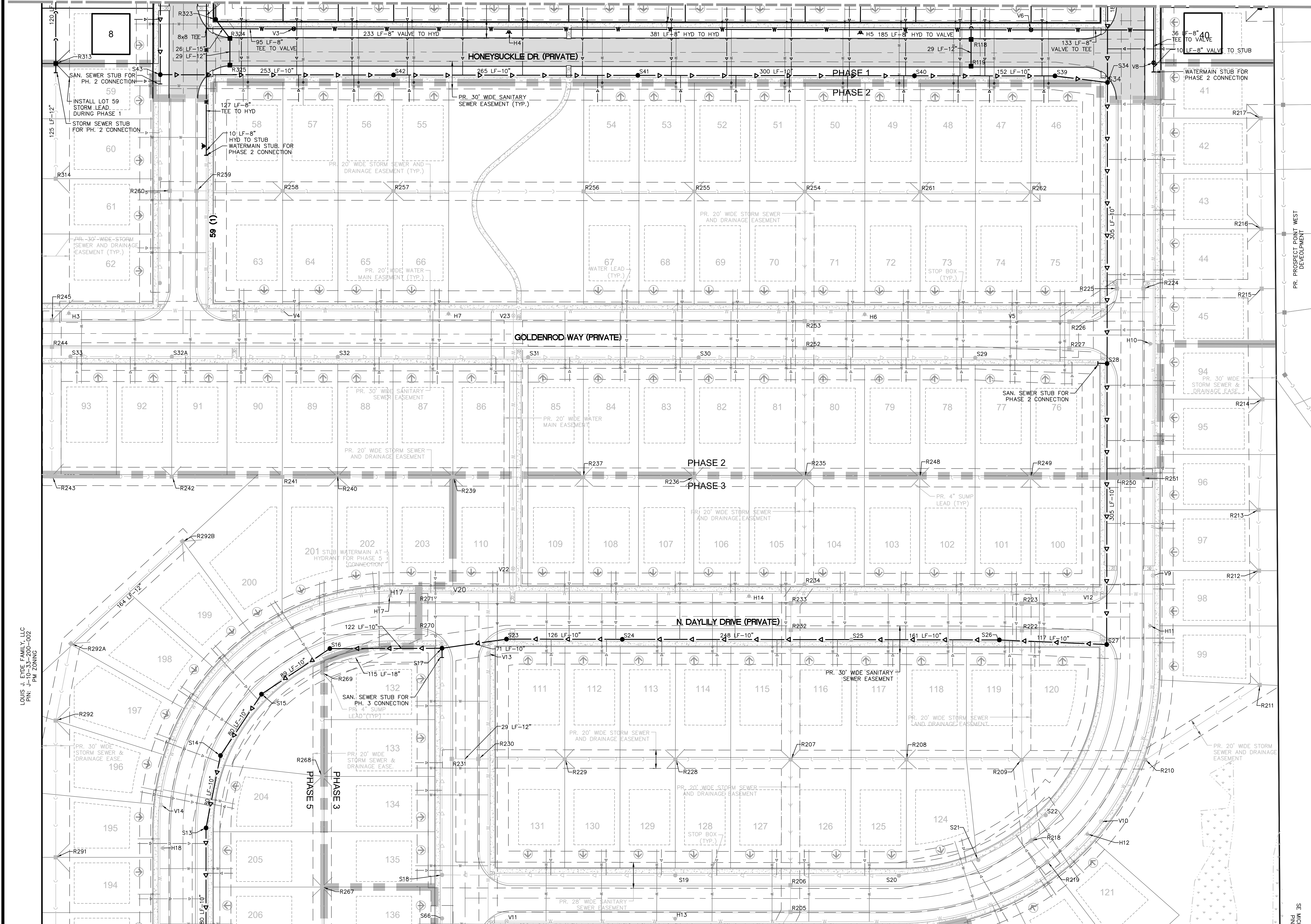
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 WASHTEENAW COUNTY, MICHIGAN

EYE COMPANY
 THE MEADOWS AT HAWTHORNE MILL
 FINAL SITE PLANS - PHASE 1
 UTILITY PLAN

DATE
 OCT. 12, 2023

REVISIONS
 SCALE: 1" = 50 FEET
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 JOB #: 19004443
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 SHEET NO. 34



SEE SHEET 35 FOR CONTINUATION

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 LOUIS J. EIDE FAMILY, LLC
 P.M. J. KIME
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WASHTENAW COUNTY, MICHIGAN

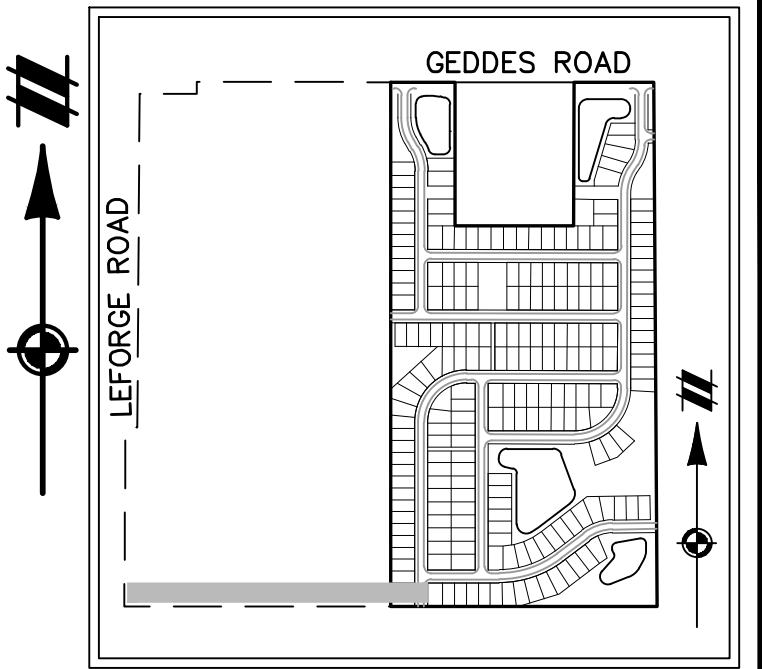
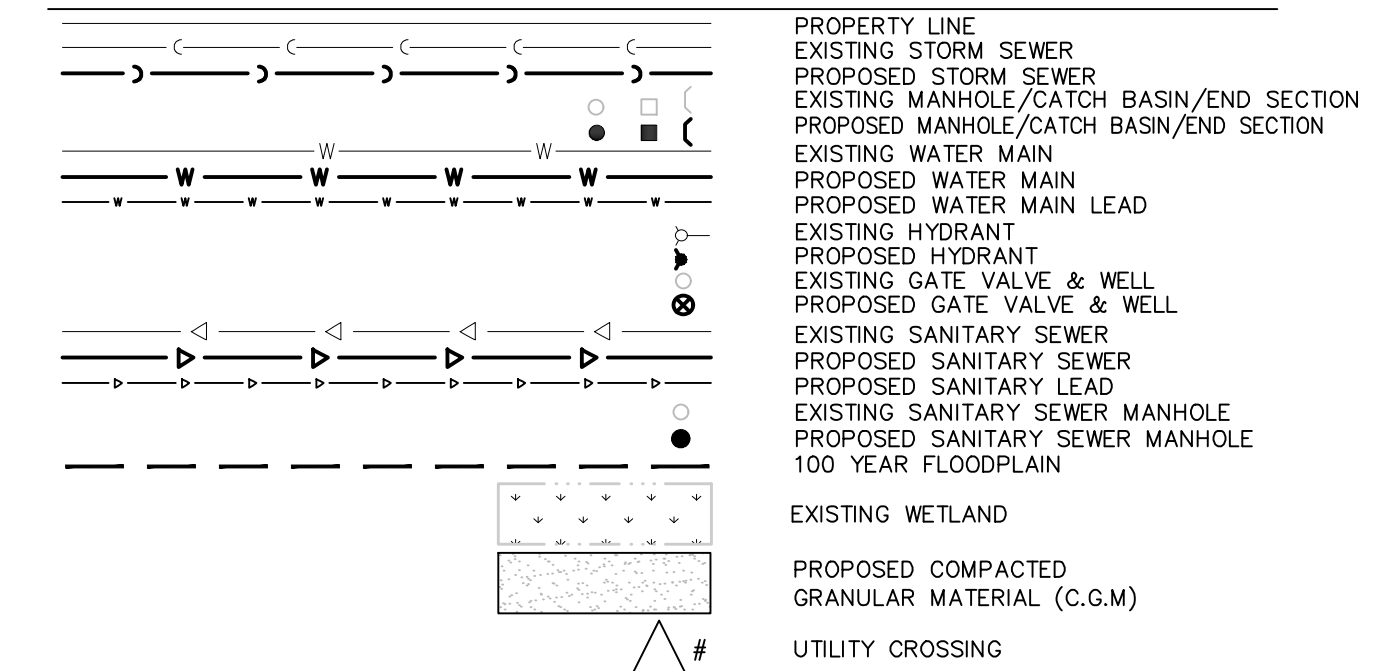
EYDE COMPANY
THE MEADOWS AT HAWTHORNE MILL
FINAL SITE PLANS - PHASE 1
OFFSITE SANITARY SEWER
PLAN & PROFILE

DATE
OCT. 12, 2023

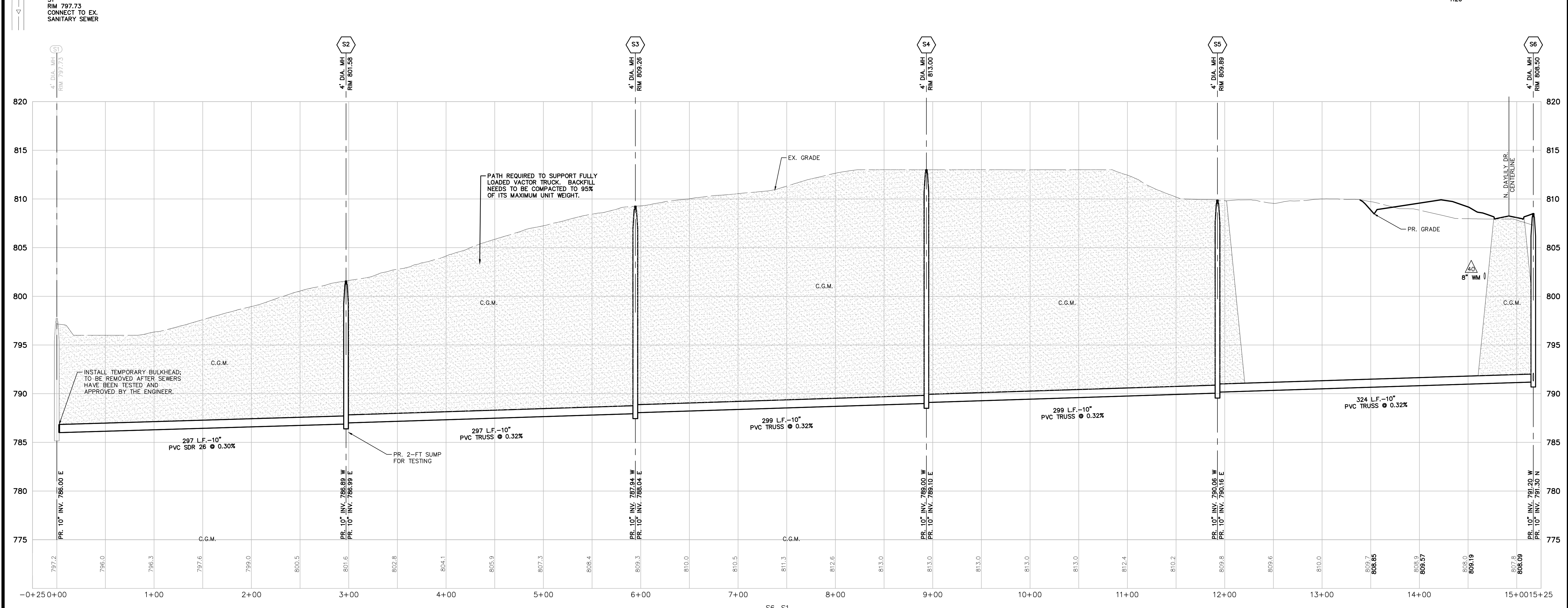
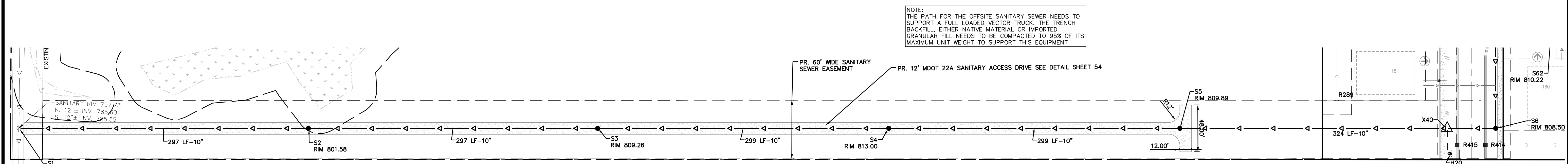
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JOB #: 19004443
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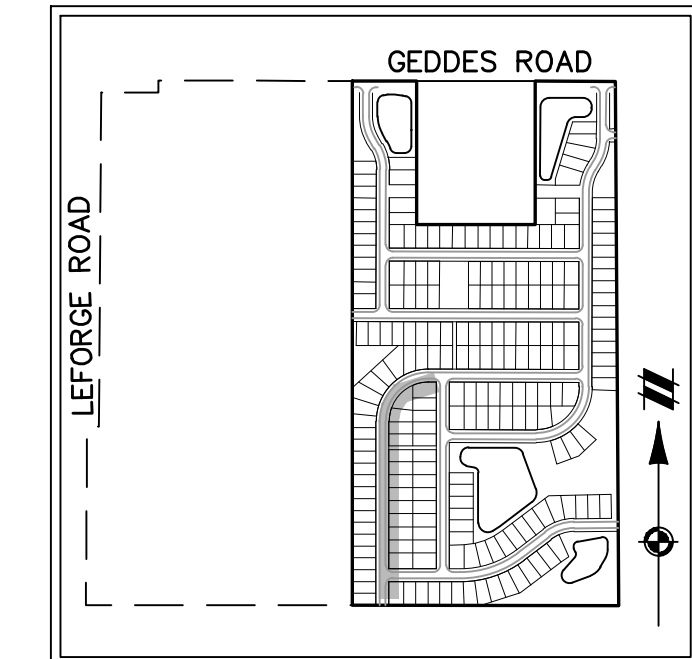
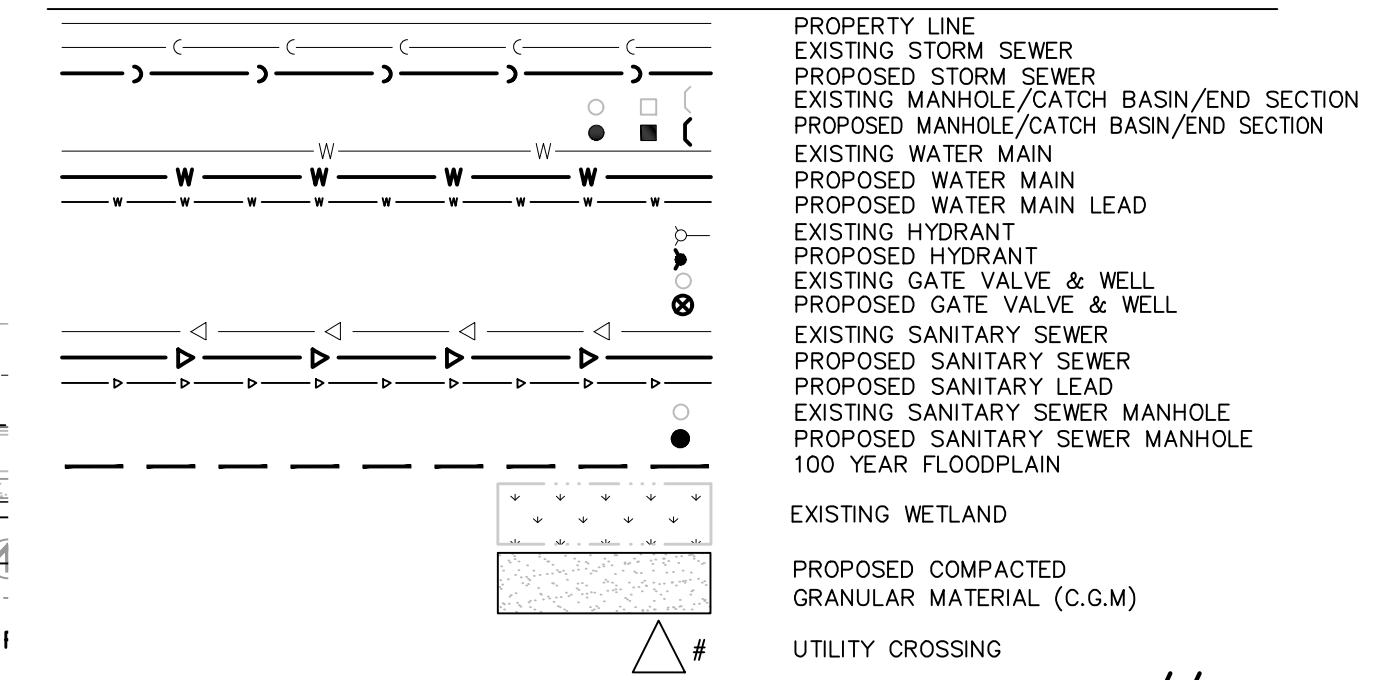


S6-S1
VERT: 1"=5'
HORIZ: 1"=50'

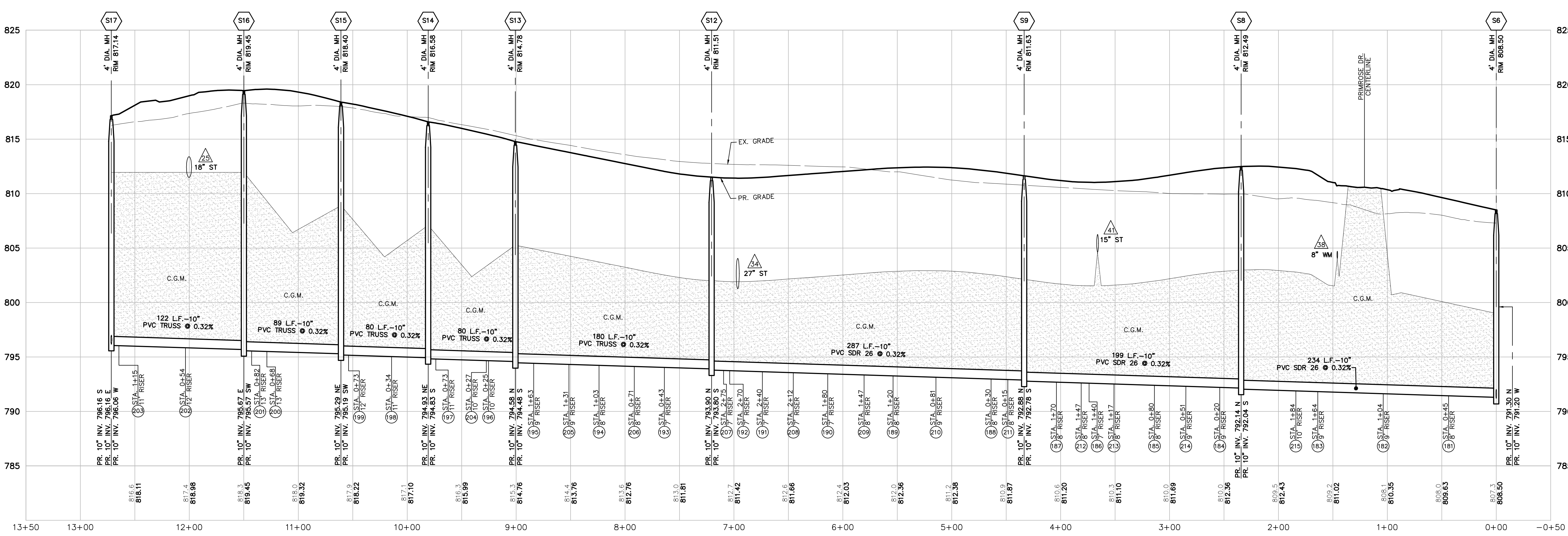
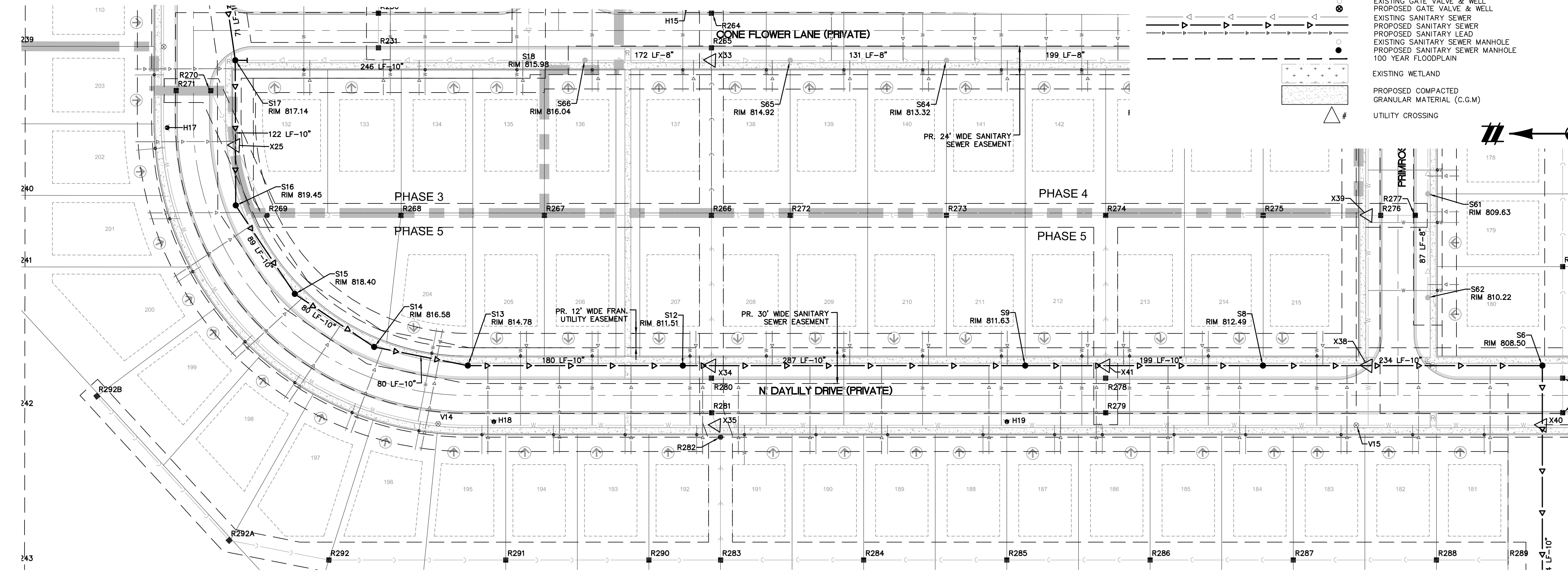
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LEGEND



KEY MAP
NOT TO SCALE



S17-S6
VERT: 1" = 5'
HORIZ: 1" = 50'

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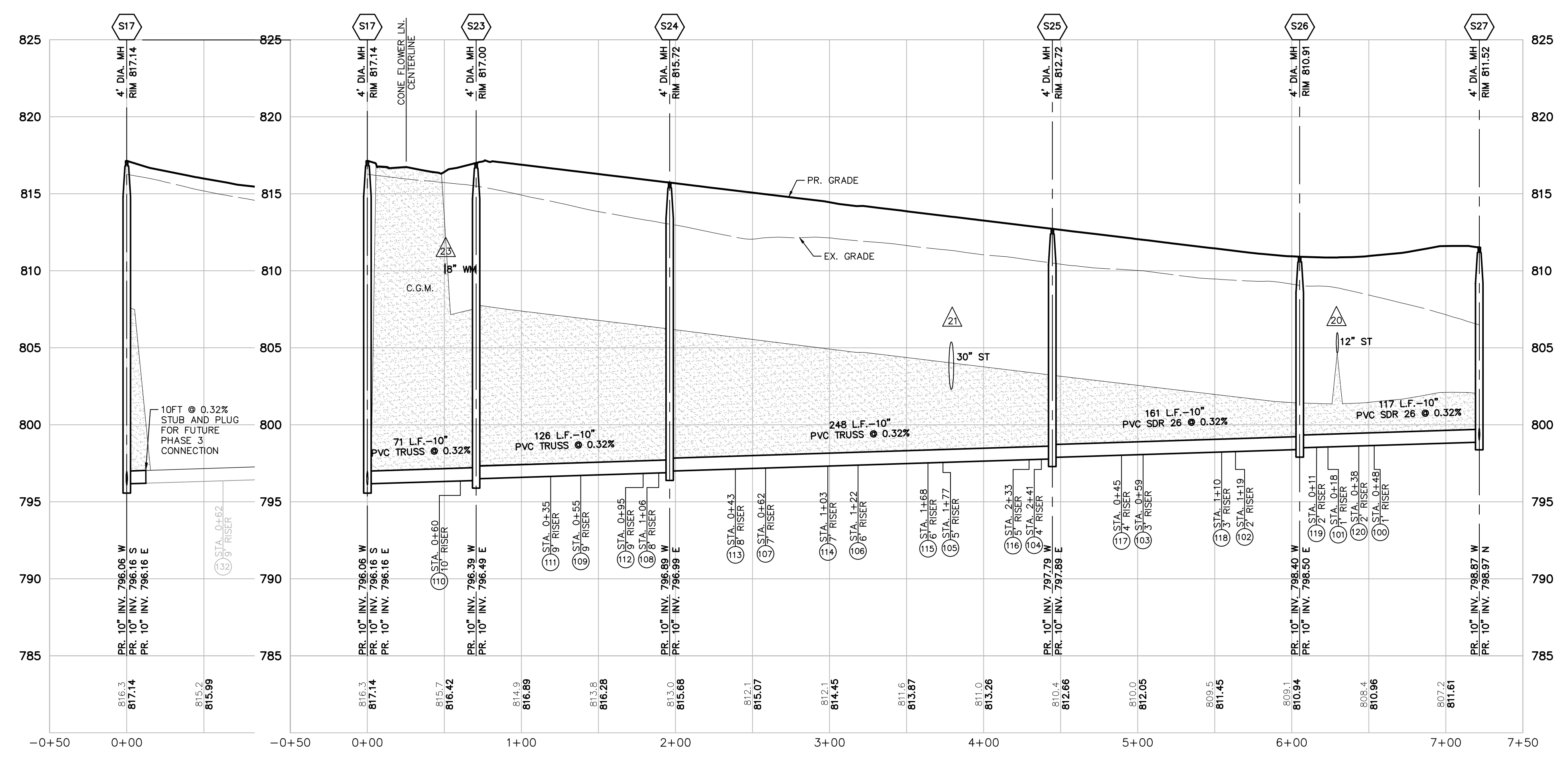
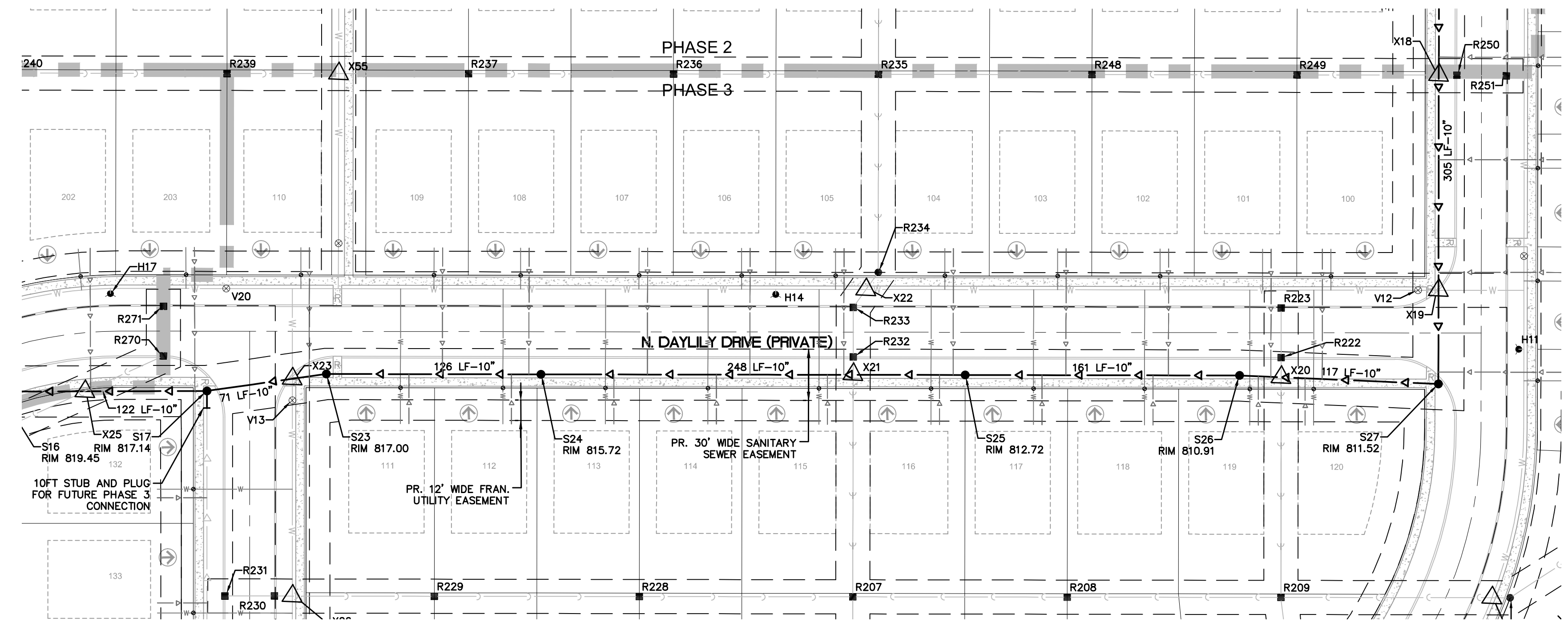
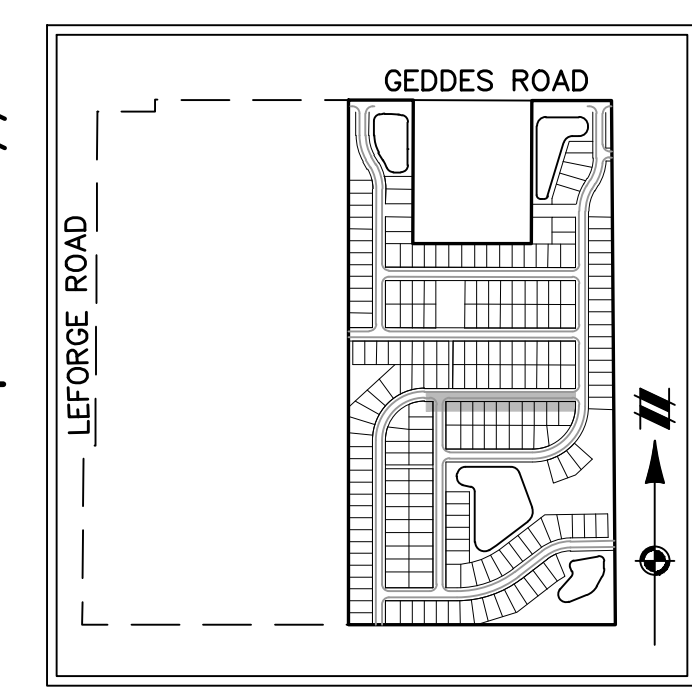
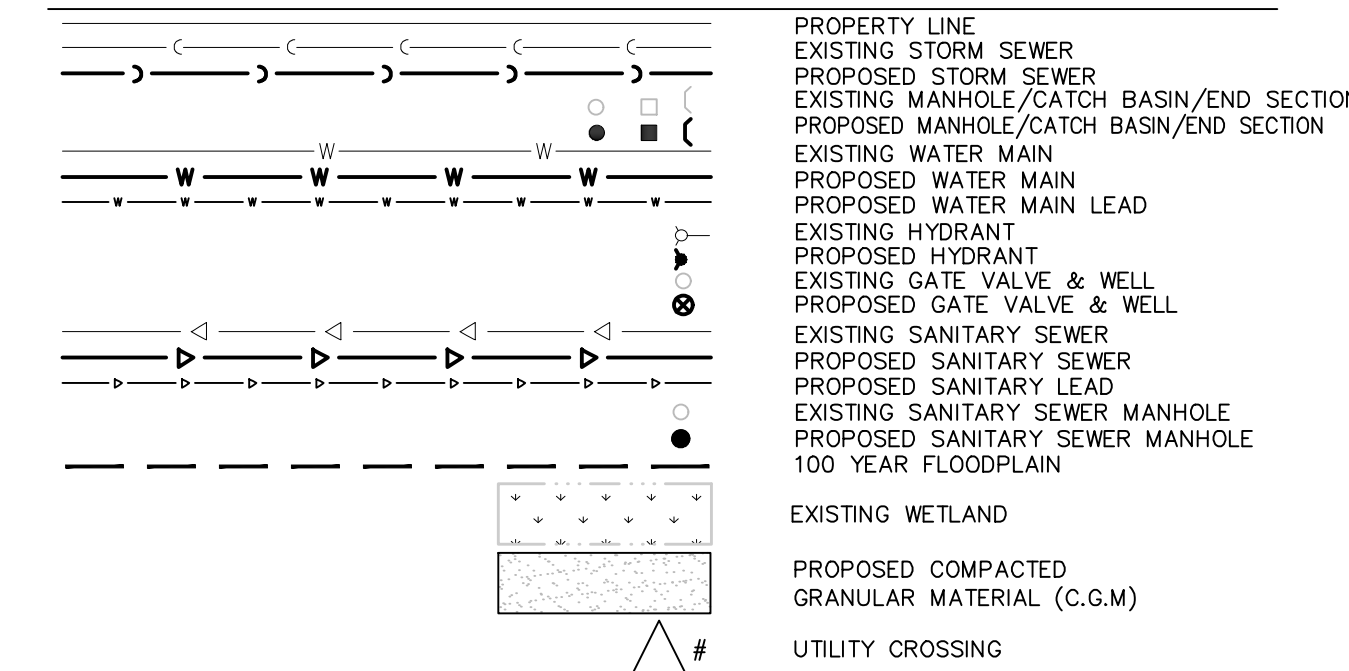
EYDE COMPANY
THE MEADOWS AT HAWTHORNE MILL
FINAL SITE PLANS - PHASE 1
SANITARY SEWER PLAN
& PROFILE

DATE
OCT. 12, 2023

Table with 2 columns: REVISIONS, and empty rows for revision notes.

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P.M.: J. KIME
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LEGEND



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FINAL SITE PLANS - PHASE 1
SANITARY SEWER PLAN
& PROFILE

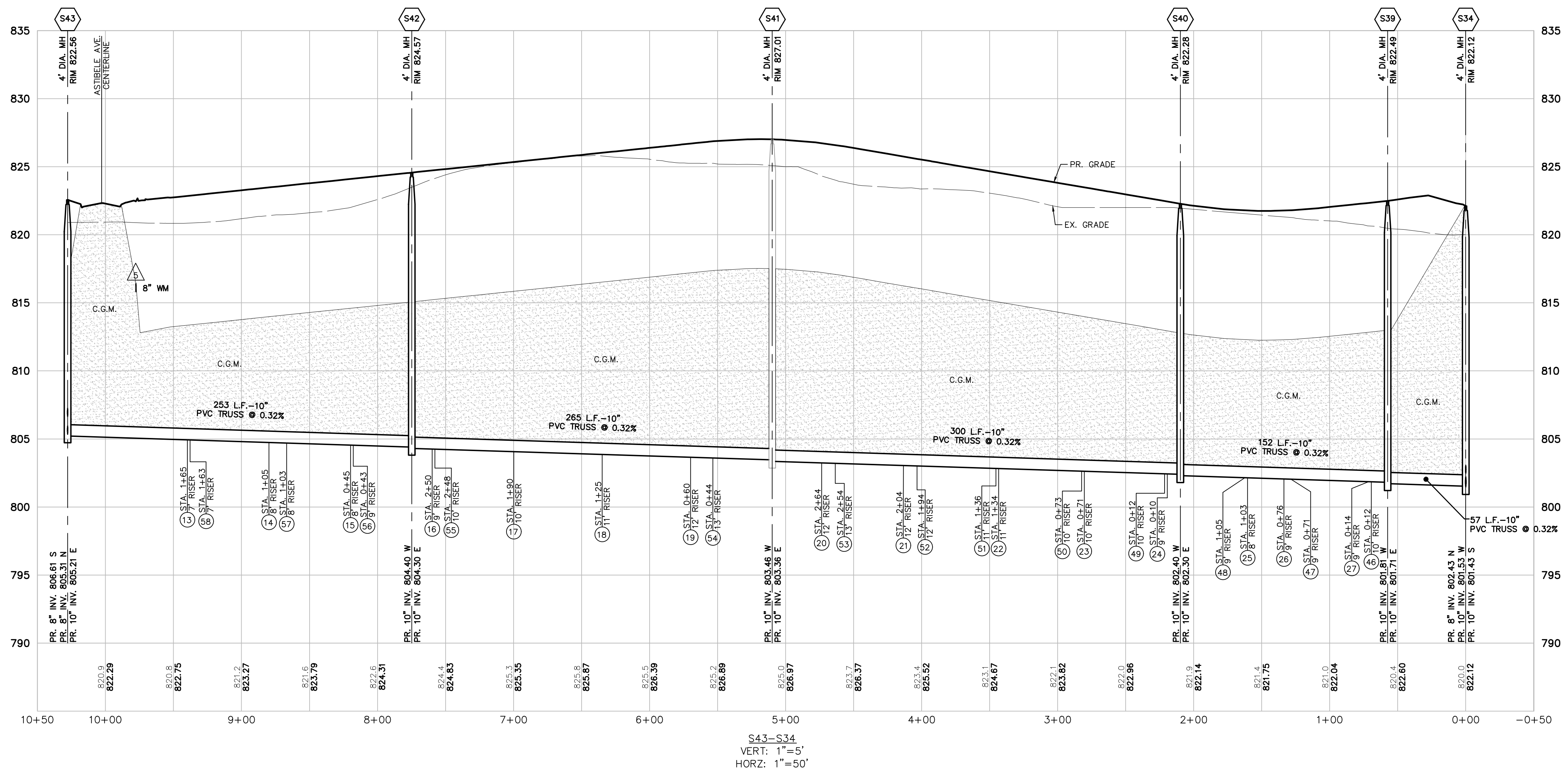
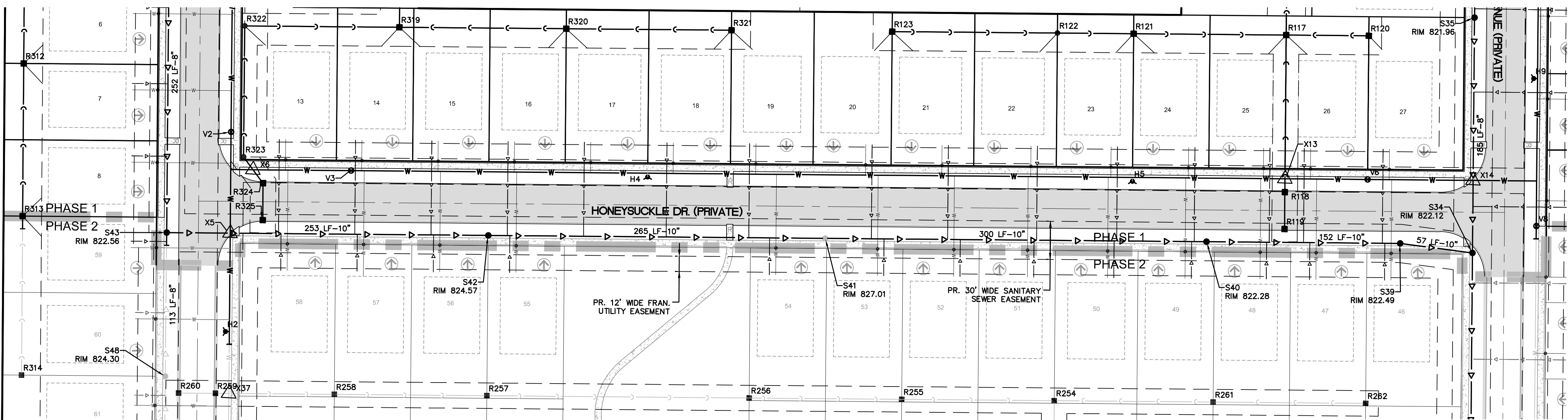
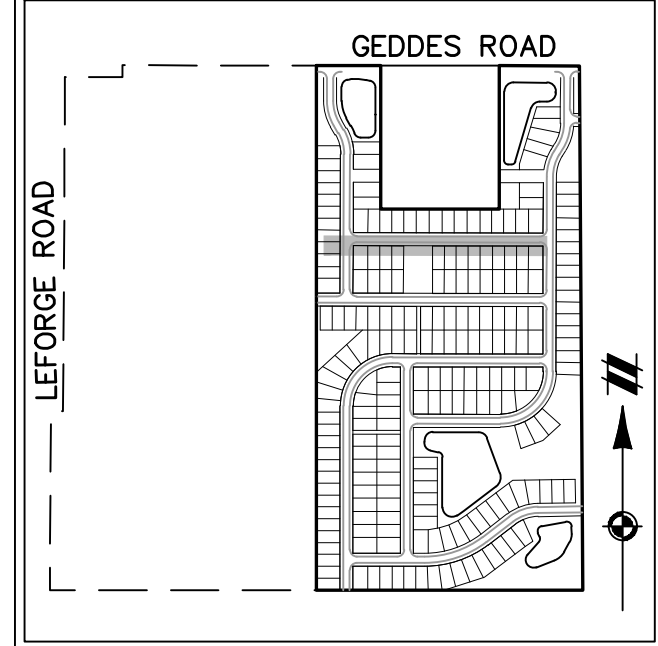
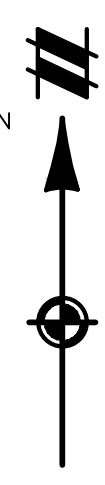
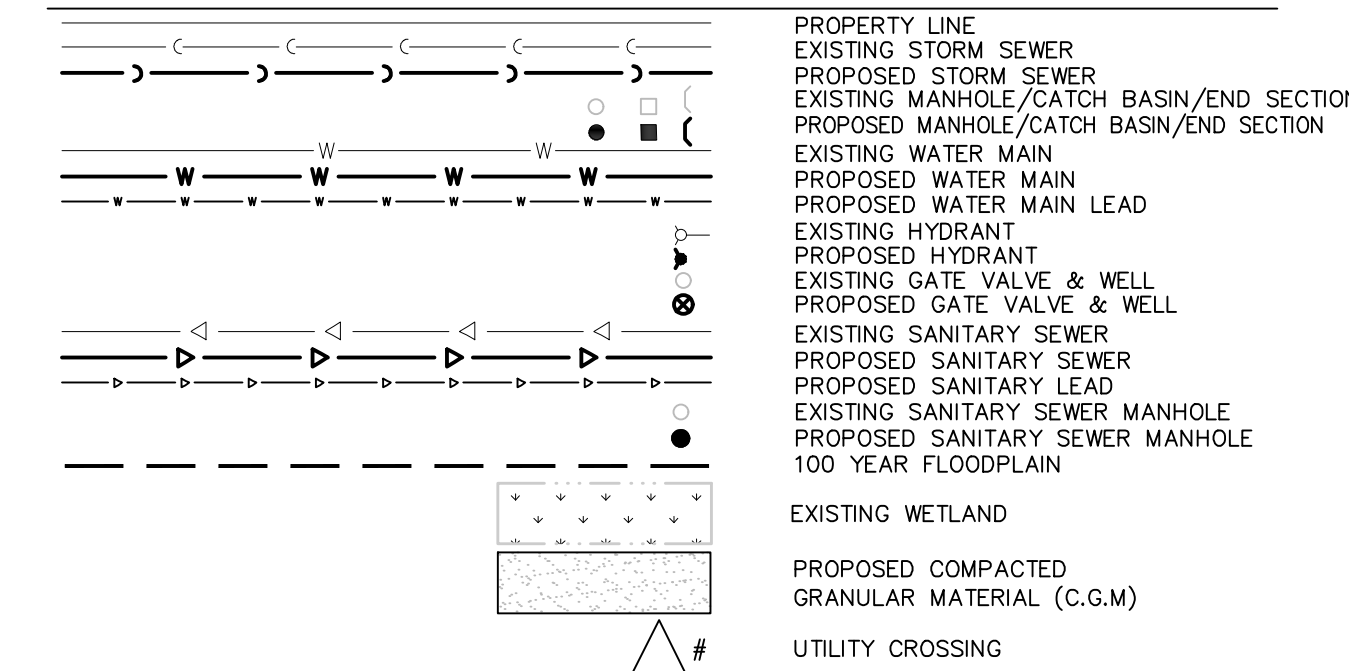
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LEGEND



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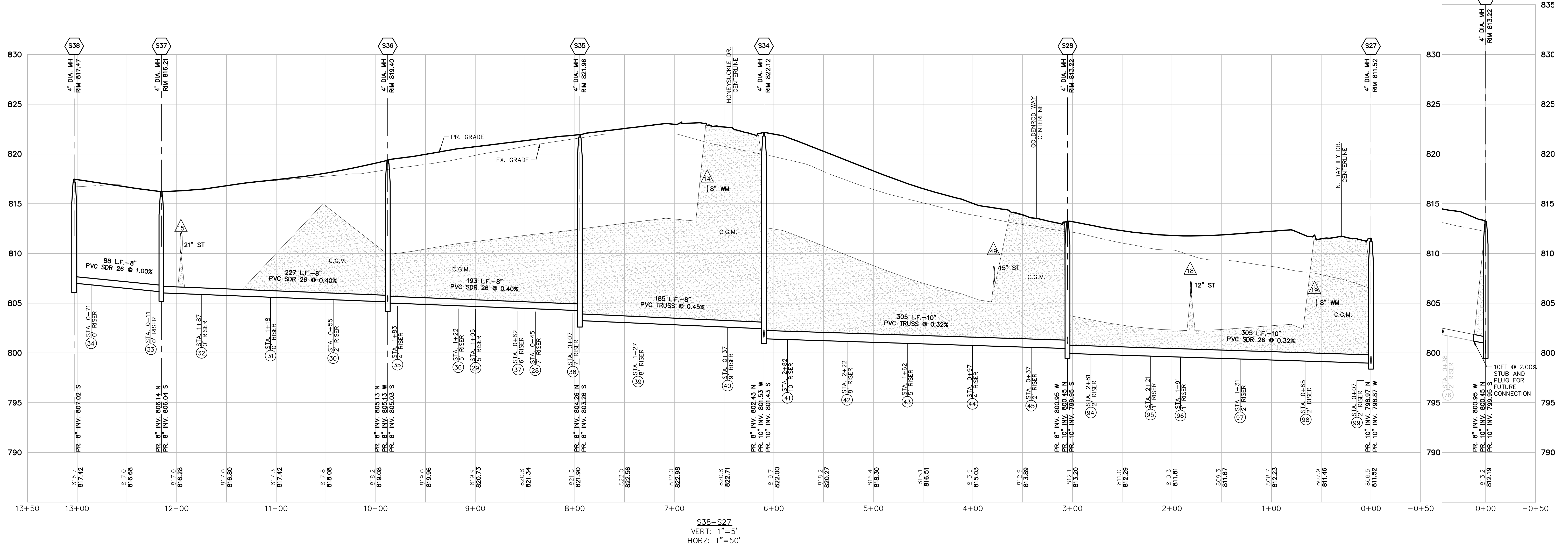
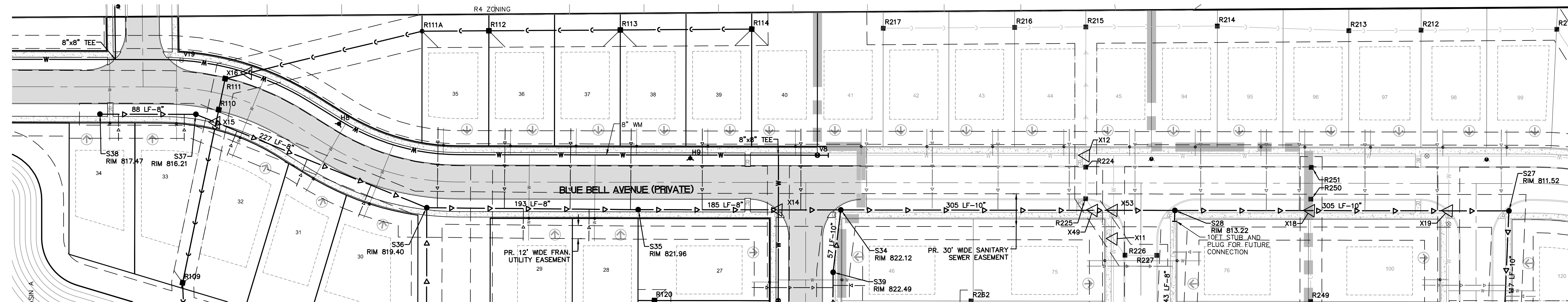
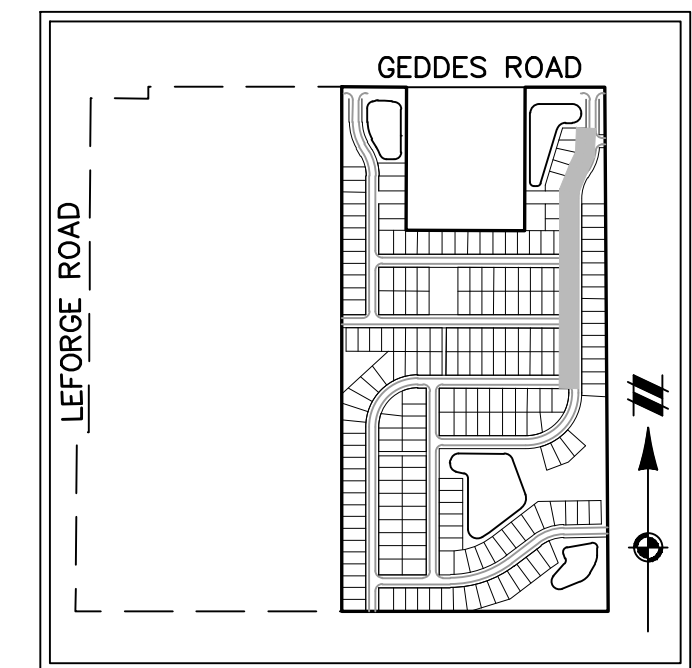
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SANITARY SEWER PLAN
& PROFILE

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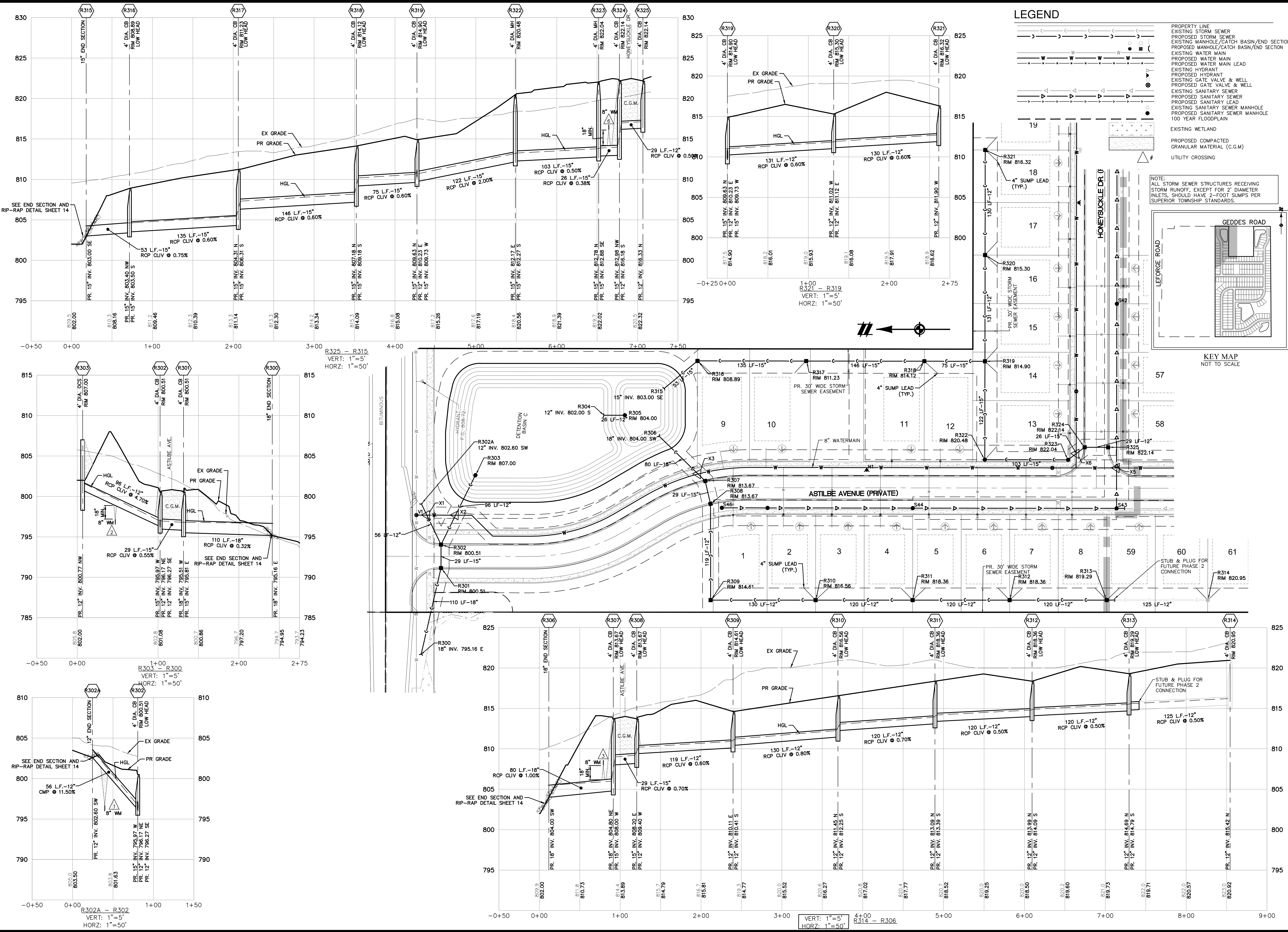
LEGEND

- PROPERTY LINE
- EXISTING STORM SEWER
- PROPOSED STORM SEWER
- EXISTING MANHOLE/CATCH BASIN/END SECTION
- PROPOSED MANHOLE/CATCH BASIN/END SECTION
- EXISTING WATER MAIN
- PROPOSED WATER MAIN
- PROPOSED WATER MAIN LEAD
- EXISTING HYDRANT
- PROPOSED HYDRANT
- EXISTING GATE VALVE & WELL
- PROPOSED GATE VALVE & WELL
- EXISTING SANITARY SEWER
- PROPOSED SANITARY SEWER
- PROPOSED SANITARY LEAD
- EXISTING SANITARY SEWER MANHOLE
- PROPOSED SANITARY SEWER MANHOLE
- 100 YEAR FLOODPLAIN
- EXISTING WETLAND
- PROPOSED COMPACTED GRANULAR MATERIAL (C.G.M.)
- UTILITY CROSSING



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LEGEND

- EXISTING STORM SEWER
- EXISTING MANHOLE/CATCH BASIN/END SECTION
- PROPOSED MANHOLE/CATCH BASIN/END SECTION
- EXISTING WATER MAIN
- PROPOSED WATER MAIN
- EXISTING HYDRANT
- PROPOSED HYDRANT
- EXISTING GATE VALVE & WELL
- PROPOSED GATE VALVE & WELL
- EXISTING SANITARY SEWER
- PROPOSED SANITARY SEWER
- PROPOSED SANITARY LEAD
- EXISTING SANITARY SEWER MANHOLE
- PROPOSED SANITARY SEWER MANHOLE
- 100 YEAR FLOODPLAIN
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- PROPOSED COMPACTED GRANULAR MATERIAL (C.G.M.)
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EYE COMPANY
THE MEADOWS AT HAWTHORNE MILL
FINAL SITE PLANS - PHASE 1
STORM SEWER PROFILE

DATE: OCT. 12, 2023

REVISIONS

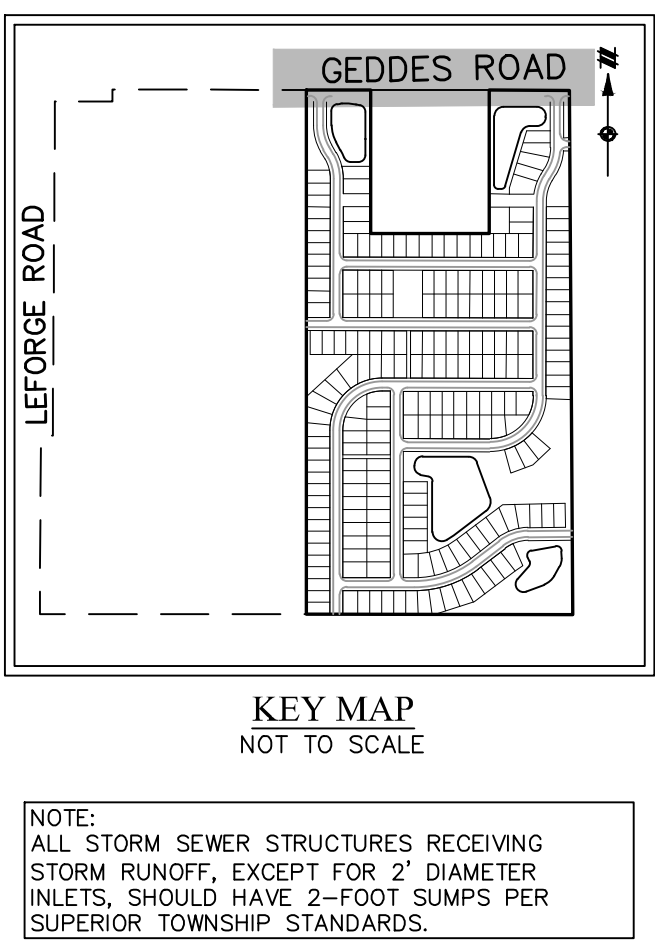
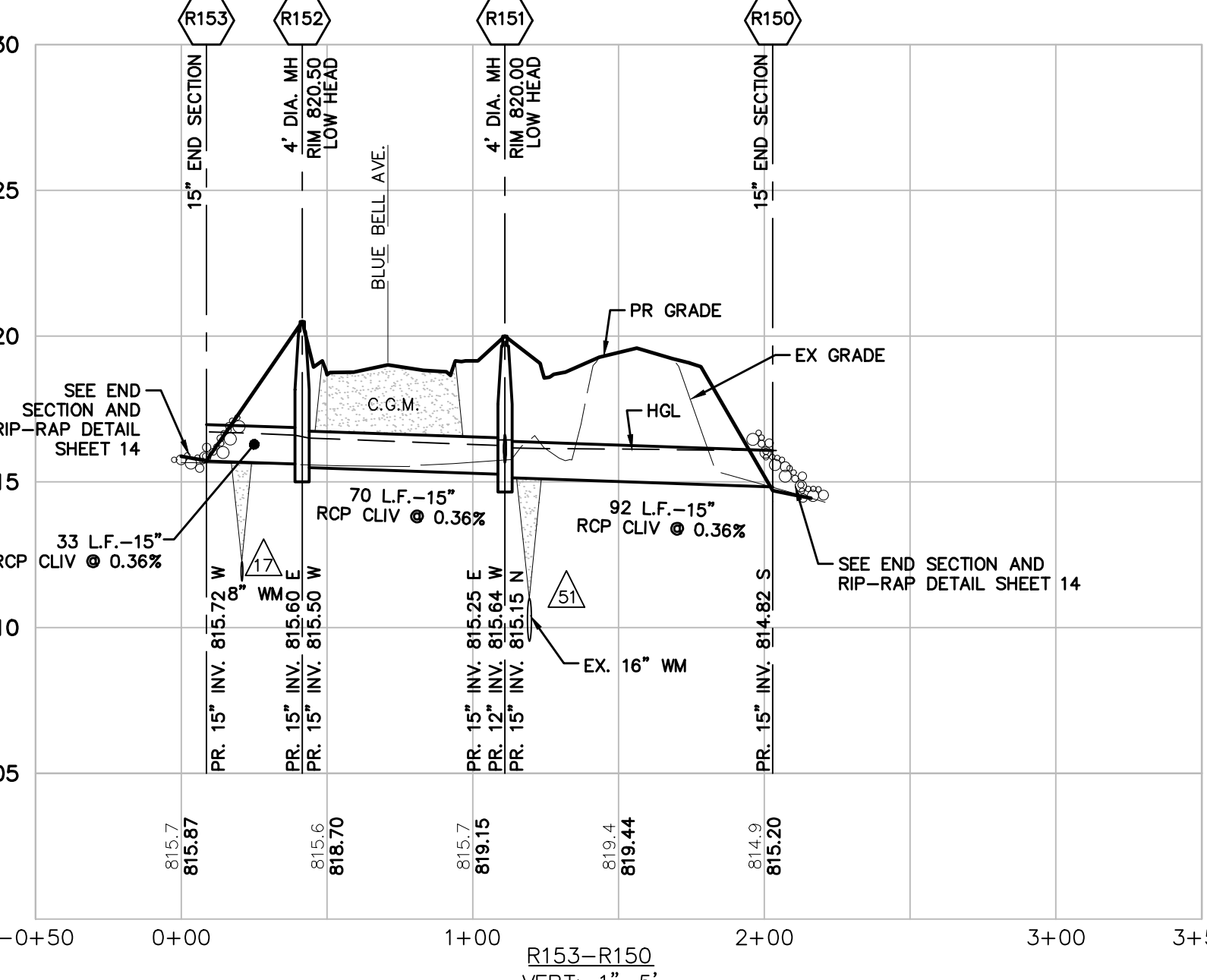
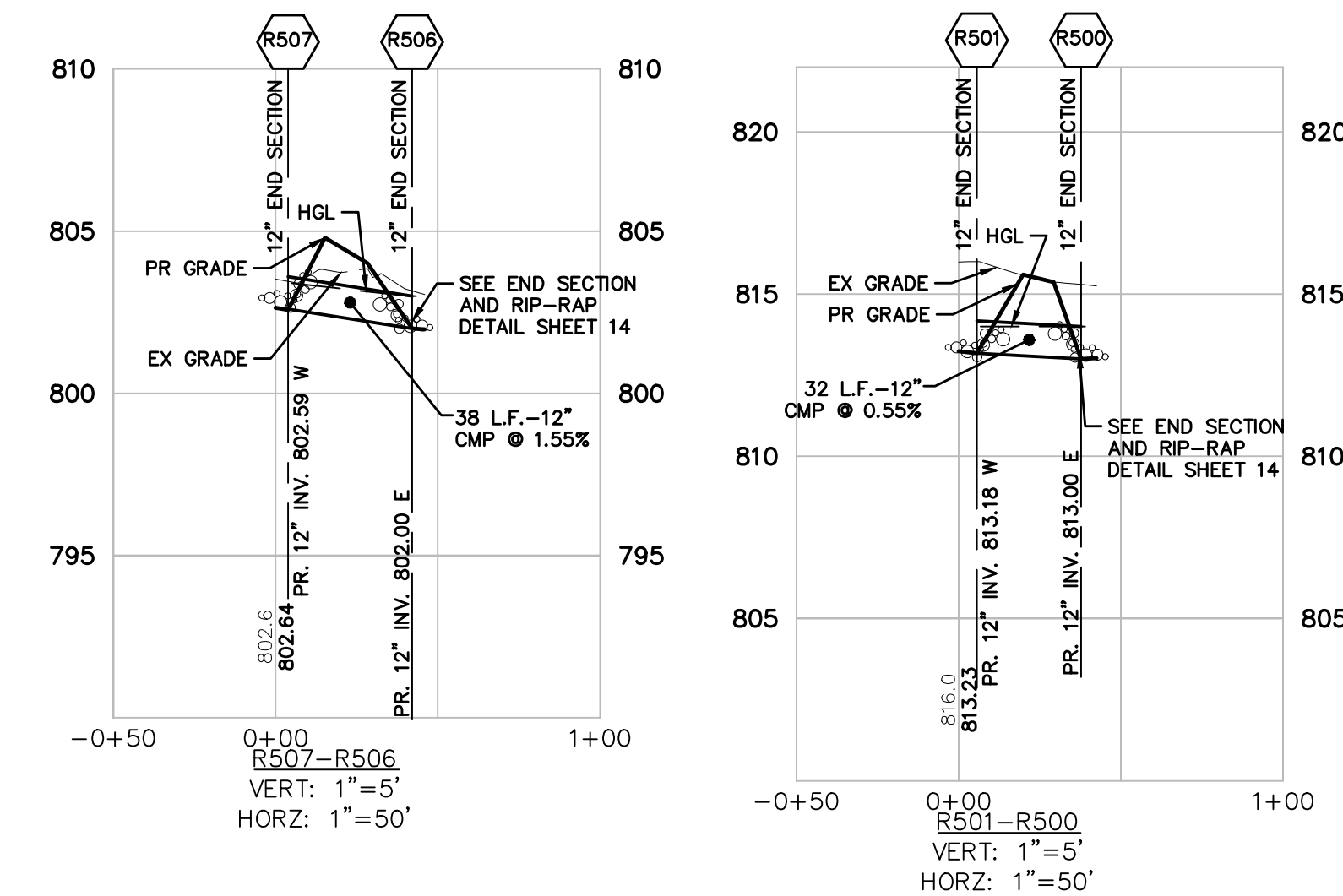
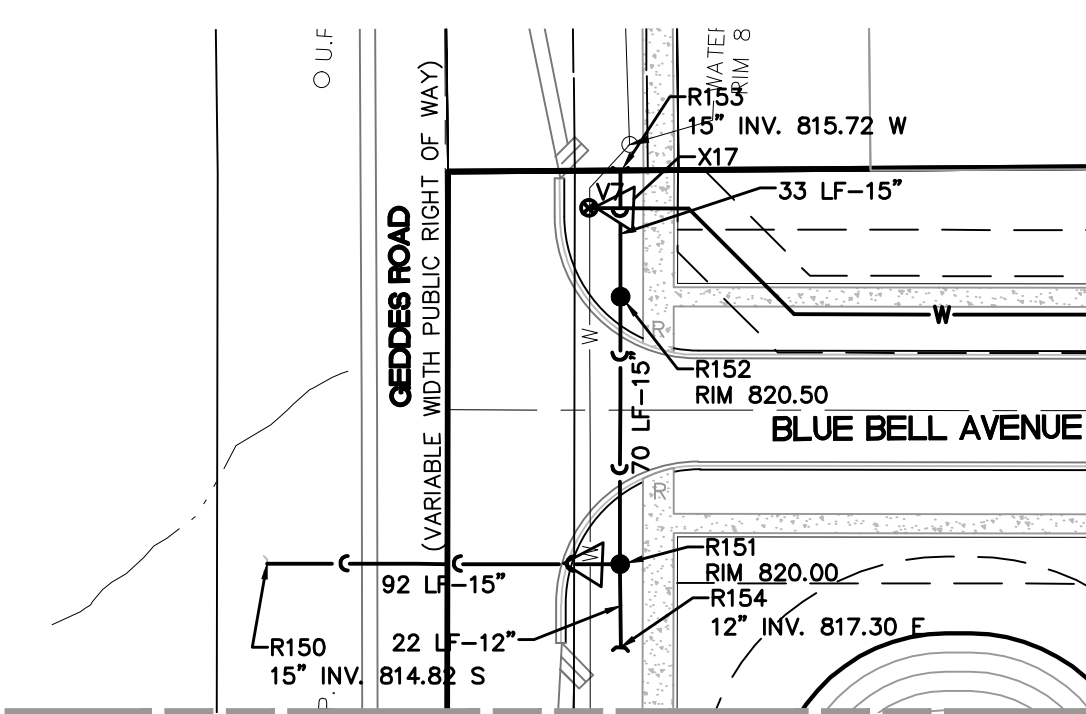
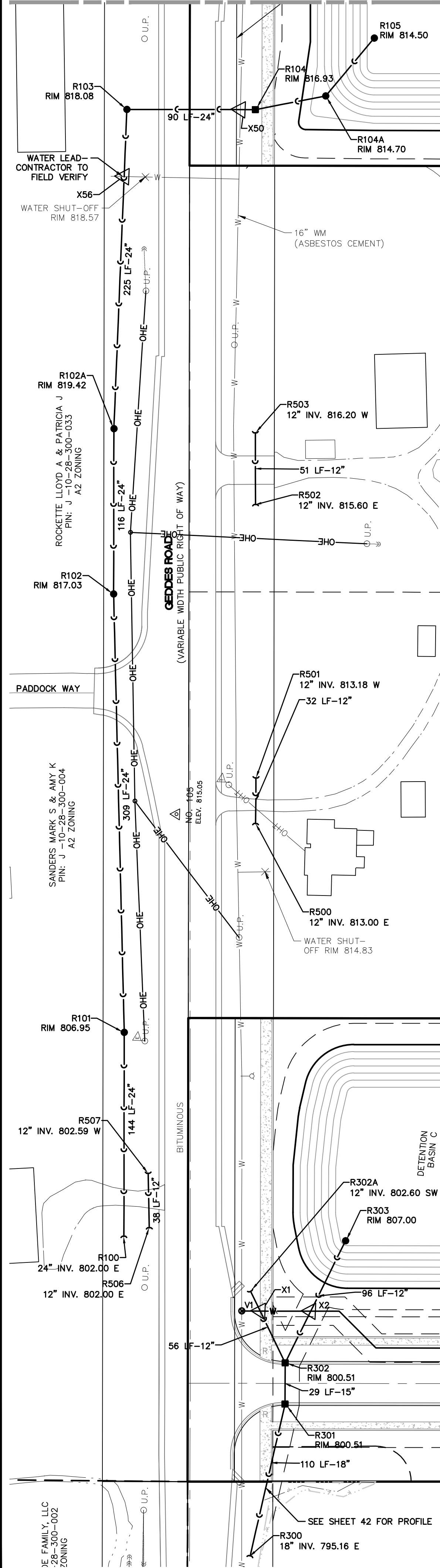
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JOB #: 19004443
FILE CODE: -
SHEET NO. 42

CAD FILE: 19004443\SP-24-R.DWG

SEE THIS SHEET FOR CONTINUATION

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LEGEND

- PROPERTY LINE
- EXISTING STORM SEWER
- EXISTING MANHOLE/CATCH BASIN/END SECTION
- PROPOSED MANHOLE/CATCH BASIN/END SECTION
- PROPOSED WATER MAIN
- EXISTING WATER MAIN LEAD
- EXISTING HYDRANT
- PROPOSED HYDRANT
- EXISTING GATE VALVE & WELL
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- 100 YEAR FLOODPLAIN
- EXISTING WETLAND
- PROPOSED COMPACTED GRANULAR MATERIAL (C.G.M.)
- UTILITY CROSSING

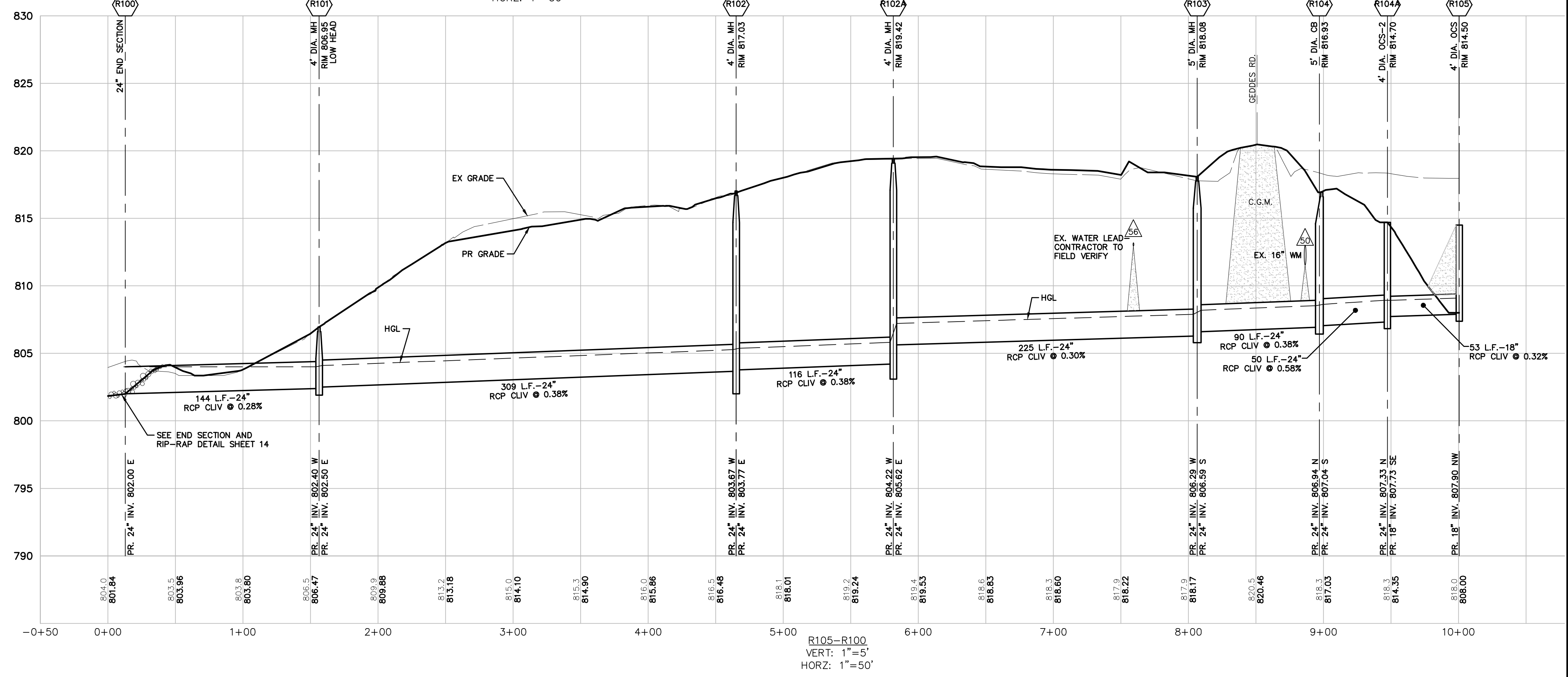
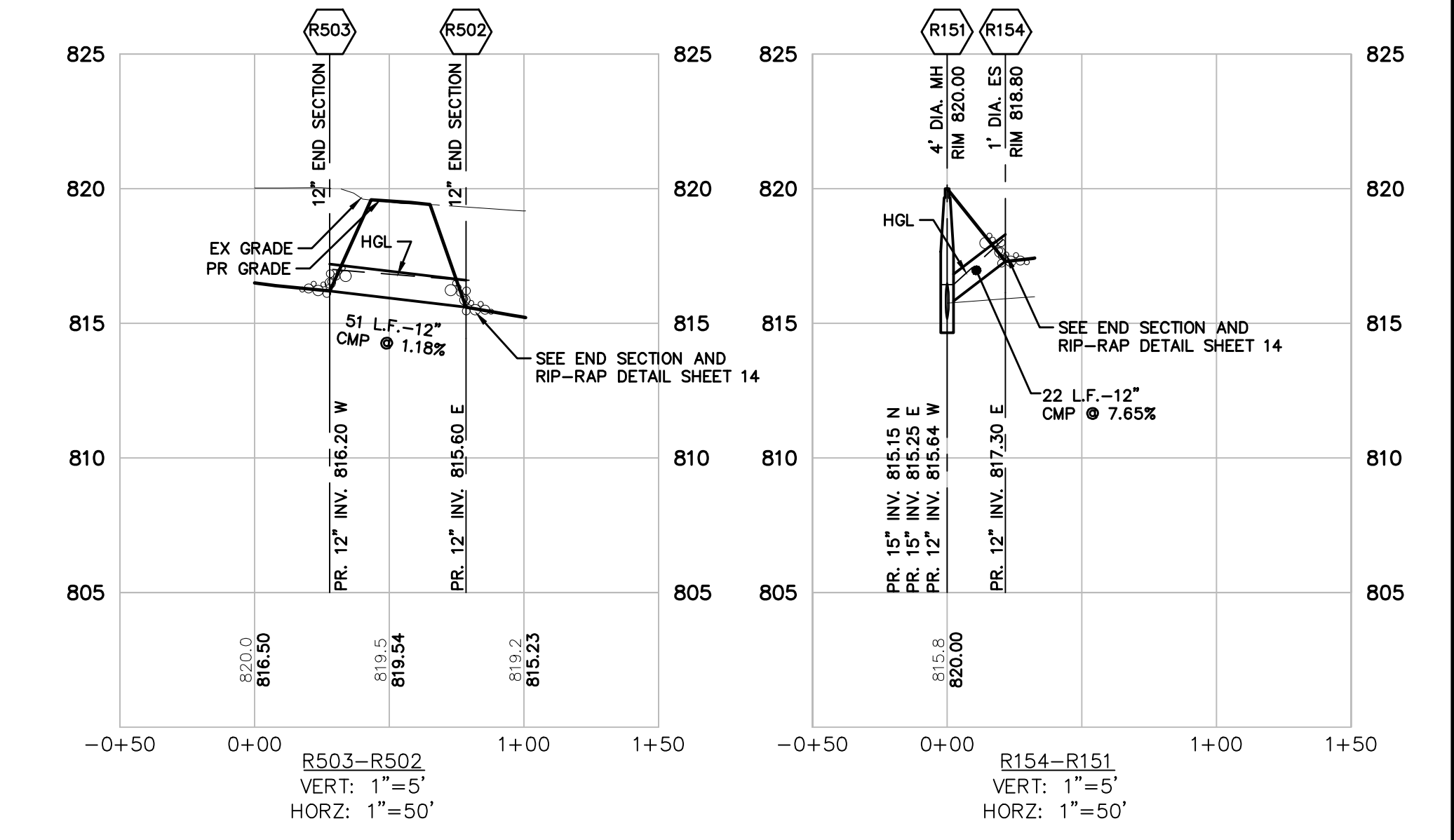
NOTE: ALL STORM SEWER STRUCTURES RECEIVING STORM RUNOFF, EXCEPT FOR 2' DIAMETER INLETS, SHOULD HAVE 2'-FOOT SUMPS PER SUPERIOR TOWNSHIP STANDARDS.

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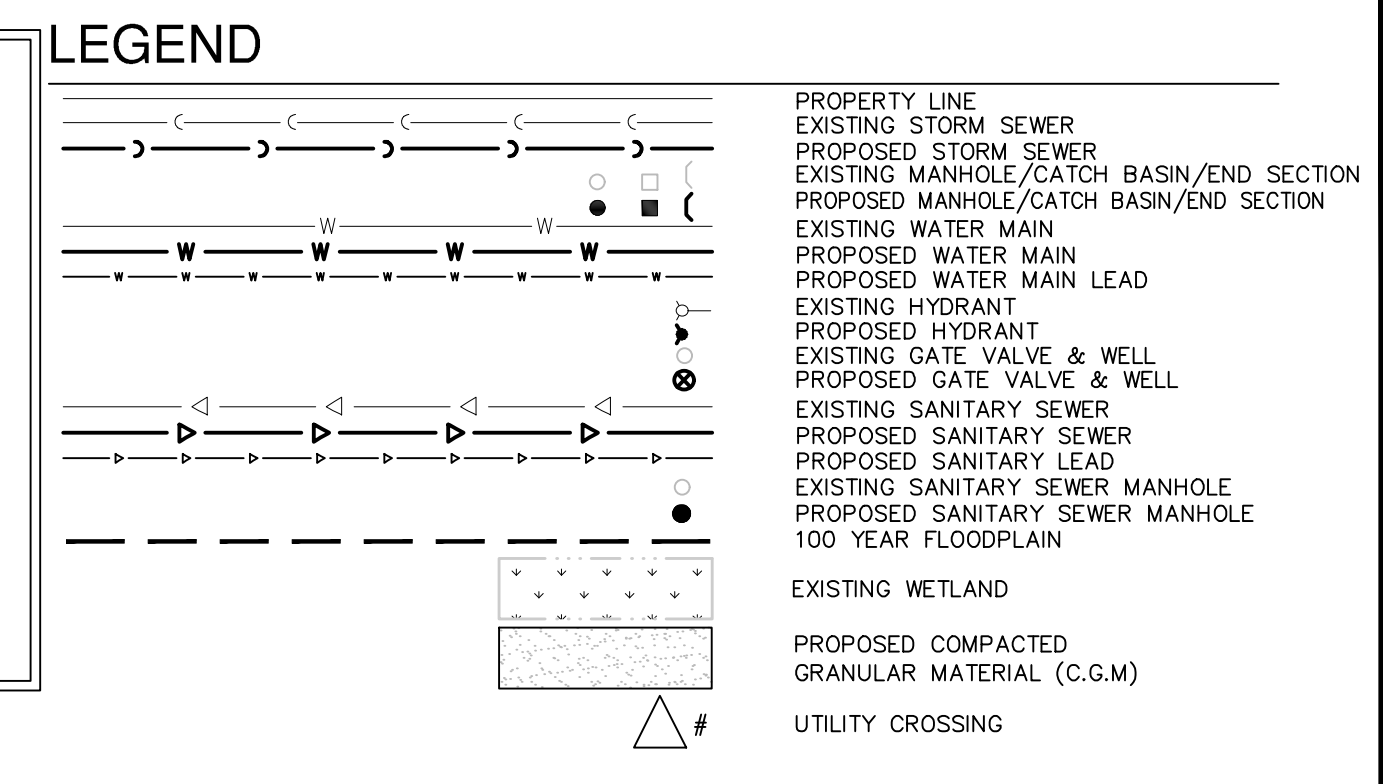
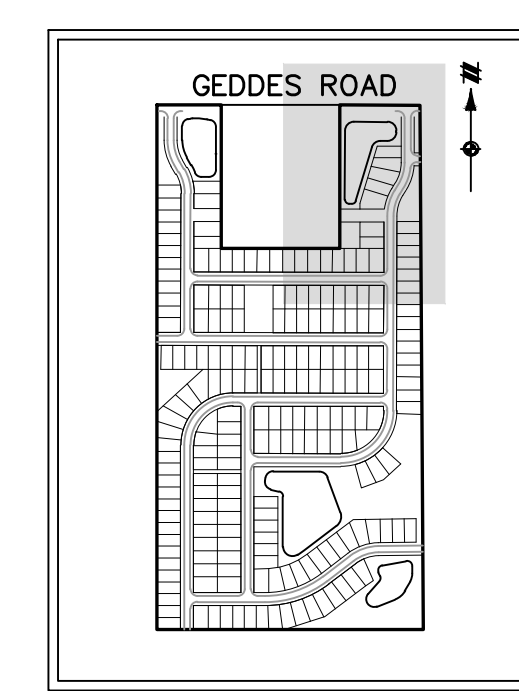
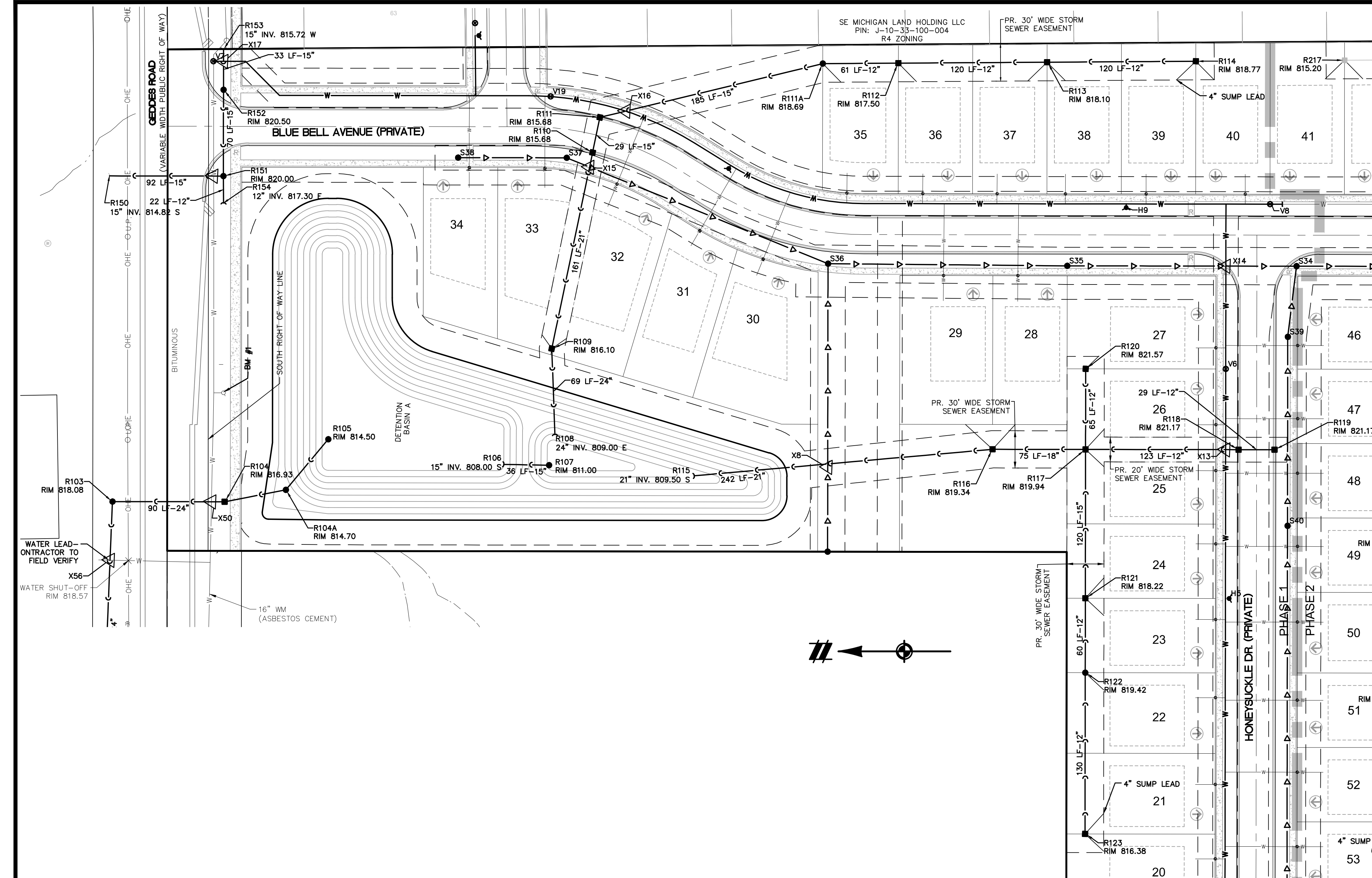
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STORM SEWER PLAN & PROFILE

DATE: OCT. 12, 2023

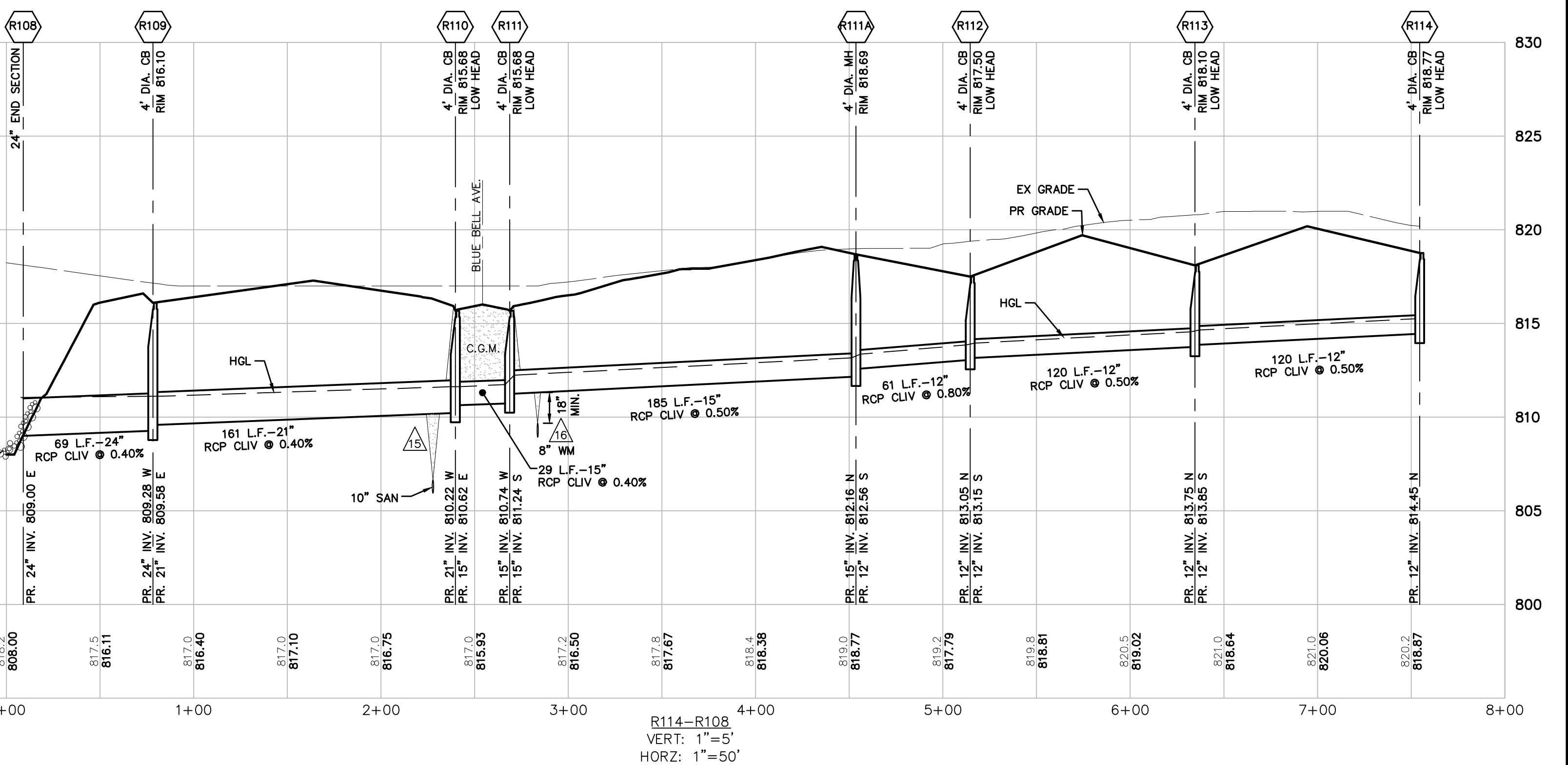
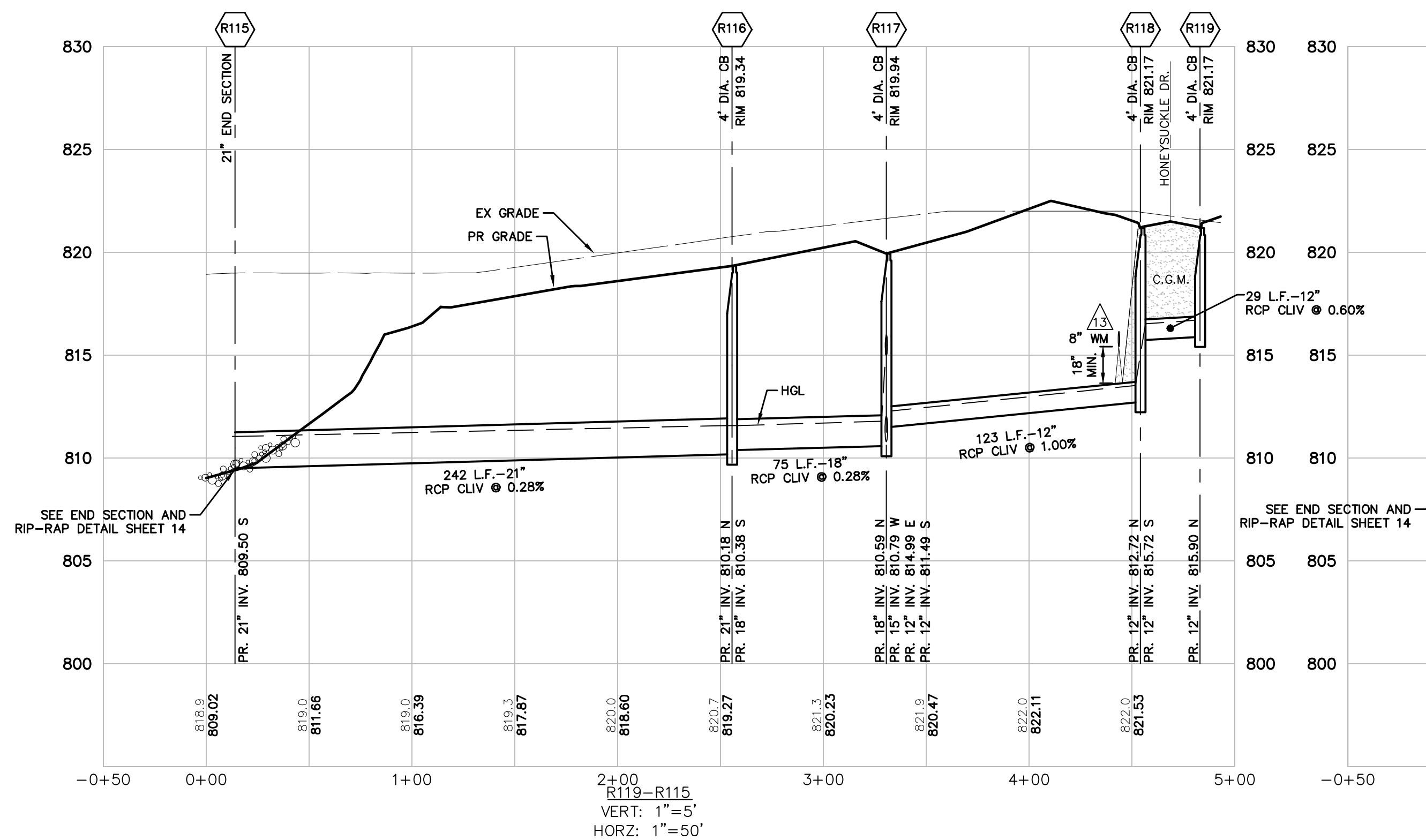
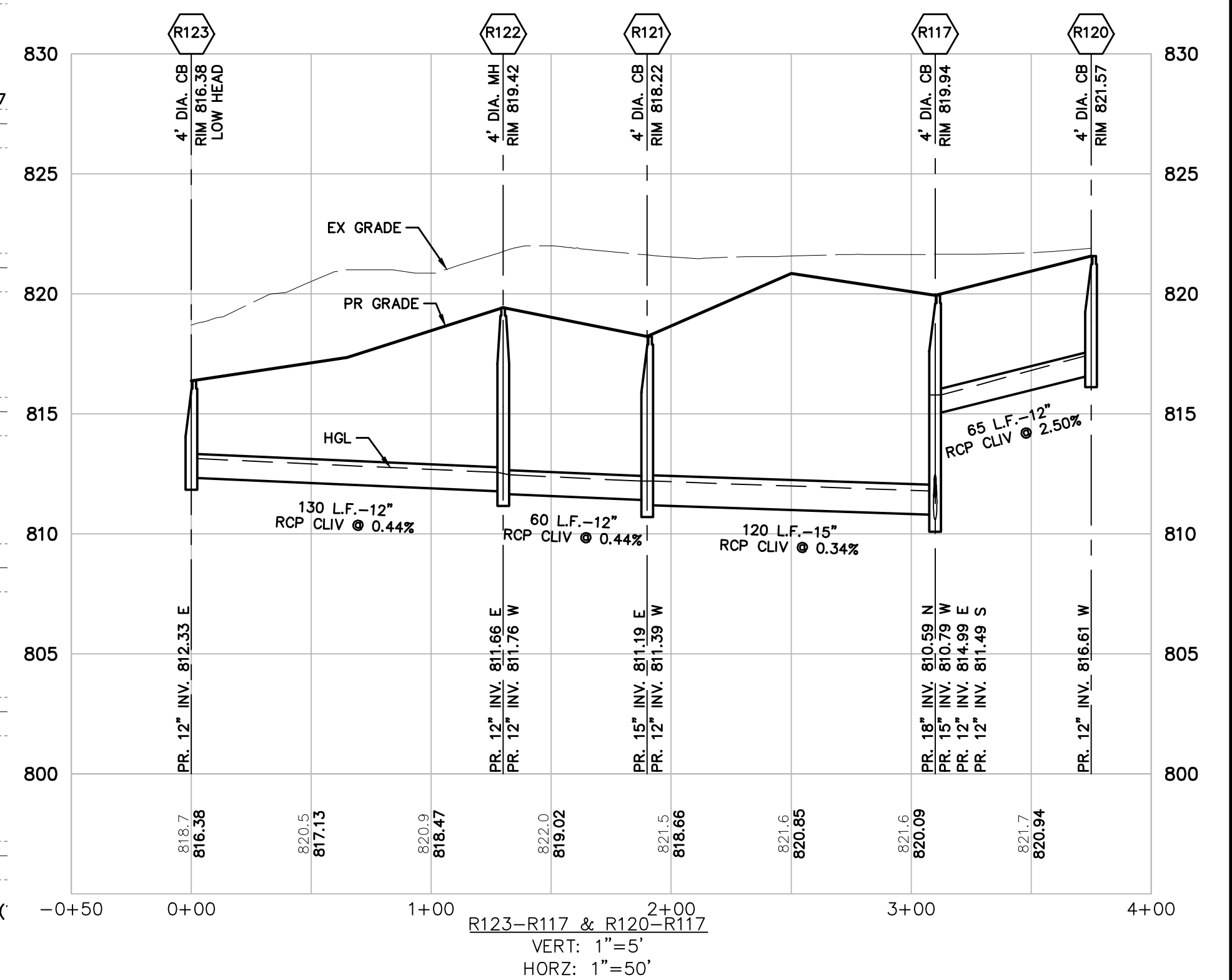
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JOB #: 19004443
FILE CODE: -
SHEET NO. 43

CAD FILE: 19004443SP-24-RDWS



NOTE:
ALL STORM SEWER STRUCTURES RECEIVING STORM RUNOFF,
EXCEPT FOR 2' DIAMETER INLETS, SHOULD HAVE 2-FOOT
SUMPS PER SUPERIOR TOWNSHIP STANDARDS.



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WASHTENAW COUNTY, MICHIGAN

EYE COMPANY

THE MEADOWS AT HAWTHORNE MILL

FINAL SITE PLANS - PHASE 1

STORM SEWER PLAN & PROFILE

DATE

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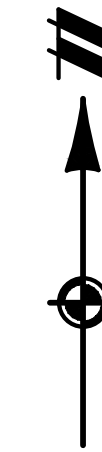
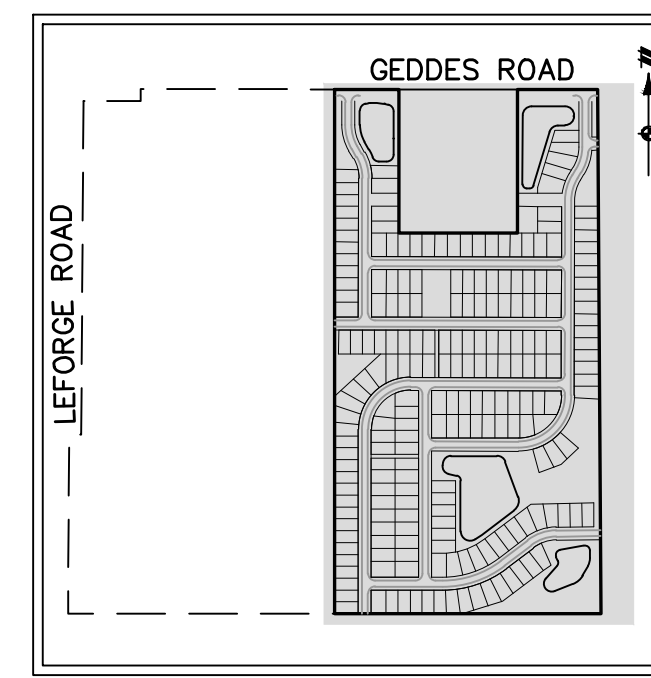
FILE CODE: -

SHEET NO. 44

CAD FILE: 19004443SP-24-RDWS

NOTES

1. TEST PITS FOR SOIL INFILTRATION TESTING WERE PERFORMED IN MAY, 2020. OF THE TWELVE TEST LOCATIONS, ONLY TP-10 CONTAINED SOILS SUITABLE FOR INFILTRATION TESTING. DUE TO THE SHALLOW DEPTH OF THE CLAYEY SAND/GRAVEL LAYER AND RELATIVELY HIGH ELEVATION OF THIS TEST LOCATION IN RELATION TO THE SURROUNDING SITE AREAS, IT WAS DETERMINED THAT THIS LOCATION WAS NOT VIALBE TO SERVE THE INFILTRATION REQUIREMENTS OF THE SITE. THEREFORE, THE PROPOSED BASINS HAVE BEEN DESIGNED WITH A TWENTY PERCENT (20%) VOLUME PENALTY, IN ACCORDANCE WITH WCMRC RULES.
2. THE DETENTION BASIN OUTLETS WILL ULTIMATELY DISCHARGE TO THE SNIDECAR DRAIN AND THE SUPERIOR NO. 1 DRAIN, WHICH FLOW SOUTH AND ULTIMATELY DISCHARGES INTO THE HURON RIVER, APPROXIMATELY 1.5 MILES SOUTHWEST OF THE PROJECT.
3. OFFSITE 1 DRAINAGE AREA IS TO DRAIN UNDETAINED DUE TO ROADWAY BEING AT OR BELOW THE POND ELEVATIONS.
4. AN EOLE PERMIT HAS BEEN ISSUED FOR THE REGULATED WETLAND IMPACTS.



LEGEND

- PROPERTY LINE
- PROPOSED STORM SEWER
- PROPOSED MANHOLE/CATCH BASIN/END SECTION
- PROPOSED DRAINAGE AREA

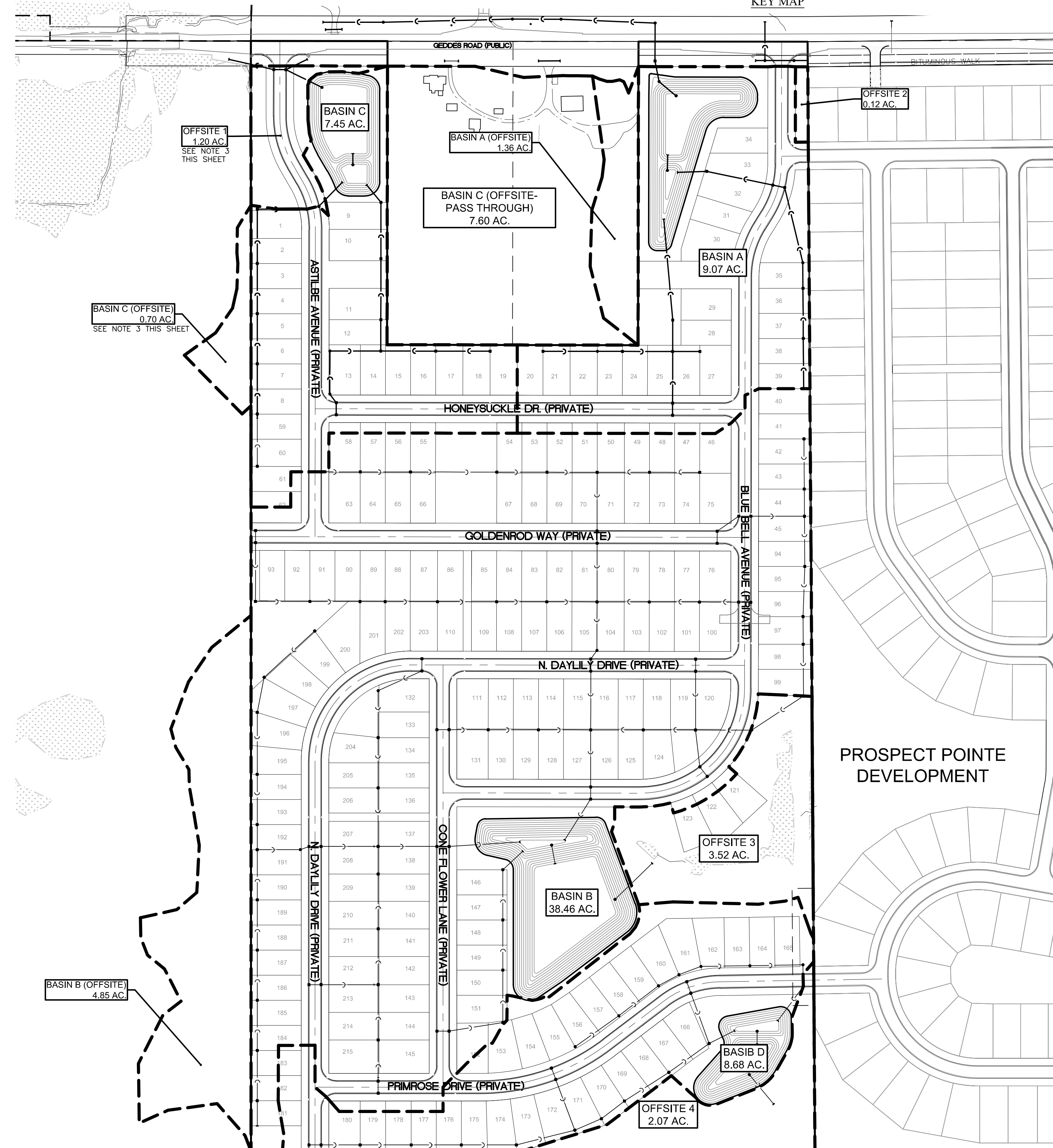


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PROSPECT POINTE DEVELOPMENT

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WASHTEENAW COUNTY, MICHIGAN

EYDE COMPANY
THE MEADOWS AT HAWTHORNE MILL
FINAL SITE PLANS - PHASE 1
DETENTION TRIBUTARY
DRAINAGE AREA PLAN

DATE
OCT. 12, 2023

REVISIONS
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JOB #: 19004443
FILE CODE: -
SHEET NO. 46

CAD FILE: 19004443\SP-15-DET-DA.DWG

DETENTION BASIN A

Project: The Meadows at Hawthorne Mill Date: 10/10/2023
 Location: Superior Township By: Atwell
 Description: Detention Basin A

W1 Determining Post-Development Cover Types, Areas, Curve Numbers and runoff coefficients

Cover Type	Soil Type	Area (sf)	Area (ac)	Curve Number	CN(Area)
Paved Parking Lots, roofs, driveways	D	116,580	2.68	0.95	110,751
Water Surfaces	D	49,000	1.12	1.00	49,000
Developed Open Space, Good Condition	D	228,509	5.27	0.45	103,379
Off-site Contributing Area	D	59,242	1.36	0.45	26,659
Total - Sum (C) (Area) =		289,689			143,333
Area Total (sf) =		454,333			
Weighted C-Sum (C)(Area)/Sum(C) =					0.64

N1C3 Variables

Previous Cover Type	Soil Type	Area (sf)	Area (ac)	Curve Number	CN(Area)
Developed Open Space, Good Condition	D	228,509	5.27	0.45	103,379
Off-site Contributing Area	D	59,242	1.36	0.45	26,659
Total - Sum (C)(Area) =		289,689			143,333
Area Total (sf) =		454,333			
Weighted CN-Sum (C)(Area)/Sum(C) =					0.64

N1C3 Variables

Impervious Cover Type	Soil Type	Area (sf)	Area (ac)	Curve Number	CN(Area)
Paved Parking Lots, roofs, driveways	D	116,580	2.68	98	114,348
Impervious, Ponds	D	49,000	1.12	98	48,022,000
Total - Sum (C)(Area) =		165,580			162,370
Area Total - Sum (C)(Area) =		165,580			165,580
Weighted CN-Sum (C)(Area)/Sum(C) =					98

W2 First Flush Runoff Calculations (Vf)

Vf = (1/T) [(1/2) (35560/T) (C) Ac]	24,231	cf
-------------------------------------	--------	----

W3 Predevelopment Bankfull Runoff Calculations (Vf-pre)

A. 2 year/24 hour storm event	P = 2.35	in
B. Previous Cover CN	CN = 78	
C. S = (1000/CN) - 10	S = 2.82	in
D. Q = (P - 0.25) / (P - 0.85)	Q = 0.69	in
E. Previous Cover Area	Area = 494,331	sf
F. V _{100-pre} = Q(1/2)Area	V _{100-pre} = 24,231	cf

W4 Previous Cover Post-development Bankfull Runoff Calculations (Vf-post)

A. 2 year/24 hour storm event	P = 2.35	in
B. Previous Cover CN	CN = 80	
C. S = (1000/CN) - 10	S = 2.50	in
D. Q = (P - 0.25) / (P - 0.85)	Q = 0.79	in
E. Previous Cover Area	Area = 288,751	sf
F. V _{100-post} = Q(1/2)Area	V _{100-post} = 18,912	cf

W5 Impervious Cover Post-development Bankfull Runoff Calculations (Vf-imp-post)

A. 2 year/24 hour storm event	P = 2.35	in
B. Previous Cover CN	CN = 98	
C. S = (1000/CN) - 10	S = 0.20	in
D. Q = (P - 0.25) / (P - 0.85)	Q = 2.12	in
E. Previous Cover Area	Area = 165,580	sf
F. V _{100-imp-post} = Q(1/2)Area	V _{100-imp-post} = 29,276	cf

W6 Previous Cover Post-development 100-year Storm Runoff Calculations (Vf-100-post)

A. 100 year storm event	P = 5.11	in
B. Previous Cover CN	CN = 80	
C. S = (1000/CN) - 10	S = 2.50	in
D. Q = (P - 0.25) / (P - 0.85)	Q = 2.99	in
E. Previous Cover Area	Area = 288,751	sf
F. V _{100-post} = Q(1/2)Area	V _{100-post} = 71,924	cf

W7 Impervious Cover Post-development 100-year Storm Runoff Calculations (Vf-100-imp-post)

A. 100 year storm event	P = 5.11	in
B. Previous Cover CN	CN = 98	
C. S = (1000/CN) - 10	S = 0.20	in
D. Q = (P - 0.25) / (P - 0.85)	Q = 4.87	in
E. Previous Cover Area	Area = 165,580	sf
F. V _{100-imp-post} = Q(1/2)Area	V _{100-imp-post} = 67,239	cf

W8 Determine Time of Concentration (Tc-hrs)

User specified, assume 30 minutes

Total Time of Concentration (hrs) = **0.50**

W9 Runoff Summary & Onsite Infiltration Requirement

Runoff Summary from Previous Worksheets	V ₁₀₀ = 24,231	cf*
V _{100-pre}	26,214	cf
V _{100-post}	18,912	cf
V _{100-imp-post}	29,276	cf
Total BF Volume (V₁₀₀)	48,208	cf
V _{100-pre}	71,924	cf
V _{100-post}	67,239	cf
Total 100-year Volume (V₁₀₀)	139,163	cf

Determine Onsite Infiltration Requirement

V _{100-pre}	48,208	cf
V _{100-post}	26,214	cf
Bankfull Volume Difference	21,993	cf*

Onsite Infiltration Requirement (V₁₀₀) = **24,231** cf

W10 Detention / Retention Requirement

A. Q _p = 238.6 (T _c) ^{-0.82}	421.23	cf/in-mi ²
B. Total Site Area	10.43	ac
C. Q _p = Q _p × Area	7.86	in
D. Peak Flow (PF) = (Q _p × Area) / 640	53,970	cf/s
E. Delta = PF - 0.15A	52,406	cf/s
F. V ₁₀₀ = (Delta/PT) × V _{100-pre}	135,129	cf

V₁₀₀ refers to total infiltration provided per worksheet W11

W11 Determine Applicable BMPs and Associated Volume Credits

Proposed BMP	Average Area (ft ²)	Storage Depth (ft)	Storage Volume (ft ³)	Ave. Design Infil. Rate (in/hr)	Infil. During Storm (ft ³)	Total Volume Reduction (ft ³)
						0
Total Volume Reduction Credit by Proposed Structural BMPs (V ₁₀₀)						0 cf

W13 Infiltration / Detention Summary

Total Infiltration Required per WCVRC Rules:	24,231	cf
Total Infiltration Provided:	0	cf
Difference:	(24,231)	cf
% Deficiency:	100.0%	
Pro-Rated 20% Detention Penalty:	20.0%	
Total Detention Required:	135,129	cf
Total Detention Required including Penalty, if applicable:	162,153	cf

Basin Stage-Storage Summary:

Elev.	Area	Avg. Area	Depth	Volume
817.2	61,000	59,000	1.2	315,300
816.0	65,000	62,000	1.0	344,500
815.0	49,000	45,750	1.0	192,500
814.0	42,500	39,750	1.0	146,750
813.0	37,000	34,500	1.0	107,000
812.0	32,000	29,000	1.0	72,800
811.0	20,000	18,500	1.0	46,500
810.0	17,000	15,500	1.0	28,000
809.0	14,000	12,500	1.0	12,500
808.0	11,000	11,000	0	0
Total Volume Provided:				192,500 cf

Vf Elev. = 809.76
 Vif Elev. = 811.07
 Vprowided Elev. = 814.34
 1' Freeboard = 815.34 Actual = **817.20**

Forebay Sizing

Required Volume (Vf) = 0.05 * V ₁₀₀	6,958	cf
--	-------	----

Forebay Stage-Storage Summary:

Elev.	Area	Avg. Area	Depth	Volume
811.00	5,400	4,600	1.0	8,825
810.00	3,700	2,900	1.0	4,375
809.00	2,100	1,500	0.85	1,275
808.15	900			
Vf Elev. =				810.96

Outlet Control Structure Sizing

1. Standpipe outlet holes sizing - "first flush" runoff
 (First flush discharge should be released in 24 hours)

Qff = Vf / 24 hr / 3600 sec	0.280	cf/s
hff(ave) = 2/3 x (Qff - X ₀)	1.171	ft
Aff(required) = Qff / 0.62 x sqrt(2*32.2*h)	0.052	sf
Selected Orifice Diameter =	1.25	in
Area of each orifice =	0.0025	sf
Number of orifice holes required =	6	holes at elev.
808.00		

Check first flush discharge release time

Aff(actual) =	0.0511	ft ²
Qff = A x 0.62 x sqrt(2*32.2*h)	0.275	cf/s
Tff = Vf / (Qff x 3600)	24.4	hrs
		O.K.

2. Standpipe outlet holes sizing - "Bank full flood" discharge
 (Bankfull volume must be detained between 30 and 48 hours)

Check release from first flush holes only	hbf(ave) = 2.044	ft
hbf(ave) = 2/3 x (Qbf - X ₀)	Qbf = 0.364	cf/s
Qbf = A x 0.62 x sqrt(2*32.2*h)	Tbf = 36.8	hrs
Tbf = Vf / (Qbf x 3600)		

3. Standpipe outlet holes sizing - "100-yr flood" discharge

Q100 = Q _p	Q100 = 1.565	cf/s
Release from above holes	hff = 6.54	ft
hff = (Q100 - X ₀)	hff = 6.54	ft
hff = 2/3 x (Q100 - X ₀)	hff = 4.36	ft
hff = A x 0.62 x sqrt(2*32.2*h)	Aff(required) = 0.040	sf
Remaining flow =	Q100 - Q ₁ = 0.924	cf/s
h100 = 2/3 * (Q100 - Q ₁)	h100 = 6.937	ft
h100 = 2/3 * (Q100 - Q ₁)	h100 = 4.224	ft
A = (Q100 - Q ₁) / (0.62 x sqrt(2*32.2*h100))	Aff(required) = 0.096	sf
Selected Orifice Diameter =	2	in
Area of each orifice =	0.0218	sf
Number of orifice holes required =	4	holes at elev.
811.07		

Reduce No. of Holes for 72hr Detention Time, Use **4** holes at elev. **811.07**

4. Risers Outlet Pipe Design

Outlet pipe designed to handle the 100-year restricted flow

100-year restricted flow =	Q ₁₀₀ = 1.565	cf/s
Choose outflow pipe diameter =	d = 15	in
Choose outflow pipe slope =	S = 0.32	%
Assume roughness factor =	n = 0.013	
Flow velocity at 100-yr restricted flow (Manning) =	V = 3.36	fps
Design Pipe Capacity =	C = 5.94	cf/s

Basin Overflow Weir

Drainage Area, A =	10.43	ac
C-factor (C) =	0.64	
Time of Conc. (T) =	30.00	min
Intensity (I) =	5.00	in/hr
Peak flow to basin (Q) =	33.36	cf/s
Weir Coef (C _w) =	3.367	
Weir Elevation =	814.7	
Height of Weir (H) =	2.5	
Min. Length of Weir (B) =	2.5	ft
L = Q / (C _w * H ^{1.5} / 2)		

Forebay Standpipe outlet holes sizing

Discharge should be released from Forebay in 24 hours

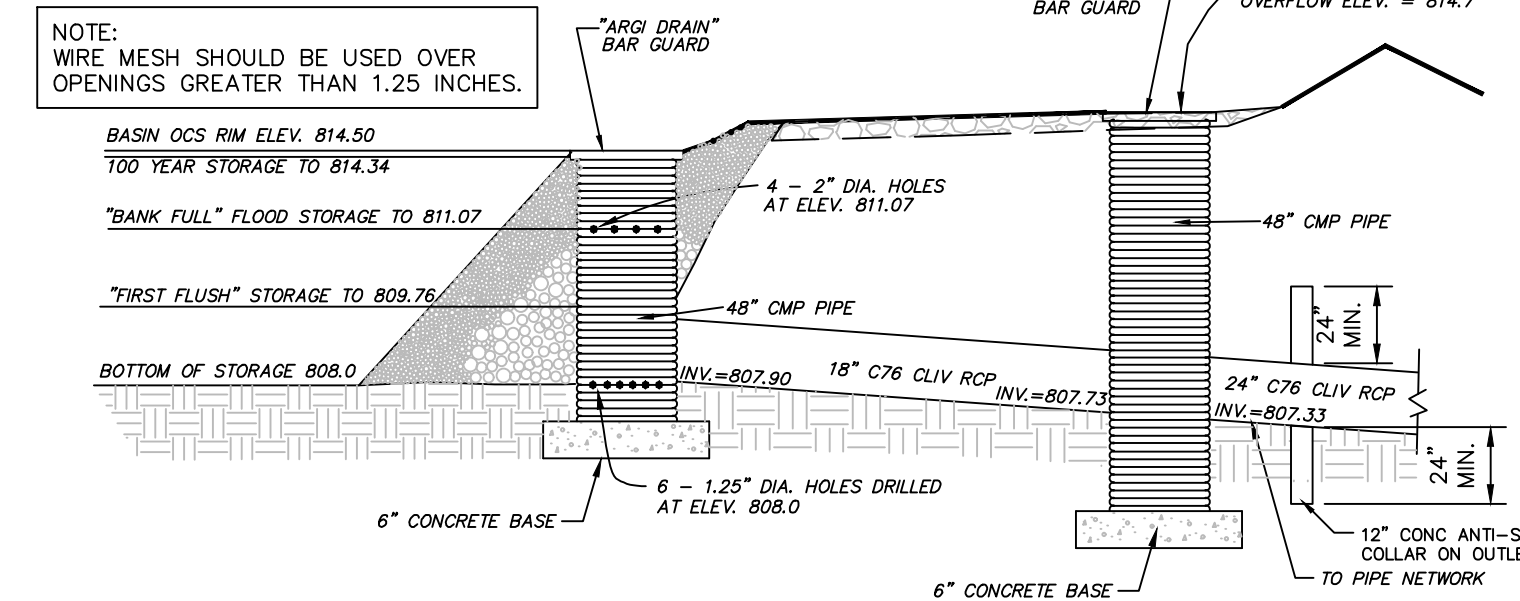
Qfb = Vf / 24 hr / 3600 sec	0.281	cf/s
hfb(ave) = 2/3 x (Qfb - X ₀)	1.87	ft
Afb(required) = Qfb / 0.62 x sqrt(2*32.2*h)	0.012	sf
Selected Orifice Diameter =	1	in
Area of each orifice =	0.0005	sf
Number of orifice holes required =	2	holes at elev.
808.15		

Check discharge release time

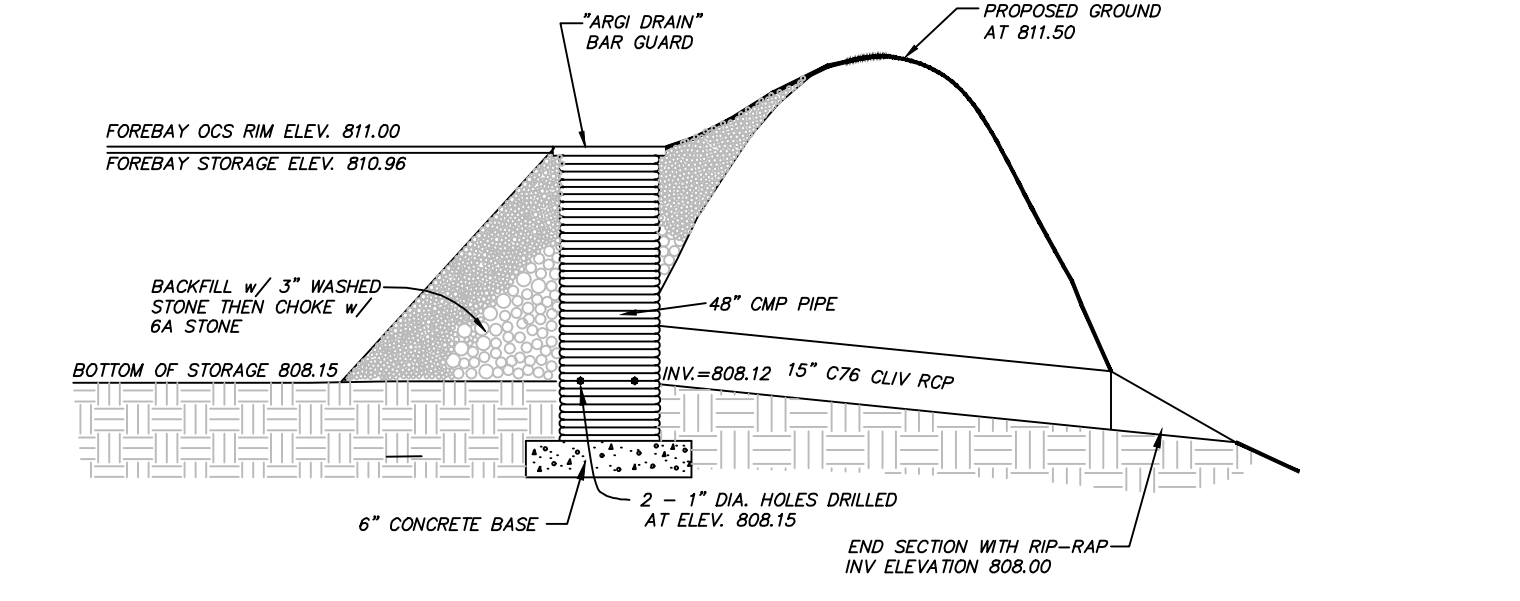
Afb(actual) =	0.0109	ft ²
Qfb = A x 0.62 x sqrt(2*32.2*h)	0.270	cf/s
Tfb = Vf / (Qfb x 3600)	26.0	hrs
		O.K.

NOTE:
 A. MOWING IS ONLY ALLOWED TWICE PER YEAR WITHIN THE STORM WATER FEATURES.
 B. AT THE TIME OF PLANT AND SEED DELIVERY, A WCVRC LANDSCAPE REVEALER MUST BE PRESENT. THE QUANTITY AND SPECIES DELIVERED WILL BE REVIEWED ON SITE. CONTACT CATIE WYTYCHAK AT WYTYCHAK@BOWSHENAW.ORG TO COORDINATE.

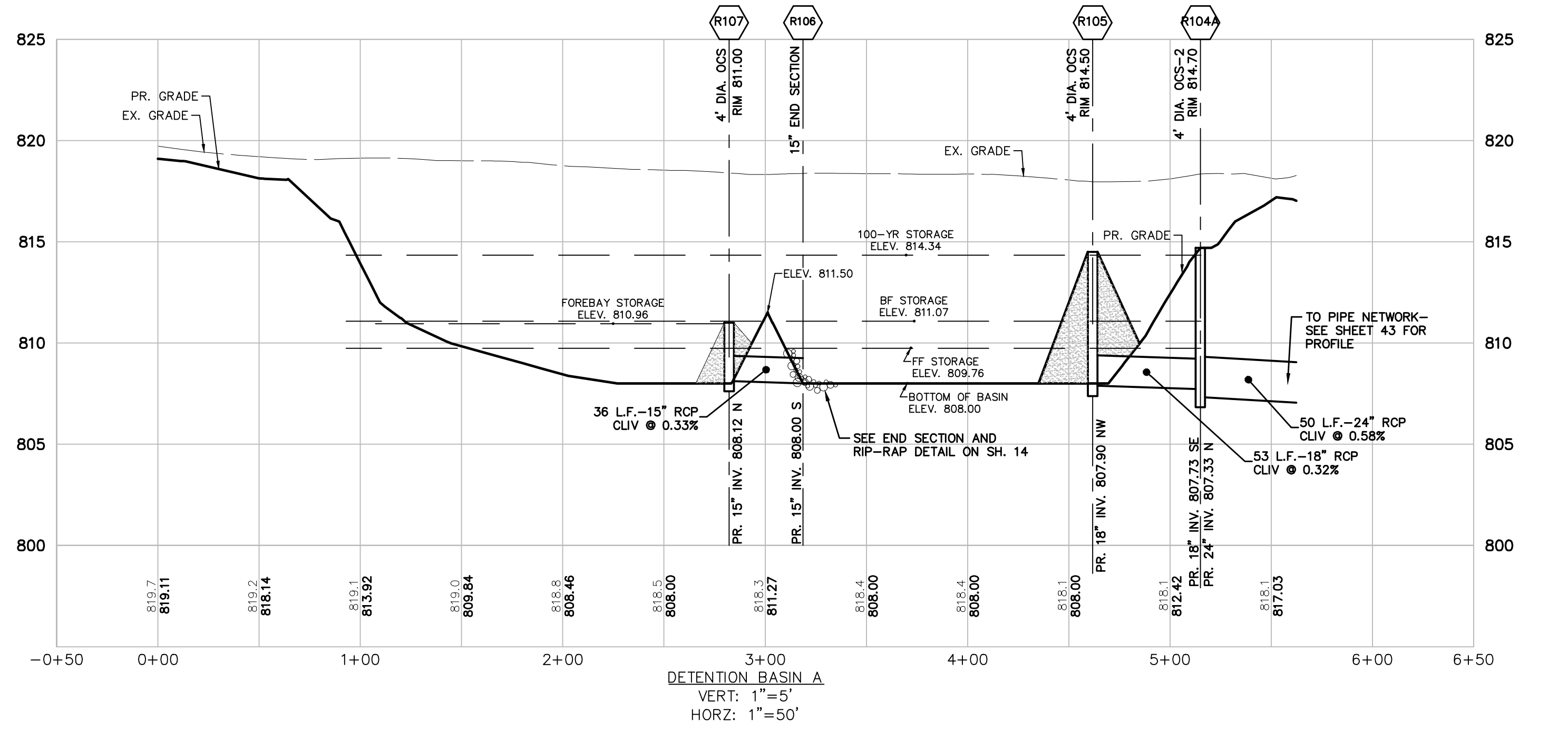
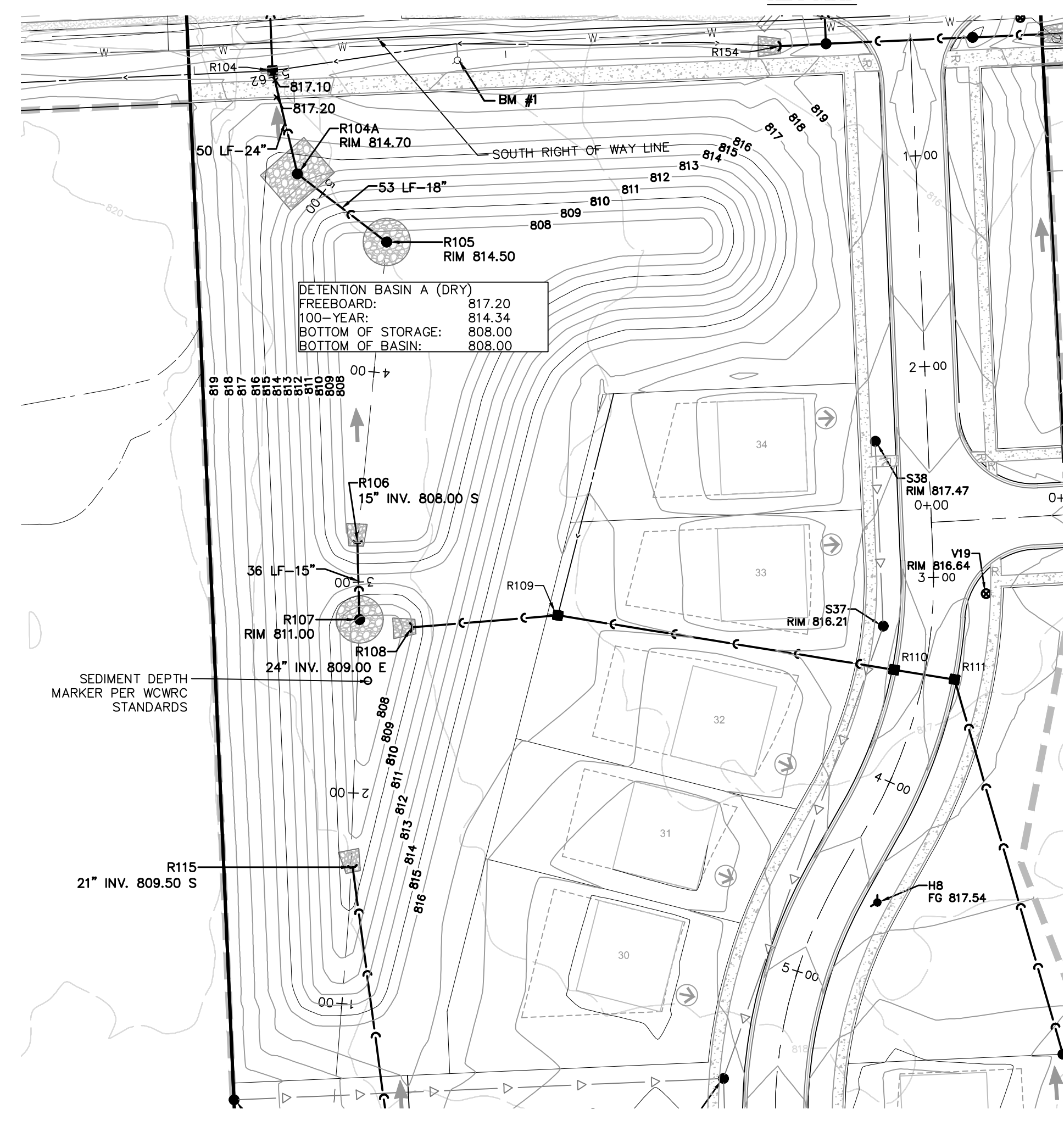
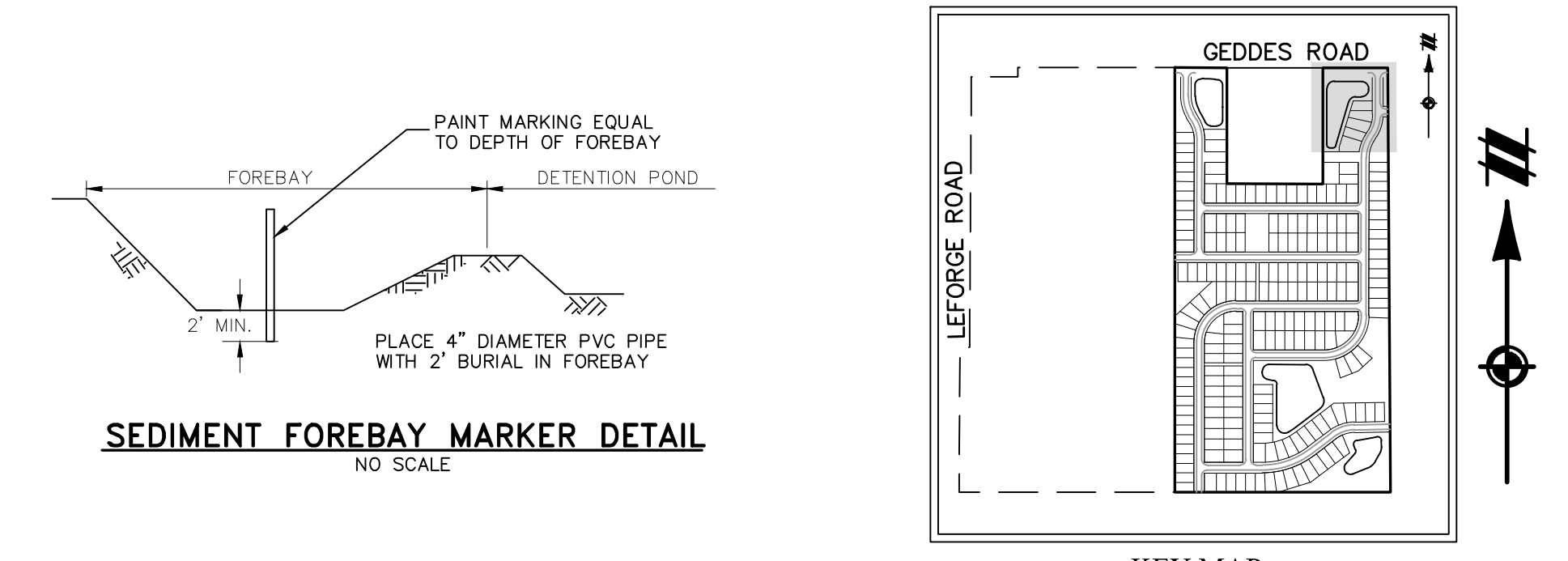
NOTE:
 A. A SECONDARY OVERFLOW STRUCTURE IS PROVIDED INSTEAD OF A GROUND OVERFLOW PATH DUE TO SITE CONDITIONS.
 B. OUTLET PIPE HAS BEEN UPSIZED TO 24" ALL THE WAY TO THE LAST DOWNSTREAM PIPE TO PROVIDE BETTER CAPACITY DURING OVERFLOW CONDITIONS. REFER TO SHEET 43 FOR OUTLET STORM SEWER PROFILE.



DETENTION BASIN - OUTLET DETAIL
NO SCALE



DETENTION BASIN - SEDIMENT FOREBAY OUTLET DETAIL
NO SCALE



DETENTION BASIN A
VERT: 1"=5'
HORIZ: 1"=50'

811
 Know what's below.
 Call before you dig.
 THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY AND ARE NOT TO BE INDEPENDENTLY VERIFIED BY THE OWNER OR ITS REPRESENTATIVE. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY ANY AND ALL UNDERGROUND UTILITIES.

NOTICE:
 CONSTRUCTION SITE SAFETY IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. NEITHER THE OWNER NOR THE ENGINEER SHALL BE RESPONSIBLE FOR SAFETY OF THE WORK OF PERSONS ENGAGED IN THE WORK OF ANY NEARBY STRUCTURES, OR OF ANY OTHER PERSONS.
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SECTION 33
 TOWN 2 SOUTH, RANGE 7 EAST
 SUPERIOR TOWNSHIP
 WASHTEENAW COUNTY, MICHIGAN

EYE COMPANY
 THE MEADOWS AT HAWTHORNE MILL
 FINAL SITE PLANS - PHASE 1
 DETENTION BASIN PLAN -
 BASIN A

DATE
 OCT. 12, 2023

REVISIONS
 0 25 50
 SCALE: 1" = 50 FEET
 DRAWN BY: KS
 CHECKED BY: AK
 P.M.: J. KIME
 JOB #: 19004443
 FILE CODE: -
 SHEET NO. 47

K:\19004443\DWG\PLAN SET\FINAL\PHASE 1\19004443-SP-17-DET.DWG 10/10/2023 4:07 PM MATTHEW MYERS

DETENTION BASIN C

Project: The Meadows at Hawthorne Mill
 Location: Superior Township
 Description: Detention Basin C

Date: 10/10/2023
 By: Atwell

W1 Determine Post-Development Cover Types, Areas, Curve Numbers and runoff coefficients					
Total Contributing Drainage Area = 8.15 Acres					Total Site Area = 8.65 Acres
Paved Parking lots, roads, driveways					
Cover Type	Soil Type	Area (sf)	Area (ac)	Runoff Coef. (C)	(C)(Area)
Paved Parking lots, roads, driveways	D	114,435	2.63	0.95	109,713
Water Surfaces	D	39,000	0.90	1.00	39,000
Developed Open Space, Good Condition	D	171,087	3.93	0.45	76,589
Off-site Contributing Area	D	30,492	0.70	0.45	13,721
Total - Sum (C)(Area) = 238,424					Area Total (sf) = 355,014
Weighted C-Sum (C)(Area)/Sum(C) or Sum(C) = 0.67					

W2 First Flush Runoff Calculations (Vf)					
A.	Vf = (1/12) (4356/1) (C) AC =	19,822 cf			

W3 Predevelopment Bankfull Runoff Calculations (Vb-Pre)					
A.	2 year/24 hour storm event	P =	2.35 in		
B.	Previous Cover CN (Good Cover Woods or Meadows)	CN =	78		
C.	S = (1000/CN) - 10	S =	2.83		
D.	Q = (P - 0.25) / (P - 0.85)	Q =	0.69 in		
E.	Previous Cover Area	Area =	355,014 sf		
F.	V _{100-yr-pre} = Q(1/12)Area	V _{100-yr-pre} =	20,484 cf		

W4 Previous Cover Post-development Bankfull Runoff Calculations (Vb-Post)					
A.	2 year/24 hour storm event	P =	2.35 in		
B.	Previous Cover CN	CN =	98		
C.	S = (1000/CN) - 10	S =	2.50 in		
D.	Q = (P - 0.25) / (P - 0.85)	Q =	0.79 in		
E.	Previous Cover Area	Area =	201,579 sf		
F.	V _{100-yr-post} = Q(1/12)Area	V _{100-yr-post} =	13,217 cf		

W5 Impervious Cover Post-development Bankfull Runoff Calculations (Vb-imp-post)					
A.	2 year/24 hour storm event	P =	2.35 in		
B.	Impervious Cover CN	CN =	98		
C.	S = (1000/CN) - 10	S =	2.50 in		
D.	Q = (P - 0.25) / (P - 0.85)	Q =	0.79 in		
E.	Previous Cover Area	Area =	153,435 sf		
F.	V _{100-yr-post} = Q(1/12)Area	V _{100-yr-post} =	27,128 cf		

W6 Previous Cover Post-development 100-year Storm Runoff Calculations (V100-Pre-Post)					
A.	100 year storm event	P =	5.11 in		
B.	Previous Cover CN	CN =	98		
C.	S = (1000/CN) - 10	S =	2.50 in		
D.	Q = (P - 0.25) / (P - 0.85)	Q =	2.99 in		
E.	Previous Cover Area	Area =	201,579 sf		
F.	V _{100-yr-pre} = Q(1/12)Area	V _{100-yr-pre} =	50,211 cf		

W7 Impervious Cover Post-development 100-year Storm Runoff Calculations (V100-imp-post)					
A.	100 year storm event	P =	5.11 in		
B.	Previous Cover CN	CN =	98		
C.	S = (1000/CN) - 10	S =	2.50 in		
D.	Q = (P - 0.25) / (P - 0.85)	Q =	4.87 in		
E.	Previous Cover Area	Area =	153,435 sf		
F.	V _{100-yr-post} = Q(1/12)Area	V _{100-yr-post} =	62,307 cf		

W8 Determine Time of Concentration (Tc-hrs)					
User specified, assume 30 minutes					Total Time of Concentration (hrs) = 0.50

W9 Runoff Summary & Onsite Infiltration Requirement					
A. Runoff Summary from Previous Worksheets					
V _{100-yr-pre}	= 50,211 cf		Total 100-year Volume (V _{100-yr})		112,518 cf
V _{100-yr-post}	= 62,307 cf		Total 100-year Volume (V _{100-yr})		112,518 cf

B. Determine Onsite Infiltration Requirement					
V _{100-yr-pre}	= 50,211 cf		Total 100-year Volume (V _{100-yr})		112,518 cf
V _{100-yr-post}	= 62,307 cf		Total 100-year Volume (V _{100-yr})		112,518 cf
Bankfull Volume Difference =					19,861 cf
Onsite Infiltration Requirement (V _{infil}) =					19,861 cf

W10 Detention / Retention Requirement					
A.	Q ₁₀₀ = 238.4 (T _c) ^{-0.82}	421.23 cfs/invol*2			
B.	Total Site Area	8.15 ac			
C.	Q ₁₀₀ = Q _{100-pre} + Q _{100-imp}	7.86 in			
D.	Peak Flow (PF) = (Q ₁₀₀ - Q _{100-pre}) / 60	42,172 cfs			
E.	Delta = PF - 0.15A	60,950 cfs			
F.	V _{det} = (Delta/2) x V _{100-yr}	109,256 cf			

* V_{infil} refers to total infiltration provided per worksheet W11

W11 Determine Applicable BMPs and Associated Volume Credits					
Proposed BMP	Average Area (ft ²)	Storage Depth (ft)	Storage Volume (ft ³)	Ave. Design Infil. Rate (in/hr)	Infil. During Storm (ft ³)
Total Volume Reduction Credit by Proposed Structural BMPs (V _{infil}) =					0 cf

W13 Infiltration / Detention Summary					
Total Infiltration Required per WCRC Rules	19,861 cf				
Total Infiltration Provided	0 cf				
Difference:	(19,861) cf				
% Deficiency:	100.0%				
Proposed 20% Detention Penalty:	20.0%				
Total Detention Required	109,256 cf				
Total Detention Required including Penalty, if applicable	131,108 cf				

Basin Stage-Storage Summary:					
Elev.	Area	Avg. Area	Depth	Volume	
808.0	43,000	41,000	1.0	177,800	
807.0	39,000	37,150	1.0	136,800	
806.0	35,300	33,400	1.0	99,600	
805.0	31,500	27,000	1.0	66,250	
804.0	22,500	21,000	1.0	39,250	
803.0	19,500	18,250	1.0	18,250	
802.0	17,000	17,000	0	0	
Total Storage Volume/Volume Provided:				136,800 cf	
V _{100-yr-pre} =				50,211 cf	
V _{100-yr-post} =				62,307 cf	
V _{100-yr-imp} =				62,307 cf	
Freeboard =				807.85	Actual: 808.00

Forebay Sizing				
Required Volume (V _{fb}) = 0.05 * V ₁₀₀	5,625 cf			

Forebay Storage Summary:				
Elev.	Area	Avg. Area	Depth	Volume
804.00	4,500	3,950	1.0	6,415
803.00	3,400	2,900	0.9	2,465
802.15	2,400			
V _{fb} Elev. =				803.80

Outlet Control Structure Sizing				
1. Standpipe outlet holes sizing - "first flush" runoff (first flush discharge should be released in 24 hours)				
Q _{fb} = V _{fb} / 24 hrs / 3600 sec	0.229 cfs			
h _{fb(ave)} = 2.75 x (Q _{fb} - 30)	0.217 ft			
A _{fb(required)} = Q _{fb} / (0.62 x sqrt(2*32.2*h))	0.054 sf			
Selected Orifice Diameter =	1.25 in			
Area of each orifice =	0.009 sf			
Number of orifice holes required =	6 holes at elev. 802.00			

2. Standpipe outlet holes sizing - "Bankfull flood" discharge (Bankfull Volume must be detained between 36 and 48 hours)				
Check release from first flush holes only				
h _{fb(ave)} = 2.75 x (Q _{fb} - 30)	1.360 ft			
Q _{fb} = A x 0.62 x sqrt(2*32.2*h)	0.297 cfs			
T _{det} = V _{fb} / (Q _{fb} x 3600)	37.8 hrs			

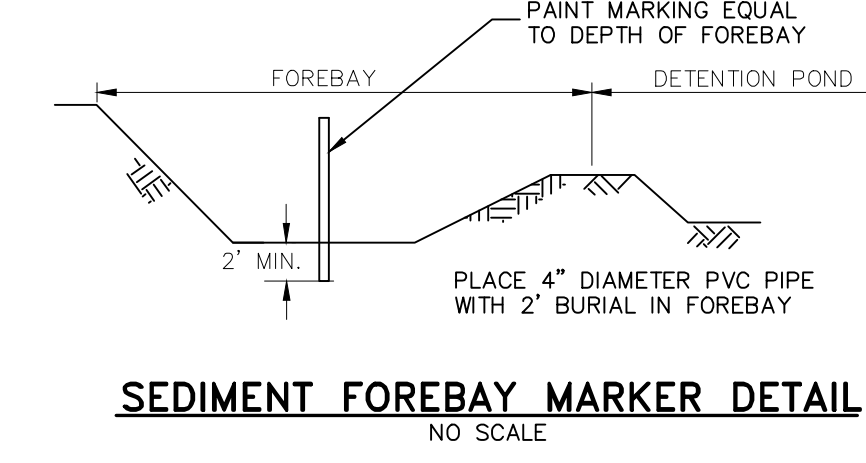
3. Standpipe outlet holes sizing - "100-yr flood" discharge				
Q ₁₀₀ = Q _a	1.223 cfs			
h _{fb} = (1000 - X) / 100	4.85 ft			
h _{fb} = (1000 - X) / 100	3.77 ft			
Q _a = A x 0.62 x sqrt(64.4*h _{fb}) + A x 0.62 x sqrt(64.4*h _{fb})	0.560 cfs			
Remaining flow =	0.662 cfs			
h ₁₀₀ = 2/3 * (100 - 20)	4.67 ft			
A = Q ₁₀₀ / (0.62 x sqrt(2*32.2*h ₁₀₀))	0.074 sf			
Selected Orifice Diameter =	3.5 in			
Area of each orifice =	0.066 sf			
Number of orifice holes required =	1 holes at elev. 804.04			
Reduce No. of holes for 72hr Detention Time, Use	1 holes at elev. 804.04			

4. Riser Outlet Pipe Design				
Outlet pipe designed to handle the 100-year restricted flow				
100-year restricted flow =	0.66 cfs			
Choose outlet pipe diameter =	12 in			
Choose outlet pipe slope =	0.7%			
Assume roughness factor =	0.024 (CIP)			
Flow velocity at 100-year restricted flow (Manning) =	5.33 fps			
Design Pipe Capacity =	4.19 cfs			

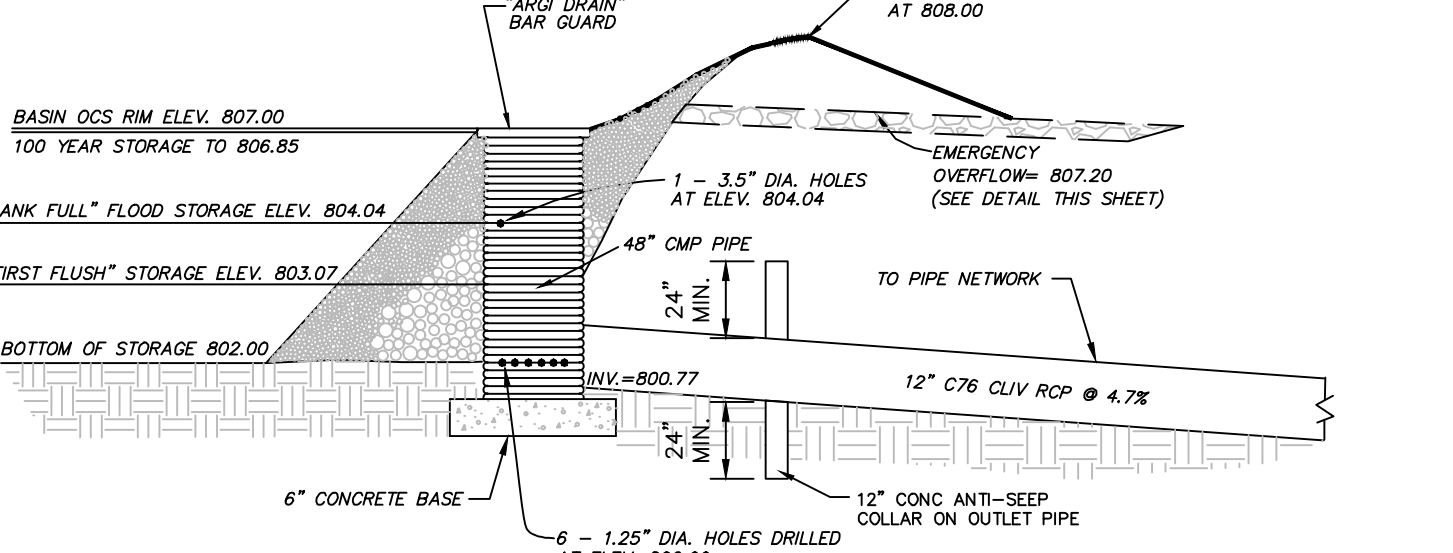
Basin Overflow Weir				
Drainage Area, A =	8.15 ac			
C-factor (C) =	0.67			
Time of Conc. (T) =	30.00 min			
Intensity (I) =	5.00 in/hr			
Peak Flow to basin (Q) =	27.30 cfs			
Weir Coef (C _w) =	3.967			
Weir Elevation:	807.2			
Height of Weir (H) =	0.8			
Min. Length of Weir (L) =	11.3 ft			

Forebay Standpipe outlet holes sizing				
Discharge should be released from forebay in 24 hours				
Q _{fb} = V _{fb} / 24 hrs / 3600 sec	0.229 cfs			
h _{fb(ave)} = 2.75 x (Q _{fb} - 30)	1.30 ft			
A _{fb(required)} = Q _{fb} / (0.62 x sqrt(2*32.2*h))	0.012 sf			
Selected Orifice Diameter =	0.75 in			
Area of each orifice =	0.003 sf			
Number of orifice holes required =	4 holes at elev. 802.15			

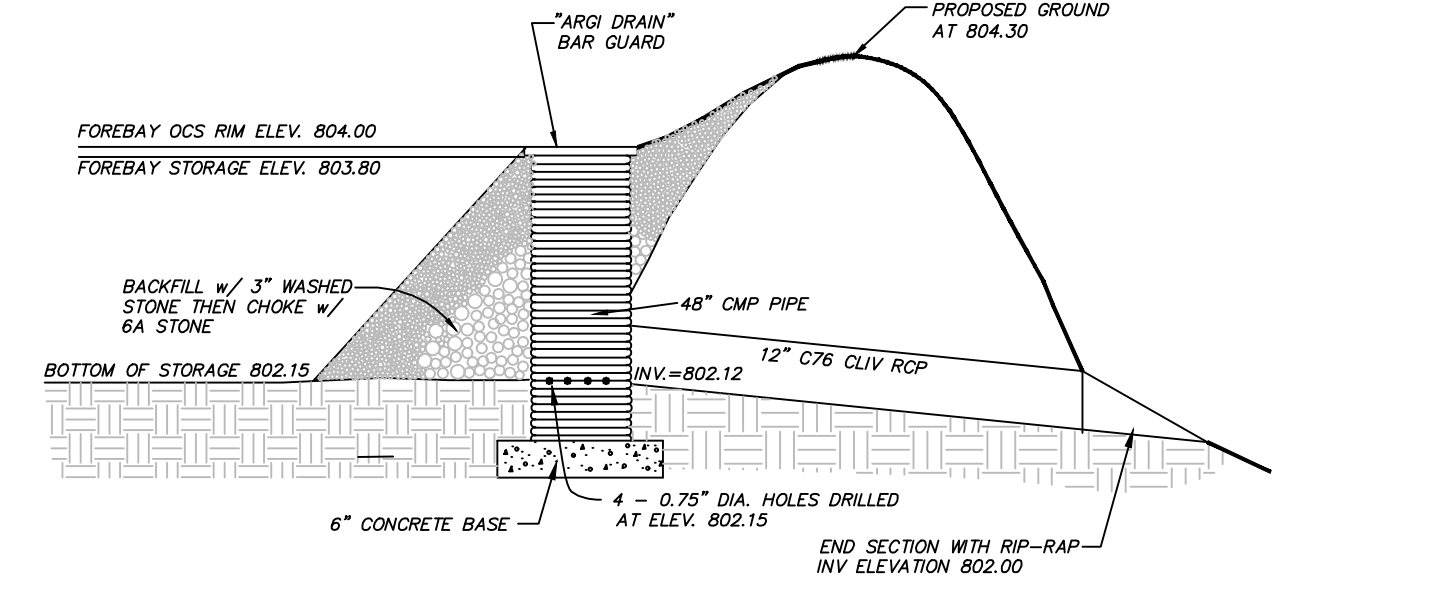
Check discharge release time				
A _{fb(actual)} =	0.0123 ft ²			
Q _{fb} = A x 0.62 x sqrt(2*32.2*h)	0.0640 cfs			
T _{det} = V _{fb} / (Q _{fb} x 3600)	24.4 hrs			



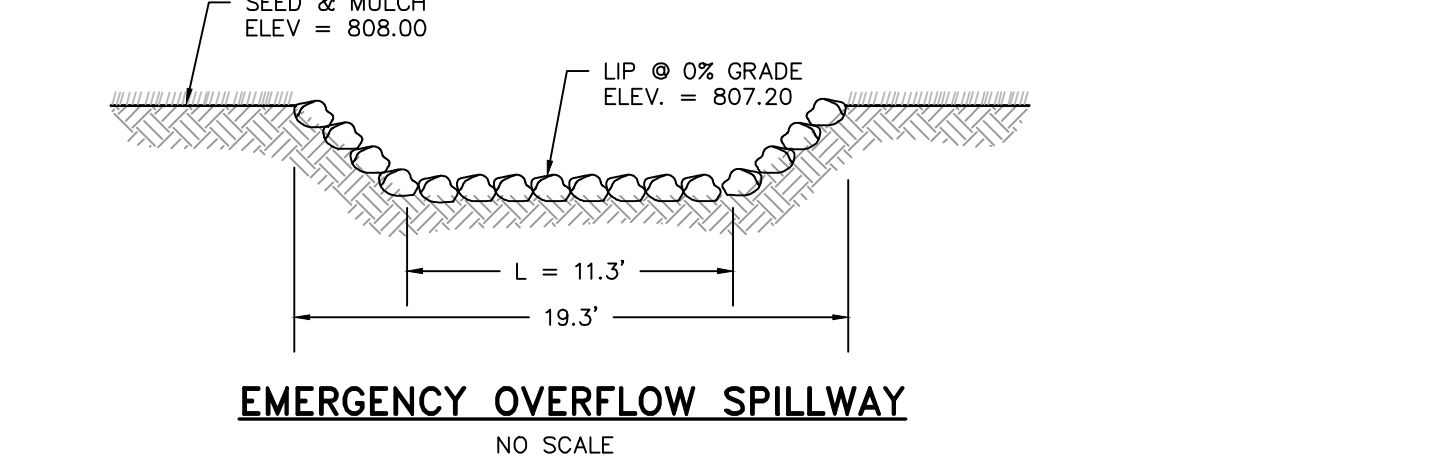
SEDIMENT FOREBAY MARKER DETAIL
NO SCALE



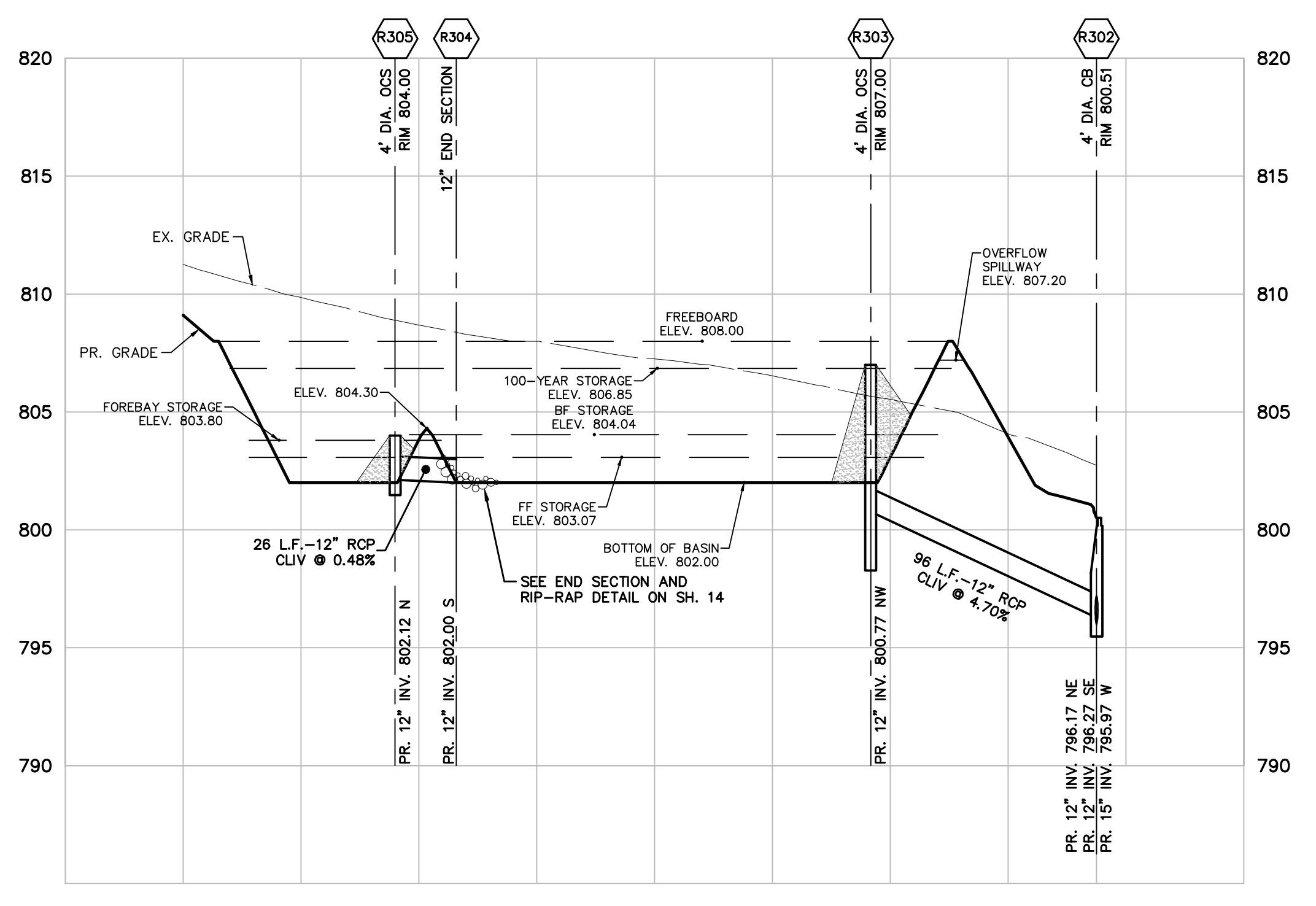
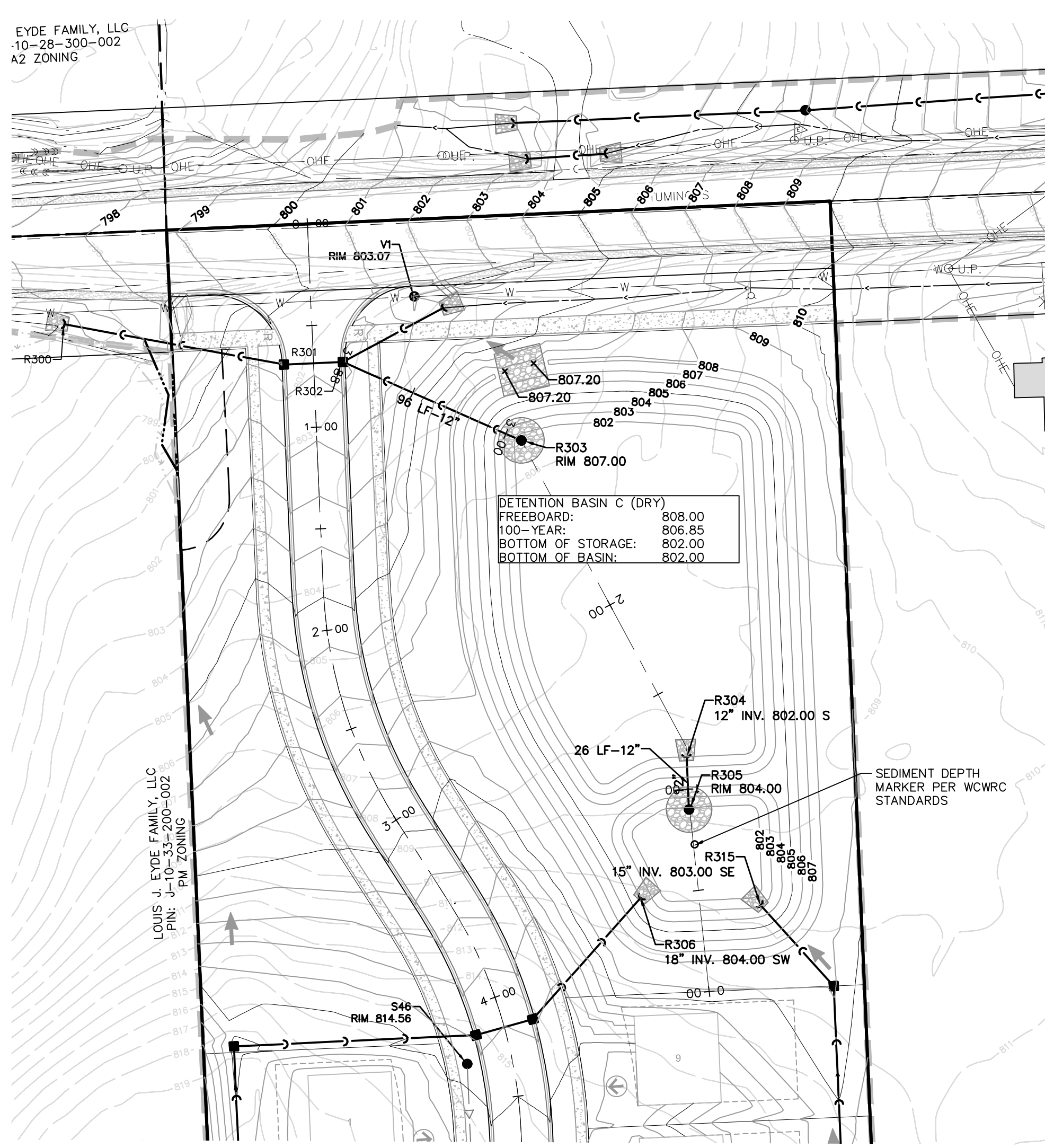
DETENTION BASIN - OUTLET DETAIL
NO SCALE



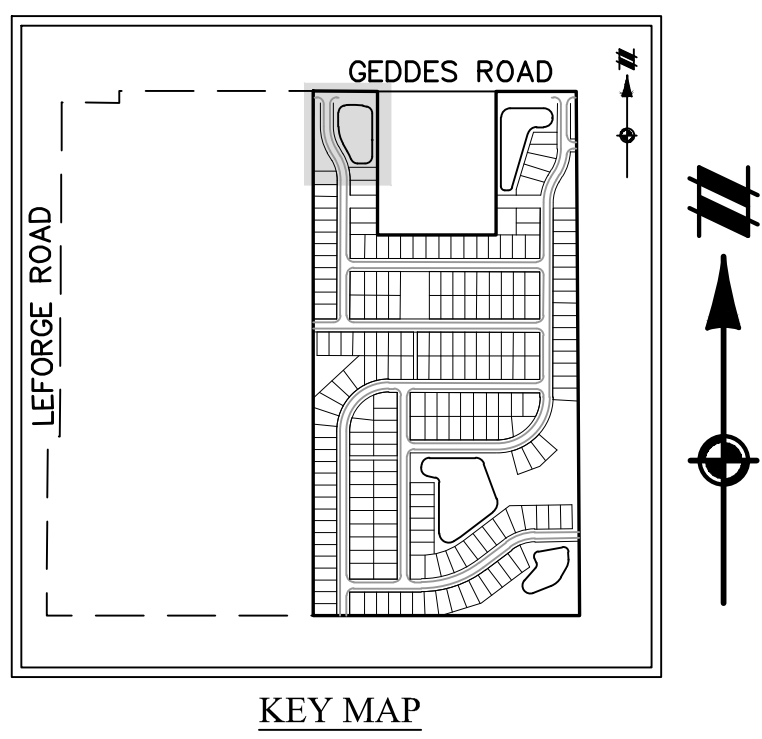
DETENTION BASIN - SEDIMENT FOREBAY OUTLET DETAIL
NO SCALE



EMERGENCY OVERFLOW SPILLWAY
NO SCALE



DETENTION BASIN C
VERT: 1"=5'
HORIZ: 1"=50'



KEY MAP

NOTE:
 A. MOWING IS ONLY ALLOWED TWICE PER YEAR WITHIN THE STORM WATER FEATURES.
 B. AT THE TIME OF PLANT AND SEED DELIVERY, A WCRC LANDSCAPE REVIEWER MUST BE PRESENT. THE QUANTITY AND SPECIES DELIVERED MUST BE REVIEWED ON SITE. CONTACT CATIE WYTYCHAK AT WYTYCHAK@WASHTENAW.ORG TO COORDINATE.

811
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SECTION 33
 TOWN 2 SOUTH, RANGE 7 EAST
 SUPERIOR TOWNSHIP
 WASHTENAW COUNTY, MICHIGAN

EYE COMPANY
 THE MEADOWS AT HAWTHORNE MILL
 FINAL SITE PLANS - PHASE 1
 DETENTION BASIN PLAN -
 BASIN C

DATE: OCT. 12, 2023
 REVISIONS
 SCALE: 1"=50 FEET
 DRAWN BY: KS
 CHECKED BY: AK
 P.M.: J. KIME
 JOB #: 19004443
 FILE CODE: -
 SHEET NO. 48

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CAD FILE: 19004443-SP-17-DET.DWG

STORM SEWER CONVEYANCE CALCULATIONS

ON-SITE STORM SEWER CONVEYANCE SYSTEM DESIGN

Project: Meadows at Hawthorne Mill
Date: May 27, 2022
Revision: 1

Community Superior Township
County: Washtenaw

$$I = \frac{B(T+D)^E}{C} \quad B = 151.8 \quad D = 19.9 \quad E = 1$$
$$C = 20 \quad 0.013 \text{ RCP} \quad 0.024 \text{ CMP}$$

FROM MH	TO MH	INCR. ACRES	C	EQUIV. AREA 100% ACRES CA	TOTAL AREA 100% ACRES CA	T TIME (MIN.)	I (IN PER HOUR)	Q-CIA C.F.S. FLOW	CAPAC-ITY OF SEWER (C.F.S.)	DIAM. OF PIPE (IN.)	LENGTH OF LINE (FT.)	SLOPE OF PIPE (%)	MIN HG BASED ON "Q" (%)	VEL. FLOW FULL (FT./SEC.)	TIME OF FLOW (MIN.)	H.G.L. ELEV. UPPER END	H.G.L. ELEV. LOWER END	GROUND ELEV. UPPER END	GROUND ELEV. LOWER END	INVERT ELEV. UPPER END	INVERT ELEV. LOWER END
R114	R113	0.13	0.47	0.06	0.06	20.00	3.80	0.24	2.52	12	120	0.50	0.00	3.2	0.6	815.25	814.65	816.77	818.10	814.45	813.85
R113	R112	0.28	0.47	0.12	0.19	20.60	3.75	0.70	2.52	12	120	0.50	0.04	3.2	0.6	814.55	813.95	816.10	817.50	813.75	813.15
R112	R111A	0.25	0.48	0.12	0.31	21.20	3.69	1.14	3.19	12	61	0.80	0.10	4.1	0.3	813.85	813.25	817.50	818.69	813.05	812.56
R111A	R111	0.00	0.00	0.00	0.31	21.50	3.67	1.14	4.57	15	185	0.50	0.03	3.7	0.8	813.15	812.24	818.69	815.68	812.16	811.24
R111	R110	1.24	0.52	0.55	0.95	22.30	3.60	3.43	4.09	15	29	0.40	0.28	3.3	0.1	811.74	811.62	815.58	815.58	810.74	810.62
R110	R109	0.53	0.45	0.13	1.30	21.70	3.65	4.75	5.56	18	75	0.28	0.21	3.1	0.4	811.79	811.58	819.94	819.34	810.59	810.38
R109	R108	0.31	0.33	0.10	1.90	23.00	3.54	6.72	14.31	24	69	0.40	0.09	4.6	0.3	811.11	811.05	816.10	817.75	809.28	809.00
R123	R122	0.28	0.45	0.13	0.13	20.00	3.80	0.49	2.36	12	130	0.44	0.02	3.0	0.7	813.13	812.56	816.38	819.42	812.33	811.76
R122	R121	0.00	0.00	0.00	0.13	20.70	3.74	0.49	2.36	12	60	0.44	0.02	3.0	0.3	812.45	812.19	819.42	818.22	811.96	811.39
R121	R117	0.25	0.47	0.12	0.25	21.00	3.71	0.94	3.77	15	120	0.34	0.02	3.1	0.7	812.19	811.79	818.22	819.94	811.19	810.79
R117	R116	0.29	0.45	0.13	1.30	21.70	3.65	4.75	5.56	18	75	0.28	0.21	3.1	0.4	811.79	811.58	819.94	819.34	810.59	810.38
R116	R115	0.24	0.31	0.08	1.38	22.10	3.51	4.98	6.38	21	242	0.28	0.10	3.5	1.2	811.58	811.05	819.34	811.98	810.18	809.50
R119	R118	0.68	0.63	0.43	0.43	20.00	3.80	1.64	2.78	12	29	0.60	0.21	3.5	0.1	816.70	816.52	821.17	821.17	815.90	815.72
R118	R117	0.70	0.65	0.45	0.88	20.10	3.80	3.34	3.56	12	123	1.00	0.88	4.5	0.5	813.52	812.29	812.17	819.94	812.72	811.49
R120	R117	0.11	0.39	0.04	0.04	20.00	3.80	0.16	5.63	12	65	2.50	0.00	7.2	0.2	817.41	815.79	821.57	819.94	816.61	814.99
R105	R104A	0.00	0.00	0.00	0.00	20.00	3.80	0.00	5.94	16	53	0.32	0.00	3.4	0.3	809.10	808.93	814.50	814.70	807.90	807.73
R104A	R104	0.00	0.00	0.00	0.00	20.30	3.78	0.00	17.23	24	50	0.58	0.00	5.5	0.2	808.93	808.64	814.70	816.93	807.33	807.04
R104	R103	0.27	0.49	0.13	0.13	20.50	3.76	0.49	13.94	24	90	0.38	0.00	4.4	0.3	808.54	808.19	816.93	818.08	806.94	806.59
R103	R102A	0.00	0.00	0.00	0.13	20.80	3.73	0.49	12.39	24	225	0.30	0.00	3.9	1.0	807.69	807.22	818.08	819.42	806.29	805.62
R102A	R102	0.00	0.00	0.00	0.13	21.80	3.64	0.49	13.94	24	116	0.38	0.00	4.4	0.4	805.82	805.37	819.42	817.03	804.22	803.77
R102	R101	0.00	0.00	0.00	0.13	22.20	3.61	0.49	13.94	24	309	0.38	0.00	4.4	1.2	805.27	804.10	817.03	806.95	803.67	802.50
R101	R100	0.00	0.00	0.00	0.13	23.40	3.51	0.49	11.97	24	144	0.28	0.00	3.8	0.6	804.00	804.00	805.95	804.75	802.40	802.00
R153	R152	1.14	0.39	0.44	0.44	20.00	3.80	1.67	3.88	15	33	0.36	0.07	3.2	0.2	816.72	816.67	820.50	820.50	815.72	815.60
R152	R151	0.00	0.00	0.00	0.44	20.20	3.79	1.67	3.88	15	70	0.36	0.07	3.2	0.4	816.50	816.25	820.50	820.00	815.50	815.25
R151	R150	0.00	0.00	0.00	0.53	20.60	3.75	1.99	3.88	15	92	0.36	0.09	3.2	0.5	816.16	816.07	820.00	816.76	815.15	814.82
R154	R151	0.19	0.49	0.09	0.09	20.00	3.80	0.35	5.34	12	22	7.65	0.01	6.8	0.1	818.10	816.44	818.80	820.00	817.30	815.64
R260	R259	0.17	0.69	0.12	0.12	20.00	3.80	0.44	2.52	12	29	0.50	0.02	3.2	0.2	817.09	816.95	823.91	823.91	816.29	816.15
R259	R258	0.33	0.61	0.20	0.32	20.20	3.79	1.21	2.52	12	93	0.50	0.11	3.2	0.5	815.95	815.48	823.91	821.36	815.15	814.68
R258	R257	0.51	0.48	0.24	0.56	20.70	3.74	2.10	2.47	12	120	0.48	0.35	3.1	0.6	815.38	814.80	821.36	820.95	814.58	814.01
R257	R256	0.98	0.38	0.38	0.94	21.30	3.68	3.48	4.48	15	206	0.48	0.29	3.6	0.9	814.60	813.61	820.95	822.60	813.60	812.61
R256	R255	0.19	0.27	0.09	0.29	22.20	3.61	3.35	4.57	15	120	0.50	0.30	3.5	0.1	813.11	812.51	822.60	820.89	812.51	811.78
R255	R254	0.52	0.47	0.25	1.24	22.70	3.56	4.40	4.57	15	120	0.50	0.46	3.7	0.5	811.51	810.91	819.59	819.59	810.51	809.91
R254	R253	0.52	0.47	0.25	2.10	23.20	3.52	3.39	8.14	18	141	0.60	0.50	4.6	0.5	809.41	808.57	819.59	819.26	808.21	807.37
R253	R252	0.67	0.76	0.51	2.61	23.70	3.48	9.06	9.40	18	29	0.80	0.75	5.3	0.1	808.47	808.24	819.26	819.26	807.27	807.04
R252	R251	0.68	0.84	0.57	3.18	23.80	3.47	11.04	12.39	27	140	0.16	0.13	3.1	0.8	805.69	805.65	819.26	812.26	804.09	803.86
R251	R250	0.44	0.41	0.18	5.62	24.60	3.41	19.17	19.59	27	116	0.40	0.38	4.9	0.4	805.56	805.10	812.26	815.55	803.76	803.30
R250	R249	0.33	0.00	0.00	5.62	25.00	3.38	19.17	20.54	27	25	0.44	0.38	5.2	0.1	805.00	804.89	815.55	812.93	803.20	803.09
R249	R248	0.50	0.68	0.59	6.21	25.10	3.37	20.94	23.19	27	29	0.56	0.46	5.8	0.1	804.73	804.63	812.93	812.93	802.99	802.83
R248	R247	0.50	0.81	0.41	9.62	25.20	3.37	22.27	23.20	30	141	0.32	0.29	4.7	0.5	804.63	804.18	812.93	809.58	802.63	802.18
R247	R246	0.48	0.40	0.19	11.32	25.70	3.33	37.68	38.89	36	136	0.34	0.32	5.5	0.4	803.98	803.51	809.58	809.42	801.58	801.11
R246	R245	0.74	0.70	0.52	11.84	26.10	3.30	39.08	40.02	36	29	0.36	0.34	5.7	0.1	803.05	802.98	809.42	809.42	801.41	801.31
R245	R244	0.35	0.67	0.24	12.08	26.20	3.29	39.77	42.68	42	115	0.18	0.16	4.4	0.4	802.95	802.78	809.42	803.78	799.61	799.40
R262	R261	0.55	0.44	0.24	0.24	20.00	3.80	0.92	3.88	15	120	0.36	0.02	3.2	0.6	810.39	809.96	813.82	816.22	809.39	808.96
R261	R254	0.81	0.47	0.38	0.62	20.60	3.75	2.51	3.88	15	125	0.39	0.13	3.2	0.7	806.65	806.41	816.22	816.59	808.86	808.41
R245	R244	0.13	0.55	0.07	0.07	20.00	3.80	0.28	2.52	12	29	0.50	0.01	3.2	0.2	802.05	819.91	823.69	823.69	819.25	819.11
R244	R243	0.41	0.62	0.25	0.33	20.20	3.79	1.24	3.56	12	140	1.00	0.12	4.5	0.5	819.71	818.30	823.69	823.01	818.91	817.50
R243	R242	0.00	0.00	0.00	0.33	20.70	3.74	1.24	5.04	12	130	2.00	0.12	6.4	0.3	817.45	814.85	823.01	817.72	816.65	814.05
R242	R241	0.60	0.39	0.24	0.57	21.00	3.71	2.10	2.39	12	120	0.45	0.35	3.0	0.7	814.75	814.21	817.72	818.62	813.95	813.41
R241	R240	0.38	0.41	0.17	0.71	21.20	3.67	2.73	3.12	12	60	0.60	0.25	3.4	0.3	814.11	813.72	818.62	815.52	813.31	812.92
R240	R239	0.00	0.00	0.00	0.75	22.00	3.62	2.73	2.87	12	125	0.65	0.59	3.7	0.6	812.82	812.01	815.52	816.46	812.02	811.21
R239	R237	0.73	0.41	0.30	1.05	22.60	3.57	3.74	3.88	15	141	0.36	0.34	3.2	0.7	811.01	810.50	816.46	816.10	810.01	809.50
R237	R236	0.27	0.44	0.12	1.16	23.30	3.51	4.09	7.91	15	120	1.50	0.40	6.5	0.3	810.40	808.60	816.46	814.30	809.40	807.60
R236	R235	0.44	0.41	0.18	1.35	23.60	3.49	4.70	8.42	15	120	1.70	0.53	6.9	0.3	808.50	806.46	814.30	812.26	807.50	806.46
R251	R250	0.41	0.64	0.26	0.26	20.00	3.80	0.99	2.78	12	29	0.60	0.08	3.5	0.1	807.18	807.01	811.18	811.18	806.38	806.21
R250	R249	0.30	0.59	0.18	0.44	20.10	3.80	1.65	2.78	12	93	0.60	0.22	3.5	0.4	806.91	806.34	811.18	809.95	806.11	805.54
R249	R248	0.46	0.41	0.19	0.63	20.50	3.76	2.36	3.88	15	120	0.36	0.13	3.2	0.6	806.34	805.91	809.95	810.25	805.34	804.91
R248	R247	0.68	0.42	0.29	0.91	21.10	3.70	3.38	5.56	18	125	0.28	0.10	3.1	0.7	805.91					

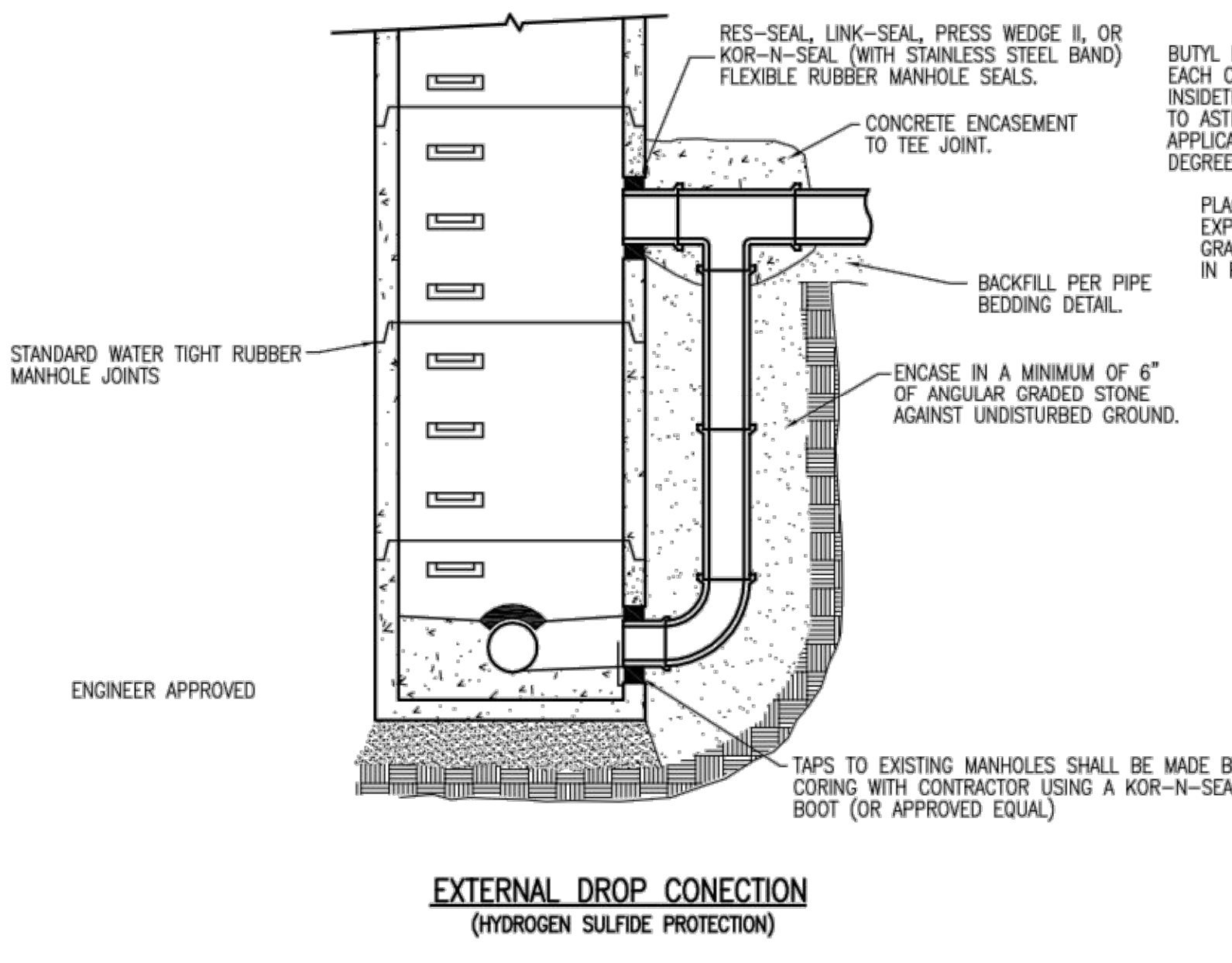
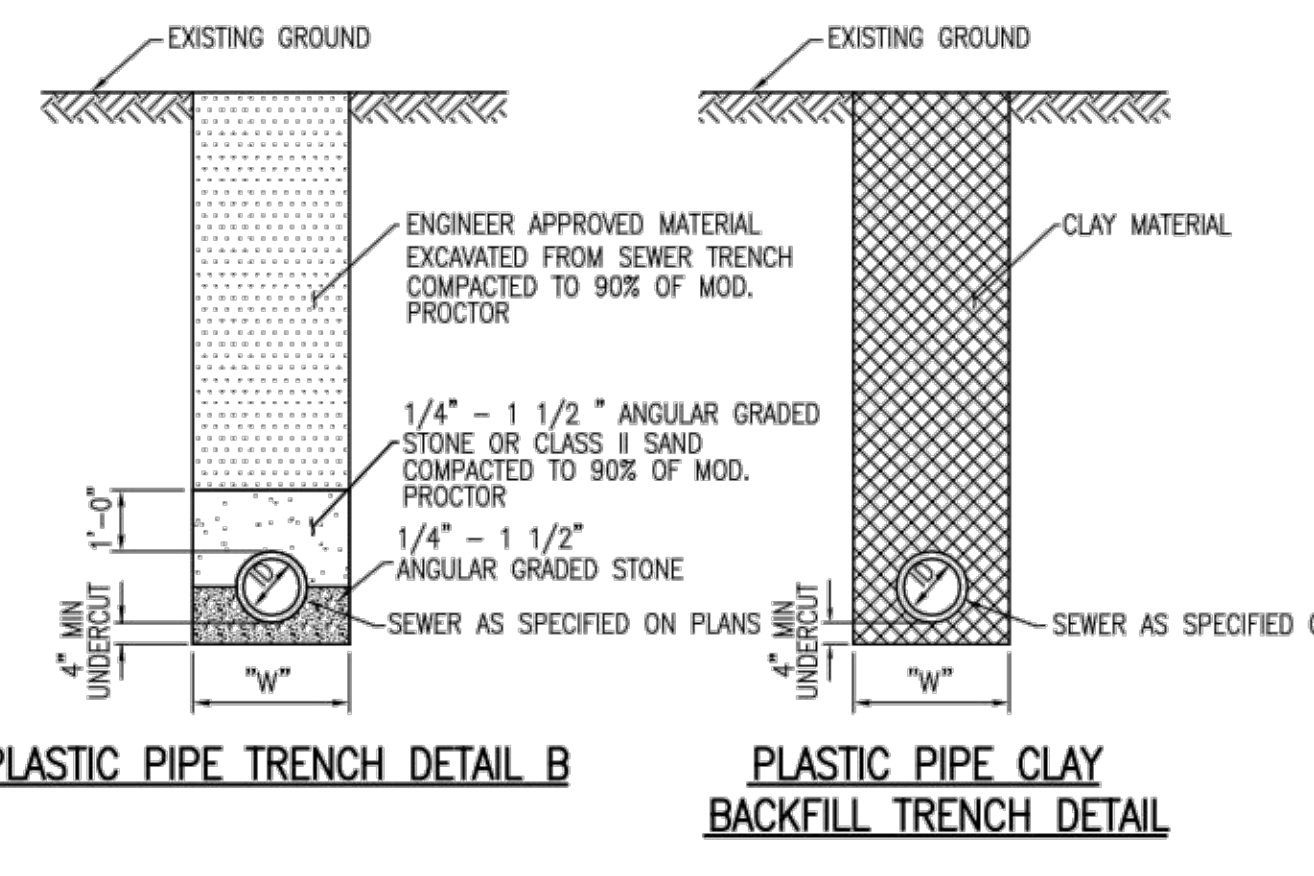
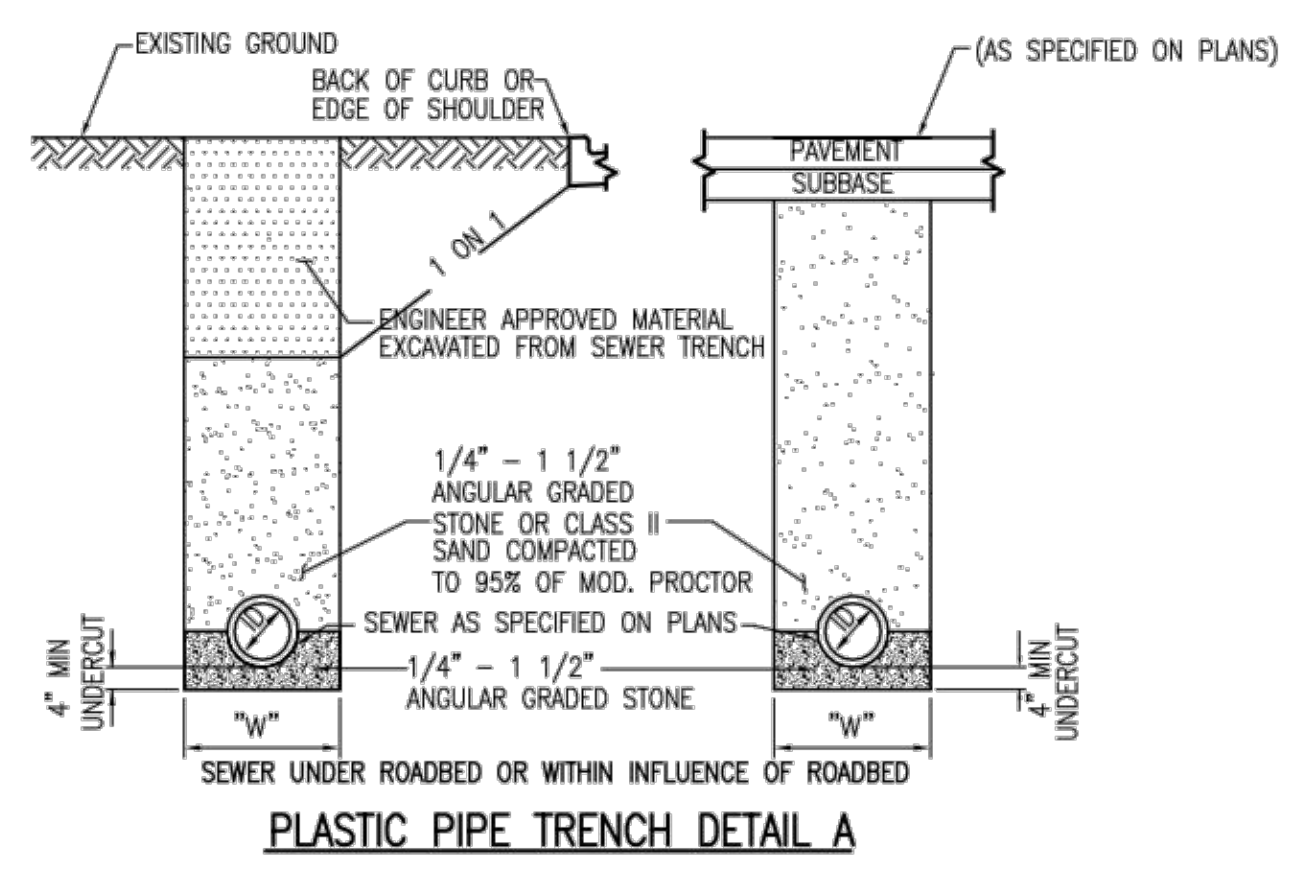
SANITARY LEAD CROSSING TABLE

SEE WATERMAIN PROFILES FOR WATERMAIN DIPPING AT L9 AND L10 LEADS CROSSING

	CROSSING PIPE 1	CROSSING PIPE 2	Clearance
L9	8" WM T/P= 807.50	6" SAN B/P= 809.10	1.60
L10	8" WM T/P= 809.06	6" SAN B/P= 810.70	1.64
L11	8" WM B/P= 812.93	6" SAN T/P= 809.50	3.43
L12	8" WM B/P= 813.65	6" SAN T/P= 810.15	3.50
L13	8" WM B/P= 816.68	6" SAN T/P= 813.83	2.85
L14	8" WM B/P= 817.31	6" SAN T/P= 814.64	2.67
L15	8" WM B/P= 817.93	6" SAN T/P= 814.45	3.48
L16	8" WM B/P= 818.56	6" SAN T/P= 815.16	3.41
L17	8" WM B/P= 819.18	6" SAN T/P= 815.96	3.22
L18	8" WM B/P= 819.85	6" SAN T/P= 816.76	3.10
L19	8" WM B/P= 820.53	6" SAN T/P= 817.55	2.98
L20	8" WM B/P= 820.56	6" SAN T/P= 817.14	3.42
L21	8" WM B/P= 819.58	6" SAN T/P= 816.95	2.63
L22	8" WM B/P= 818.39	6" SAN T/P= 815.72	2.67
L23	8" WM B/P= 817.31	6" SAN T/P= 814.52	2.79
L24	8" WM B/P= 816.27	6" SAN T/P= 813.33	2.95
L25	8" WM B/P= 815.62	6" SAN T/P= 812.04	3.58
L26	8" WM B/P= 815.61	6" SAN T/P= 812.95	2.66
L27	8" WM B/P= 816.17	6" SAN T/P= 812.75	3.42
L35	8" WM B/P= 813.28	6" SAN T/P= 810.89	2.40
L36	8" WM B/P= 814.38	6" SAN T/P= 811.64	2.75
L37	8" WM B/P= 815.10	6" SAN T/P= 812.40	2.70
L38	8" WM B/P= 818.76	6" SAN T/P= 813.19	5.58
L39	8" WM B/P= 816.54	6" SAN T/P= 812.89	3.65
L40	8" WM B/P= 816.84	6" SAN T/P= 813.49	3.35
L41	8" WM B/P= 815.47	6" SAN T/P= 813.25	2.22
L42	8" WM B/P= 813.15	6" SAN T/P= 811.04	2.11
L43	8" WM B/P= 810.87	6" SAN T/P= 807.87	3.01
L44	8" WM B/P= 809.02	6" SAN T/P= 806.86	2.37
L63	8" WM B/P= 821.68	6" SAN T/P= 818.31	3.38
L64	8" WM B/P= 822.84	6" SAN T/P= 820.07	2.78
L65	8" WM B/P= 822.51	6" SAN T/P= 819.59	2.92
L66	8" WM B/P= 821.30	6" SAN T/P= 818.34	2.97
L67	8" WM B/P= 817.38	6" SAN T/P= 814.42	2.96
L68	8" WM B/P= 816.23	6" SAN T/P= 813.18	3.05
L69	8" WM B/P= 815.08	6" SAN T/P= 811.38	3.70
L70	8" WM B/P= 813.92	6" SAN T/P= 810.48	3.44
L71	8" WM B/P= 812.67	6" SAN T/P= 809.51	3.17
L72	8" WM B/P= 811.52	6" SAN T/P= 807.61	3.92
L73	8" WM B/P= 810.36	6" SAN T/P= 806.71	3.66
L74	8" WM B/P= 809.24	6" SAN T/P= 805.83	3.41
L75	8" WM B/P= 808.34	6" SAN T/P= 804.91	3.43
L94	8" WM B/P= 806.66	6" SAN T/P= 803.77	2.89
L95	8" WM B/P= 805.52	6" SAN T/P= 802.58	2.94
L96	8" WM B/P= 805.16	6" SAN T/P= 802.49	2.68
L97	8" WM B/P= 805.31	6" SAN T/P= 803.29	2.02
L98	8" WM B/P= 806.09	6" SAN T/P= 803.08	3.01
L99	8" WM B/P= 806.14	6" SAN T/P= 802.90	3.25
L100	8" WM B/P= 804.75	6" SAN T/P= 801.58	3.18
L101	8" WM B/P= 804.67	6" SAN T/P= 801.47	3.20
L102	8" WM B/P= 805.11	6" SAN T/P= 802.17	2.94
L103	8" WM B/P= 805.84	6" SAN T/P= 802.98	2.87
L104	8" WM B/P= 806.64	6" SAN T/P= 803.66	2.98
L105	8" WM B/P= 807.35	6" SAN T/P= 804.48	2.88
L106	8" WM B/P= 808.08	6" SAN T/P= 805.28	2.80
L107	8" WM B/P= 808.80	6" SAN T/P= 806.09	2.71
L108	8" WM B/P= 809.64	6" SAN T/P= 806.73	2.92
L109	8" WM B/P= 810.25	6" SAN T/P= 807.57	2.69
L110	8" WM B/P= 811.17	6" SAN T/P= 808.29	2.89
L121	8" WM B/P= 803.11	6" SAN T/P= 801.53	1.58
L122	8" WM B/P= 803.11	6" SAN T/P= 801.37	1.75
L123	8" WM B/P= 803.62	6" SAN T/P= 801.02	2.61
L146	8" WM B/P= 808.44	6" SAN T/P= 804.76	3.68
L147	8" WM B/P= 807.89	6" SAN T/P= 804.16	3.53
L148	8" WM B/P= 806.78	6" SAN T/P= 803.66	3.12
L149	8" WM B/P= 805.86	6" SAN T/P= 803.33	2.53
L150	8" WM B/P= 804.94	6" SAN T/P= 802.00	2.94
L151	8" WM B/P= 803.96	6" SAN T/P= 800.64	3.33
L152	8" WM B/P= 802.78	6" SAN T/P= 798.32	4.46
L153	8" WM B/P= 800.54	6" SAN T/P= 796.62	3.93
L154	8" WM B/P= 798.52	6" SAN T/P= 794.88	3.64
L155	8" WM B/P= 796.36	6" SAN T/P= 793.52	2.84
L156	8" WM B/P= 794.47	6" SAN T/P= 791.58	2.90
L157	8" WM B/P= 792.71	6" SAN T/P= 789.53	3.19
L158	8" WM B/P= 791.06	6" SAN T/P= 787.73	3.34
L159	8" WM B/P= 789.40	6" SAN T/P= 785.93	3.48
L160	8" WM B/P= 788.13	6" SAN T/P= 784.93	3.21
L161	8" WM B/P= 786.81	6" SAN T/P= 783.98	2.84
L162	8" WM B/P= 785.67	6" SAN T/P= 782.46	3.22
L163	8" WM B/P= 784.42	6" SAN T/P= 781.85	2.58
L164	8" WM B/P= 783.16	6" SAN T/P= 780.24	2.93
L165	8" WM B/P= 781.91	6" SAN T/P= 778.96	2.95
L181	8" WM B/P= 803.32	6" SAN T/P= 801.34	1.99
L182	8" WM B/P= 804.67	6" SAN T/P= 802.53	2.15
L183	8" WM B/P= 805.80	6" SAN T/P= 802.72	3.08
L184	8" WM B/P= 806.16	6" SAN T/P= 803.11	3.06
L185	8" WM B/P= 805.30	6" SAN T/P= 802.30	3.00
L186	8" WM B/P= 804.87	6" SAN T/P= 801.49	3.38
L187	8" WM B/P= 805.06	6" SAN T/P= 802.59	2.48
L188	8" WM B/P= 805.88	6" SAN T/P= 802.88	3.00
L189	8" WM B/P= 806.18	6" SAN T/P= 803.17	3.02
L190	8" WM B/P= 805.76	6" SAN T/P= 802.36	3.40
L191	8" WM B/P= 805.32	6" SAN T/P= 802.55	2.77
L192	8" WM B/P= 805.26	6" SAN T/P= 802.65	2.62
L193	8" WM B/P= 805.87	6" SAN T/P= 802.94	2.93
L194	8" WM B/P= 807.07	6" SAN T/P= 804.12	2.95
L195	8" WM B/P= 808.27	6" SAN T/P= 805.32	2.95
L196	8" WM B/P= 809.23	6" SAN T/P= 806.59	2.64
L197	8" WM B/P= 810.26	6" SAN T/P= 807.72	2.54
L198	8" WM B/P= 811.21	6" SAN T/P= 807.98	3.23
L199	8" WM B/P= 812.15	6" SAN T/P= 809.08	3.07
L200	8" WM B/P= 813.32	6" SAN T/P= 810.43	2.89
L201	8" WM B/P= 813.30	6" SAN T/P= 810.48	2.82
L202	8" WM B/P= 812.73	6" SAN T/P= 809.85	2.89
L203	8" WM B/P= 811.99	6" SAN T/P= 809.05	2.95
L156	8" ST B/P= 796.22	6" SAN T/P= 791.73	4.49
L157	8" ST B/P= 794.98	6" SAN T/P= 789.66	5.32
L158	8" ST B/P= 792.98	6" SAN T/P= 787.85	5.12
L159	8" ST B/P= 791.78	6" SAN T/P= 786.05	5.73
L160	8" ST B/P= 790.48	6" SAN T/P= 785.04	5.44
L161	8" ST B/P= 789.26	6" SAN T/P= 784.12	5.15
L162	8" ST B/P= 788.02	6" SAN T/P= 782.58	5.45
L163	8" ST B/P= 786.82	6" SAN T/P= 781.97	4.85
L164	8" ST B/P= 785.42	6" SAN T/P= 780.36	5.06
L165	8" ST B/P= 784.22	6" SAN T/P= 779.08	5.13

SANITARY LEAD TABLE

* L1	808.63	24	1.04%	6	817.55	0	808.88
L2	808.00	24	1.04%	6	819.55	0	808.25
L3	807.74	24	1.04%	6	820.00	0	807.99
L4	807.49	24	1.04%	6	821.00	2	809.74
L5	807.14	28	1.04%	6	821.70	3	810.43
L6	806.89	28	1.04%	6	822.40	4	811.18
L7	806.64	28	1.04%	6	823.10	5	811.93
L8	806.40	28	1.04%	6	823.80	6	812.69
* L9	808.65	72	1.04%	6	817.00	0	809.40
* L10	808.23	74	1.04%	6	818.20	2	811.00
L11	807.43	74	1.04%	6	821.10	1	809.20
L12	807.08	74	1.04%	6	821.80	2	809.85
L13	805.76	74	1.04%	6	824.90	7	813.53
L14	805.57	74	1.04%	6	825.50	8	814.34
L15	805.38	74	1.04%	6	826.10	8	814.15
L16	805.09	74	1.04%	6	826.70	9	814.86
L17	804.89	74	1.04%	6	827.40	10	815.66
L18	804.69	74	1.04%	6	828.10	11	816.46
L19	804.48	74	1.04%	6	828.70	12	817.25
L20	804.07	74	1.04%	6	828.70	12	816.84
L21	803.88	74	1.04%	6	827.90	12	816.65
L22	803.65	74	1.04%	6	826.70	11	815.42
L23	803.45	74	1.04%	6	825.70	10	814.22
L24	803.26	74	1.04%	6	824.60	9	813.03
L25	802.97	74	1.04%	6	823.70	8	811.74
L26	802.88	74	1.04%	6	823.70	9	812.65
L27	802.68	74	1.04%	6	824.30	9	812.57
L28	805.27	29	1.04%	6	823.60	7	812.57
L29	805.51	29	1.04%	6	822.80	5	810.81
L30	806.18	31	1.04%	6	820.20	2	808.50
L31	806.43	26	1.04%	6	820.00	0	806.71
L32	806.71	24	1.04%	6	818.90	0	806.96
L33	807.08	24	1.04%	6	818.55	0	807.33
L34	807.68	24	1.04%	6	819.30	0	807.93
L35	805.82	74	1.04%	6	821.70	4	810.59
L36	805.58	73	1.04%	6	822.50	5	811.34
L37	805.34	74	1.04%	6	823.30	6	812.10
L38	805.12	74	1.04%	6	824.00	7	812.89
L39	803.82	74	1.04%	6	824.70	8	812.59
L40	803.42	74	1.04%	6	825.10	9	813.19
L41	802.18	74	1.04%	6	824.10	10	812.95
L42	801.99	72	1.04%	6	821.90	8	810.74
L43	801.80	74	1.04%	6	819.50	5	807.57
L44	801.59	74	1.04%	6	817.65	4	806.36
L45	801.40	74	1.04%	6	816.10	2	804.17
L46	802.68	29	1.04%	6	824.40	10	812.98
L47	802.87	29	1.04%	6	823.80	9	812.17
L48	802.98	29	1.04%	6	823.75	9	812.28
L49	803.26	29	1.04%	6	824.60	10	813.57
L50	803.46	29	1.04%	6	825.60	10	813.76
L51	803.66	29	1.04%	6	826.70	11	814.96
L52	803.85	29	1.04%	6	827.70	12	816.15
L53	804.04	29	1.04%	6	828.70	13	817.34
L54	804.43	29	1.04%	6	828.80	13	817.73
L55	805.08	29	1.04%	6	826.80	10	815.38
L56	805.37	29	1.04%	6	826.20	9	814.67
L57	805.56	29	1.04%	6	825.50	8	813.87
L58	805.76	29	1.04%	6	824.90	7	813.06
L59	807.65	28	1.04%	6	824.70	5	812.95
L60	809.34	23	1.04%	6	825.70	5	814.58
L61	812.38	24	1.04%	6	826.60	3	815.63
L62	812.98	24	1.04%	6	827.50	3	816.23
L63	812.24	74	1.04%	6	830.10	5	818.01
L64	812.00	74	1.04%	6	831.00	7	819.12
L65	811.52	74	1.04%	6	830.60	7	818.29
L66	811.27	74	1.04%	6	829.60	6	818.04
L67	810.35	74	1.04%	6	825.50	3	814.12
L68	810.11	74	1.04%	6	824.60	2	812.88
L69	809.31	74	1.04%	6	823.50	1	811.08
L70	808.41	74	1.04%	6	822.30	1	810.18
L71	807.44	74	1.04%	6	821.10	1	809.21
L72	806.54	74	1.04%	6	819.90	0	807.31
L73	805.64	74	1.04%	6	818.70	0	806.41
L74	805.74	76	1.04%	6	817.60	0	805.53
L75	802.80	78	1.04%	6	816.30	1	804.



DIA OF SEWER	DIA OF DROP CONNECTION
8" & 10"	8"
12", 15" & 18"	12"
21", 24" & 30"	18"
36" & 42"	24"

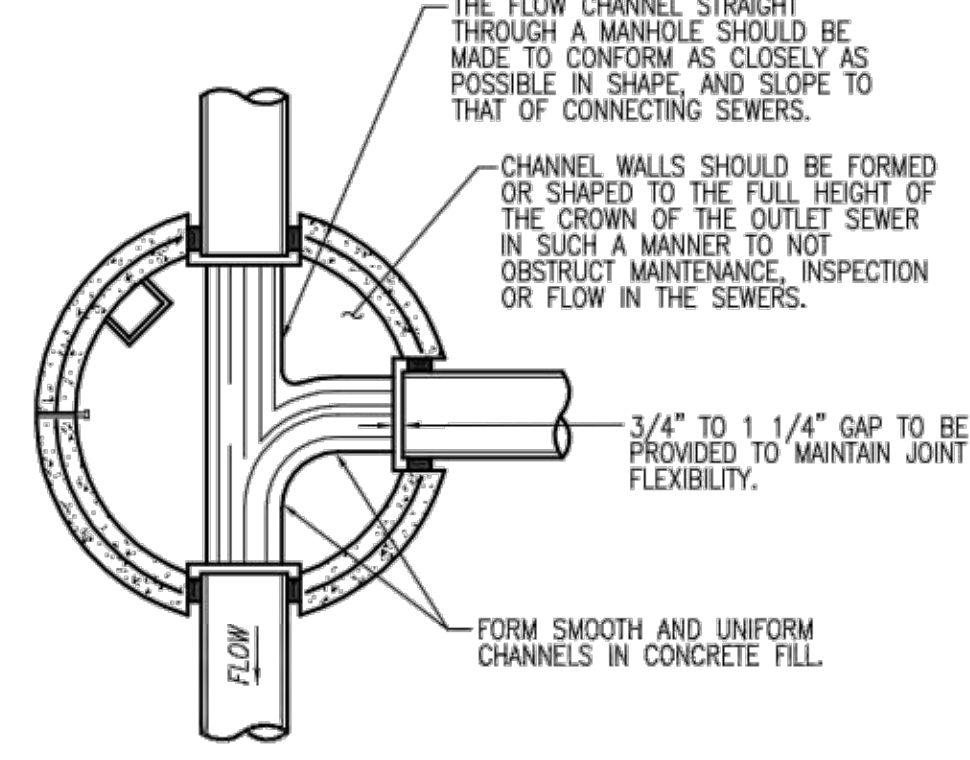
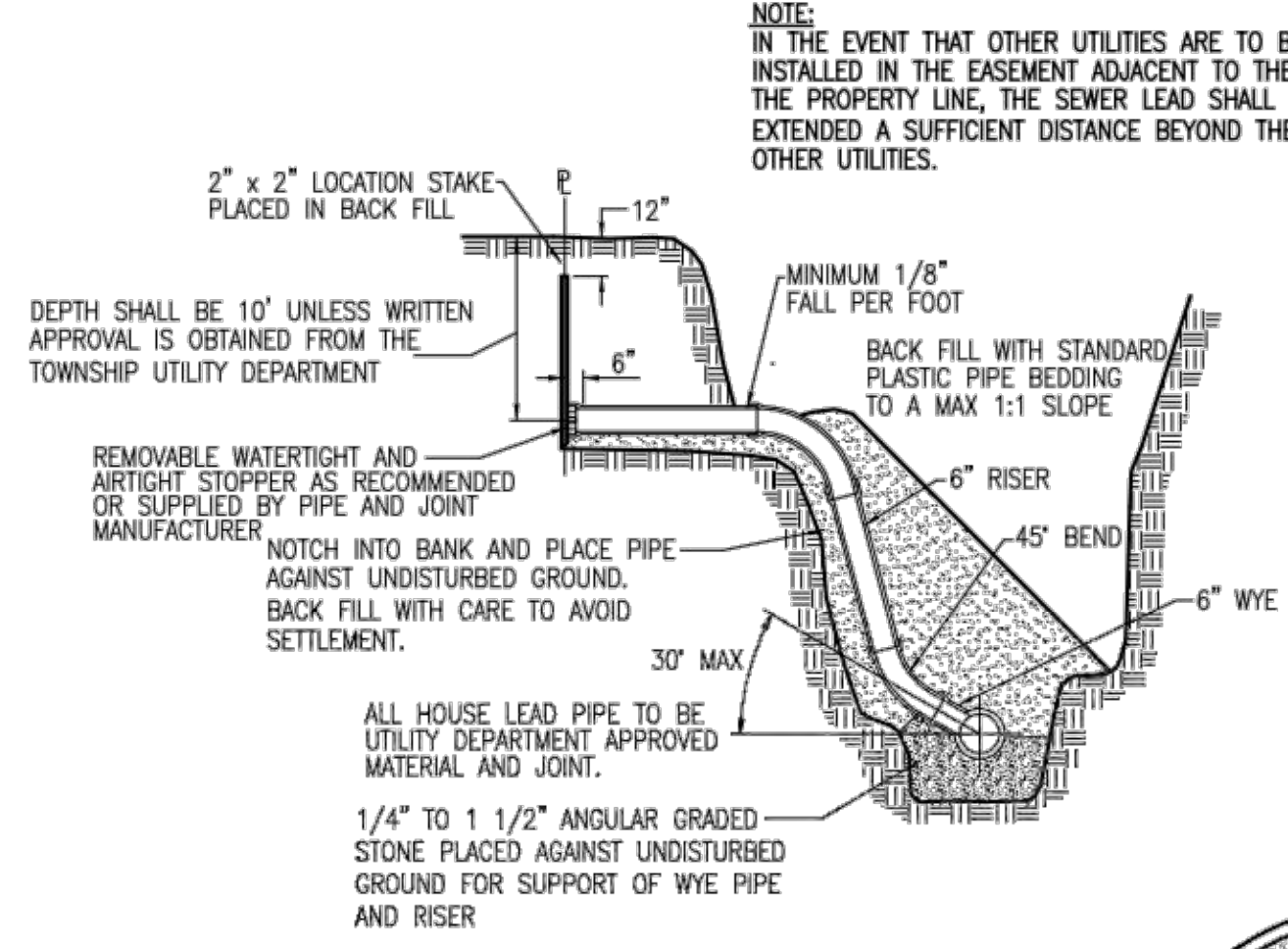
THE FOLLOWING ARE MINIMUM TRENCH WIDTHS:

L.D. PIPE SIZE (INCHES)	LESS THAN 18"	21"	24"	30"	36"
"W" TRENCH WIDTH (FEET)	3.0	3.5	4.0	5.0	6.0

L.D. PIPE SIZE (INCHES)	42"	48"	54"	60"	66"	72"	78"
"W" TRENCH WIDTH (FEET)	7.0	8.0	9.5	10.0	10.5	11.0	11.5

L.D. PIPE SIZE (INCHES)	84"	90"	96"	102"	108"
"W" TRENCH WIDTH (FEET)	12.0	12.5	13.0	13.5	14.0

ESTIMATED PAVEMENT REMOVAL WIDTH IS TO BE TRENCH WIDTH "W" PLUS 1' EACH SIDE OF THE TRENCH (6" MINIMUM).

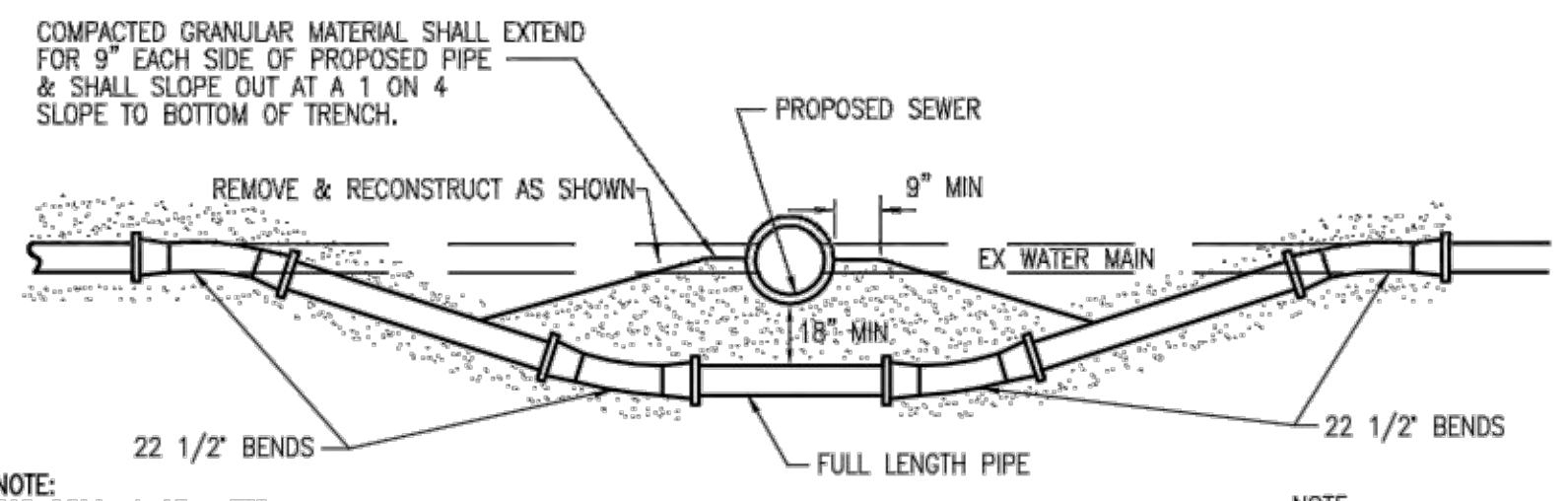
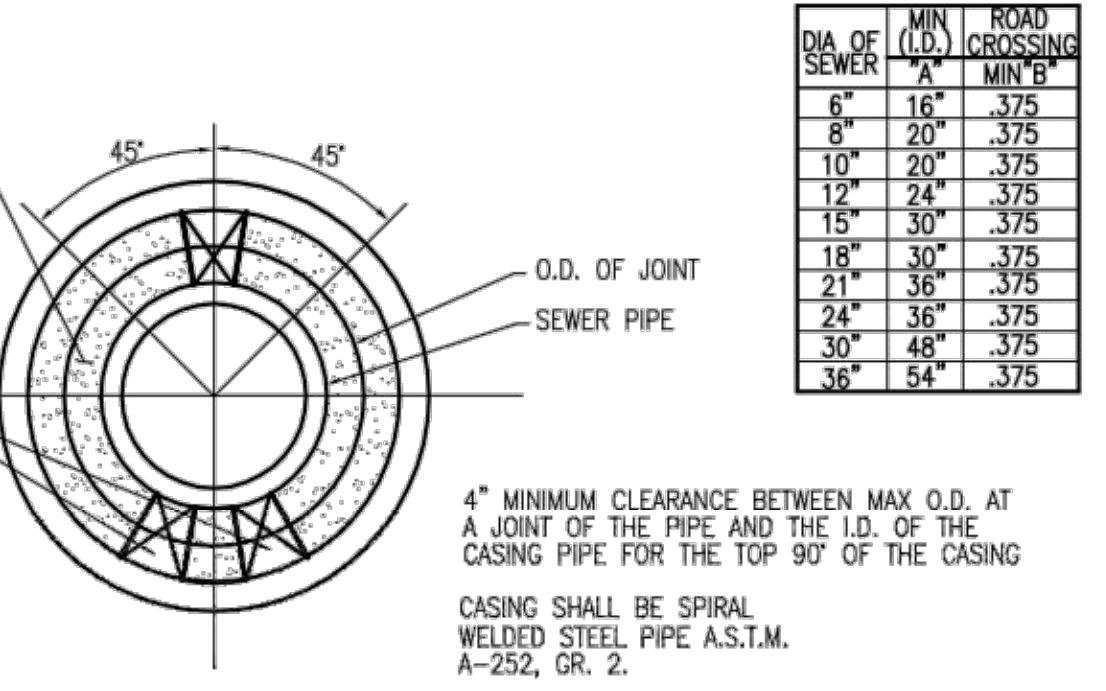


DETAILS ARE TYPICAL FOR SEWER PIPE CONSTRUCTED IN CASING PIPE. 8" BRICK BULKHEAD SHALL BE PLACED AT EACH END OF CASING PIPE AFTER PIPE INSTALLATION.

SEWER PIPE SHALL BE STRAPPED TO PRE-MANUFACTURED SKIDS (WOOD SKIDS WILL NOT BE ACCEPTED) SKIDS SHALL BE 85% OF PIPE LENGTH (TYP).

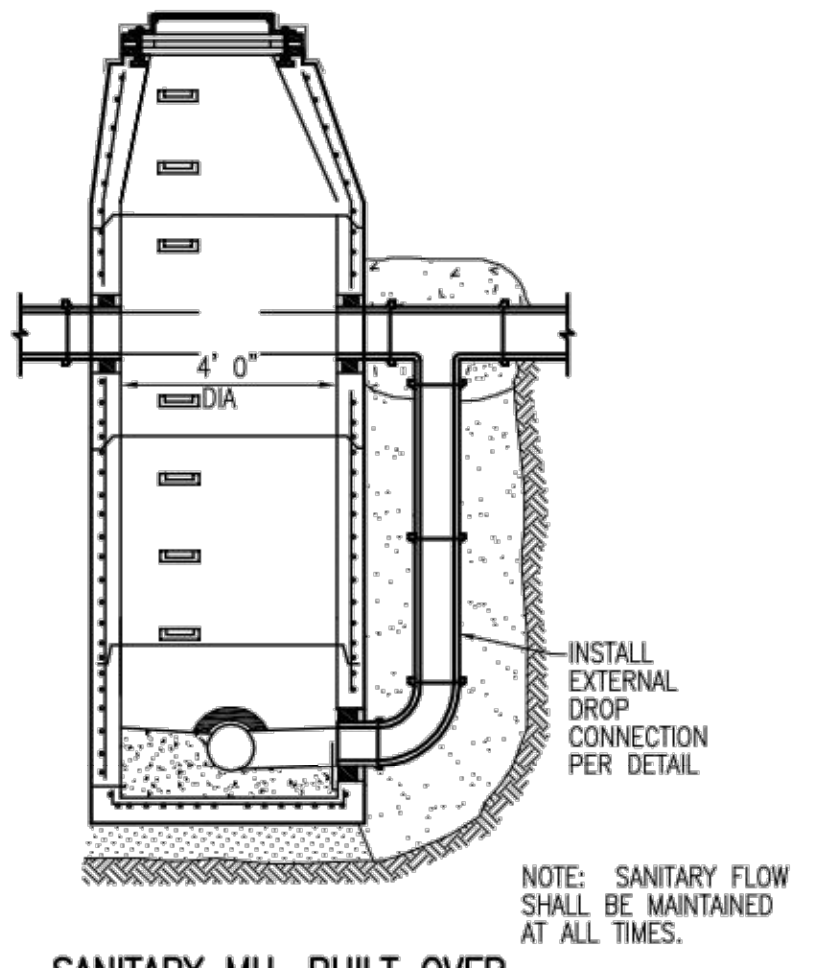
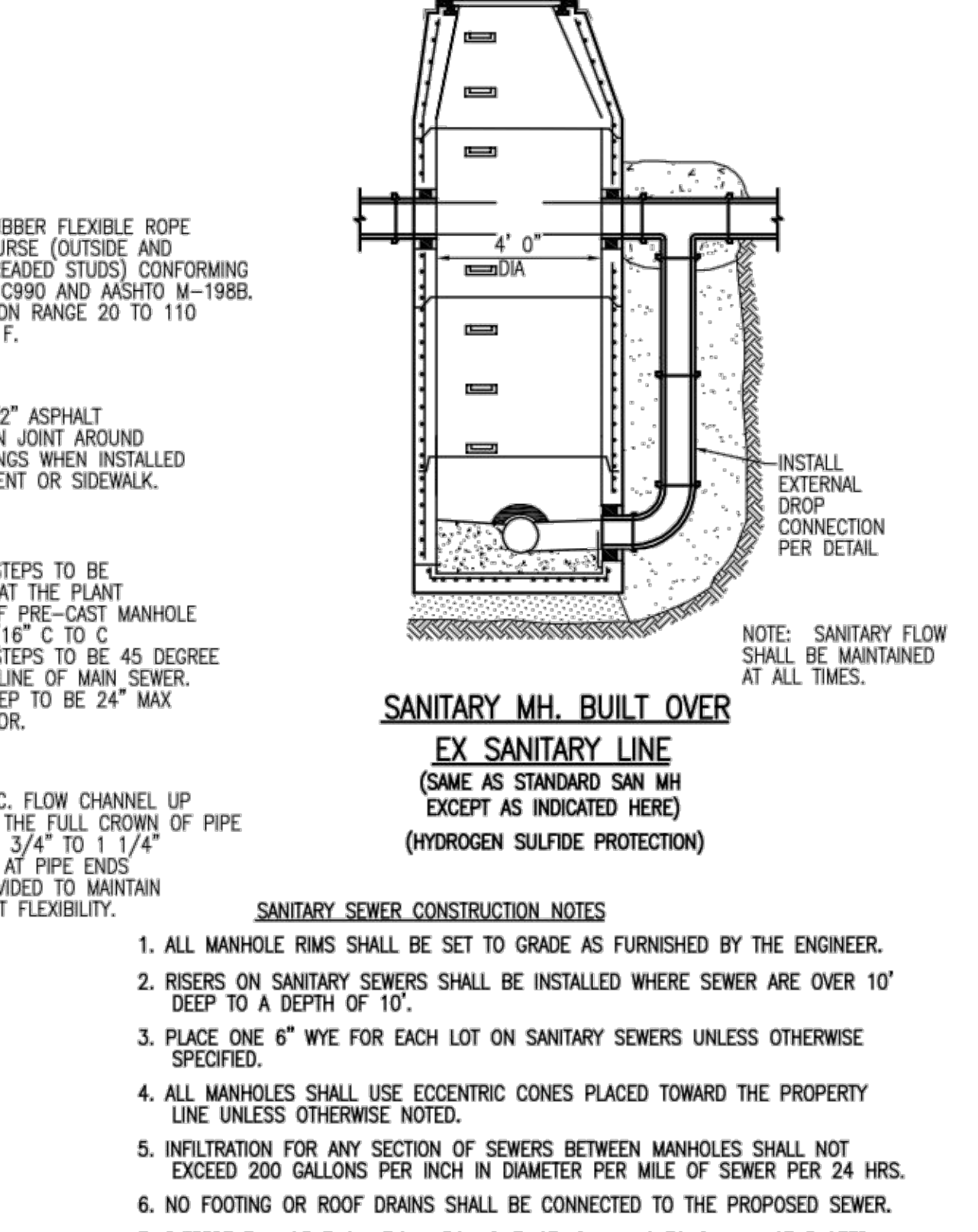
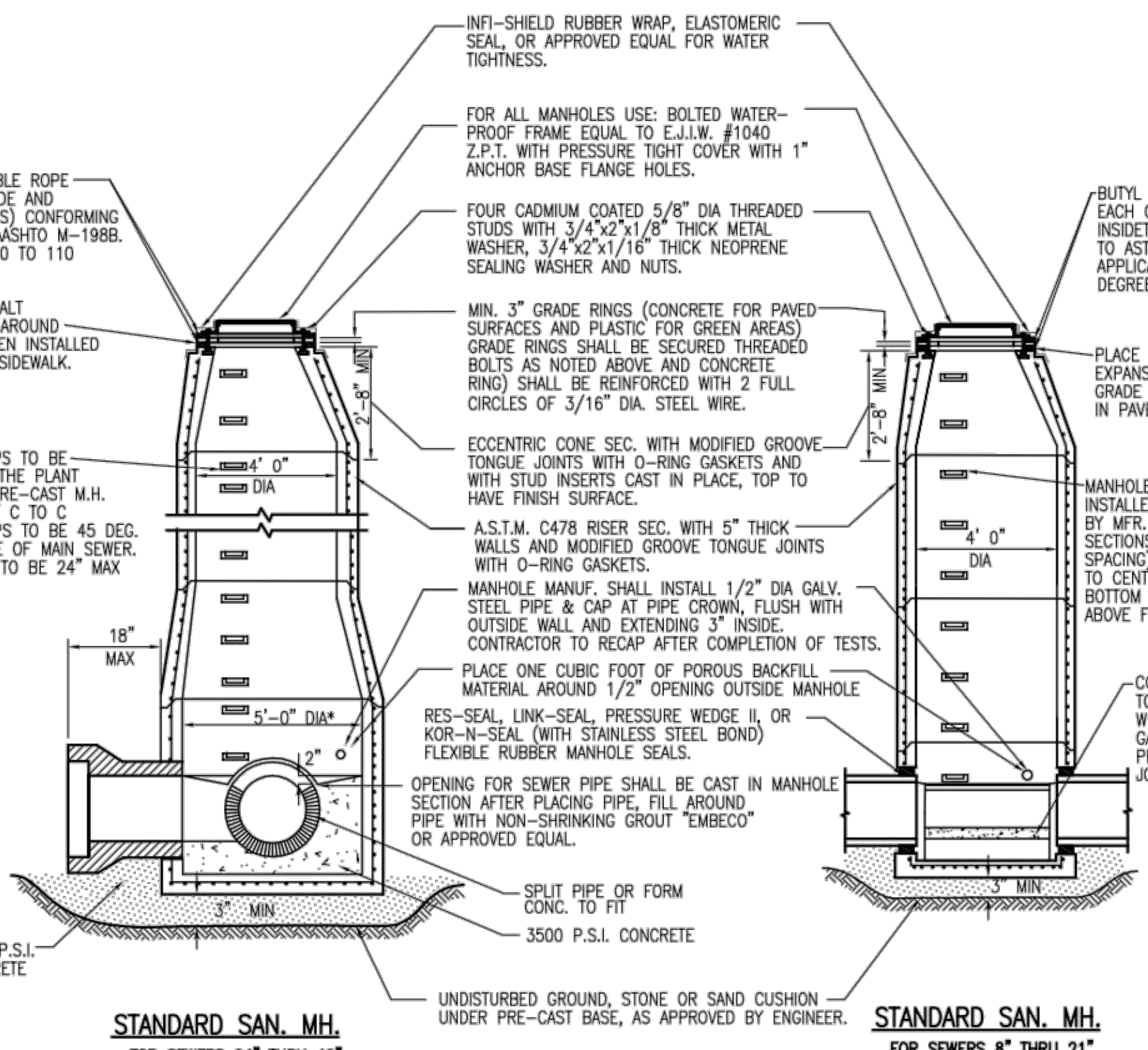
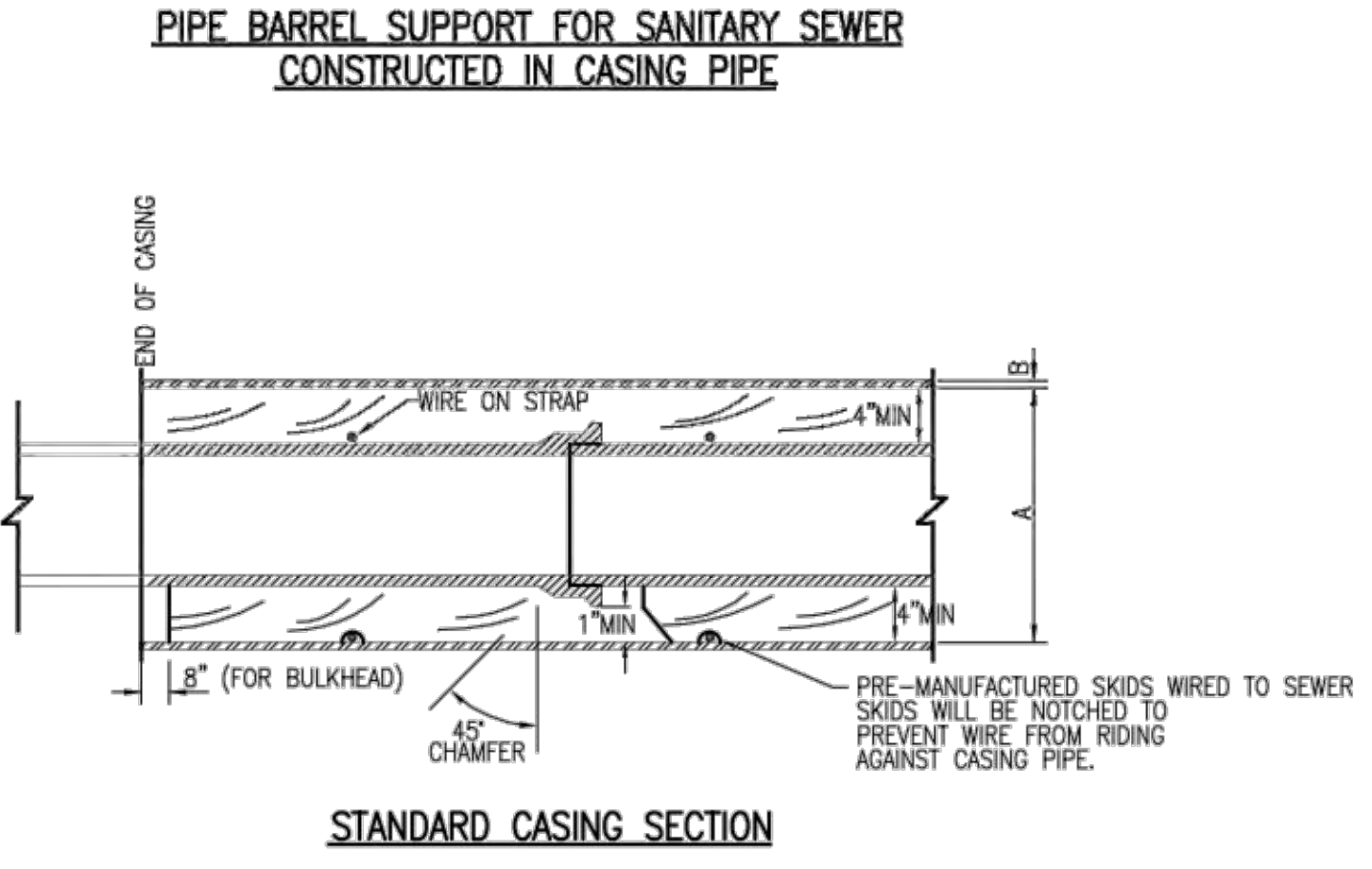
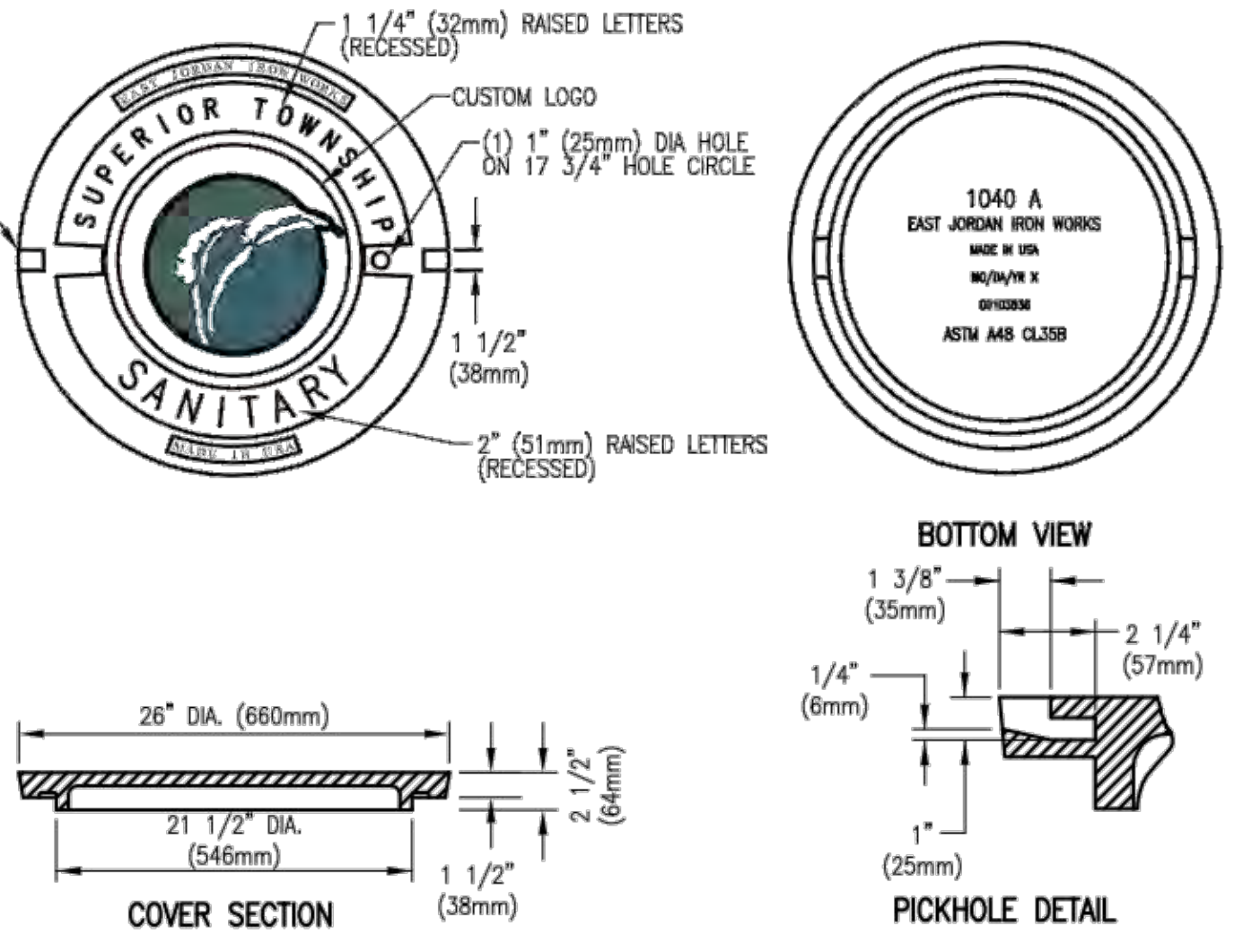
THE THREE SKIDS SHALL BE PLACED TO PREVENT THE CARRIER PIPE FROM ROLLING OVER OR SHIFTING.

THE CONTRACTOR SHALL SUBMIT IN WRITING THE DETAILS OF THE APPROPRIATE PIPE CASING INSTALLATION FOR THE REVIEW AND APPROVAL BY THE ENGINEER BEFORE INSTALLATION OF ANY CASING STARTS.



NOTE: FOR BEDDING OF WATER MAIN SEE STANDARD SAND ENCASUREMENT DETAIL "B" THIS SHEET.

NOTE: FOR ANCHORAGE DETAILS AT BENDS SEE WATER DETAIL SHEET - W.D.1



- SANITARY SEWER CONSTRUCTION NOTES**
- ALL MANHOLE RIMS SHALL BE SET TO GRADE AS FURNISHED BY THE ENGINEER.
 - RISERS ON SANITARY SEWERS SHALL BE INSTALLED WHERE SEWER ARE OVER 10' DEEP TO A DEPTH OF 10'.
 - PLACE ONE 6" WYE FOR EACH LOT ON SANITARY SEWERS UNLESS OTHERWISE SPECIFIED.
 - ALL MANHOLES SHALL USE ECCENTRIC CONES PLACED TOWARD THE PROPERTY LINE UNLESS OTHERWISE NOTED.
 - INFILTRATION FOR ANY SECTION OF SEWERS BETWEEN MANHOLES SHALL NOT EXCEED 200 GALLONS PER INCH IN DIAMETER PER MILE OF SEWER PER 24 HRS.
 - NO FOOTING OR ROOF DRAINS SHALL BE CONNECTED TO THE PROPOSED SEWER.
 - DIFFERENTIAL OF EXCAVATION AROUND EXISTING MANHOLES SHALL NOT EXCEED 6 FEET.
 - NO CONNECTION RECEIVING STORM WATER, SURFACE WATER, OF GROUND WATER SHALL BE MADE TO SANITARY SEWERS.
 - ALL SEWERS SHALL BE SUBJECT TO TELEVISION INSPECTION AND AIR INFILTRATION OR EXFILTRATION TESTS, OR A COMBINATION OF SAME WITHIN 30 DAY OF BACKFILL, PRIOR TO ACCEPTANCE. ALL SEWERS OVER 24" DIAMETER OR SMALLER, WHERE THE GROUND WATER LEVEL ABOVE THE TOP OF THE SEWER IS OVER 2 FEET, SHALL BE SUBJECTED TO INFILTRATION TESTS. ALL SEWERS OF 24" DIAMETER OR LESS, WHERE THE GROUND WATER LEVEL ABOVE THE TOP OF THE SEWER IS 2 FEET OR LESS, SHALL BE SUBJECTED TO AIR TESTS OR EXFILTRATION TESTS.
 - ALL SEWER SYSTEM CONSTRUCTION SHALL CONFORM TO THE CURRENT STANDARDS AND GENERAL SPECIFICATIONS OF THE AGENCY OR AGENCIES HAVING JURISDICTION OF THE SEWER SYSTEM AND CONSTRUCTION AREA.
 - ALL SEWERS SHALL BE TELEVISED, WITH TEST RESULTS APPROVED BY THE TOWNSHIP UTILITY DEPARTMENT PRIOR TO PLACING THE SEWER IN SERVICE.
 - NO SEWER SHALL BE CONSTRUCTED LESS THAN 8" IN DIAMETER.
 - ALL STUBS SHALL HAVE A WATER AND AIR TIGHT BULKHEAD.
 - ALL ELEVATIONS ARE BASED ON NAVD 88 (ORIGINAL).
 - THE MINIMUM DIAMETER OF MANHOLES SHALL BE 48 INCHES, LARGER DIAMETERS ARE PREFERABLE FOR LARGER DIAMETER SEWERS. A MINIMUM ACCESS DIAMETER OF 22 INCHES SHALL BE PROVIDED.
 - CONTRACTOR SHALL PERFORM 9 POINT MANDREL TESTING PER SECTION X, OPEN CUT SEWERS OF THE SPECIFICATIONS.
 - SANITARY SEWER SHALL BE COMPOSITE ABS OR PVC TRUSS PIPE CONFORMING TO REQUIREMENTS OF ASTM D-2680 OR, PVC PIPE CONFORMING TO ASTM D-3034 (SDR 26) MAY BE USED WHERE DEPTH IS LESS THAN 15'.
 - JOINTS FOR PLASTIC PIPE SHALL BE PUSH-ON TYPE OR IN SPECIAL APPLICATIONS SOLVENT JOINTS, WHERE SPECIFIED, SHALL CONFORM TO ASTM D-3112 AND F-477. SOLVENT-CEMENTED JOINTS, WHERE SPECIFIED, SHALL CONFORM TO ASTM D-2855.
 - JOINTS ON TRUSS PIPE SHALL CONSIST OF ABS PLASTIC COUPLINGS CHEMICALLY CEMENTED TO THE ENDS OF THE PIPE BEING CONNECTED. SOLVENTS AND METHODS USED IN MAKING THE CHEMICAL BOND SHALL BE IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTRUCTIONS, AND THE INSTALLATION TECHNIQUE SHALL CONFORM TO ASTM D-2321, OR LATEST REVISION.
 - MANHOLE STEPS TO BE PLASTIC COATED MEETING THE REQUIREMENTS IN ASTM D2146, TYPE II, GRADE 49108, M.A. INDUSTRIES P.S.I. POLYPROPYLENE, OR E.J.L.W. 8502.
 - A VIDEOTAPE (WITH LOG AND LEAD LOCATIONS) SHALL BE SUBMITTED AND APPROVED BY THE TOWNSHIP PRIOR TO THE FINAL ACCEPTANCE. SAID VIDEOTAPE SHALL BE PERFORMED PRIOR TO FINAL ACCEPTANCE OF THE SYSTEM.
 - PRIOR TO ANY EXCAVATION, THE CONTRACTOR SHALL TELEPHONE MISS DIG (800-482-7171) A MINIMUM OF 30 DAYS AFTER DAYS AFTER COMPLETION OF BACKFILL. FOR THE LOCATION IF UNDER GROUND PIPELINE AND CABLE FACILITIES AND SHALL ALSO NOTIFY REPRESENTATIVES.
 - MINIMUM DEPTH TO TOP OF PIPE SHALL BE 4 FEET.
 - THE MAXIMUM DEPTH TO INVERT OF ANY SANITARY SEWER SHALL NOT EXCEED THE DEPTH RECOMMENDED BY THE MANUFACTURER FOR EACH SIZE AND CLASS AND PIPE.
 - INLET AND OUTLET PIPES SHALL BE JOINED TO THE MANHOLE WITH A GASKETED FLEXIBLE WATER TIGHT CONNECTION THAT ALLOWS DIFFERENTIAL SETTLEMENT OF THE PIPE AND MANHOLE WALL TO TAKE PLACE.
 - A BENCH SHALL BE PROVIDED ON EACH SIDE OF ANY MANHOLE CHANNEL WHEN THE PIPE DIAMETER IS LESS THAN THE MANHOLE DIAMETER. THE BENCH SHOULD BE SLOPED NO LESS THAN 1/2 INCH PER FOOT. NO LATERAL SEWER, SERVICE CONNECTION, OR DROP MANHOLE PIPE SHALL DISCHARGE ONTO THE SURFACE OF THE BENCH.
 - WHERE WORK IS TO BE PERFORMED IN THE VICINITY OF A SUPERIOR TOWNSHIP MAIN, CONTRACTOR SHALL NOTIFY THE SUPERIOR TOWNSHIP UTILITY DEPARTMENT AT (734) 480-5500 AT LEAST 3 WORKING DAYS PRIOR TO START OF CONSTRUCTION AS WELL AS TOWNSHIP ENGINEER TO SCHEDULE INSPECTION.
 - AN ELASTOMERIC SEAL OR RUBBER WRAP SHALL BE PLACED AROUND EACH MANHOLE FRAME AND CONE SECTION FOR WATER TIGHTNESS.

SANITARY SEWER ACCEPTANCE TESTS

GENERAL

ALL SANITARY SEWERS SHALL BE SUBJECTED TO INFILTRATION, EXFILTRATION OR LOW PRESSURE AIR TESTS, OR A COMBINATION THEREOF PRIOR TO FINAL ACCEPTANCE BY THE TOWNSHIP. IN ADDITION, ALL PVC AND ABS PLASTIC SEWERS SHALL BE SUBJECTED TO DEFLECTION TESTING BY MEANS OF A NINE-POINT MANDREL DEFLECTION TEST.

THE TOWNSHIP'S INSPECTOR SHALL BE PRESENT FOR ALL TESTING OPERATIONS. IF TESTING IS TO BE DONE BY THE CONTRACTOR, ONLY PROPERLY TRAINED PERSONNEL SHALL BE ALLOWED TO PERFORM THE TESTING WORK. IF TESTING IS TO BE DONE BY MUNICIPAL AGENCY WORK FORCES, THEN THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING WITH THE INSPECTOR IN ORDER TO SCHEDULE THE TESTING.

IN THE EVENT THAT THE SEWER PIPE FAILS ANY OF THE REQUIRED TESTS, THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIRING THE PIPE AND REPEATING THE TEST UNTIL ACCEPTABLE RESULTS ARE ACHIEVED. THE METHOD OF TESTING AND MEASUREMENT SHALL BE APPROVED BY THE TOWNSHIP. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY EQUIPMENT AND LABOR FOR MAKING THE TESTS.

INFILTRATION TEST

ALL SANITARY SEWERS THAT ARE OVER TWENTY-FOUR (24) INCHES IN DIAMETER SHALL BE SUBJECTED TO AN INFILTRATION TEST. ALSO, ALL SANITARY SEWERS THAT ARE TWENTY-FOUR (24) INCHES IN DIAMETER AND SMALLER AND WHERE THE GROUND WATER LEVEL IS MORE THAN TWO (2) FEET ABOVE THE TOP OF THE SEWER SHALL BE SUBJECTED TO AN INFILTRATION TEST.

THE INFILTRATION RATE FOR ALL SANITARY SEWERS SHALL NOT EXCEED A MAXIMUM OF TWO HUNDRED (200) GALLONS PER INCH DIAMETER PER MILE OF SEWER PER TWENTY-FOUR (24) HOURS.

LOW PRESSURE AIR TEST

ALL SANITARY SEWERS THAT ARE TWENTY-FOUR (24) INCHES IN DIAMETER OR SMALLER AND WHERE THE GROUND WATER LEVEL IS TWO (2) FEET OR LESS ABOVE THE TOP OF THE SEWER SHALL BE SUBJECTED TO A LOW PRESSURE AIR TEST.

THE PROCEDURE FOR AIR TESTING OF SEWERS SHALL BE AS FOLLOWS:

THE SEWER LINE SHALL BE TESTED IN INCREMENTS BETWEEN MANHOLES. THE LINE SHALL BE CLEANED AND PLUGGED AT EACH MANHOLE. SUCH PLUGS SHALL BE DESIGNED TO HOLD AGAINST THE TEST PRESSURE AND SHALL PROVIDE AN AIRTIGHT SEAL. ONE OF THE PLUGS SHALL HAVE AN ORIFICE THROUGH WHICH AIR CAN BE INTRODUCED INTO THE SEWER. AN AIR SUPPLY LINE SHALL BE CONNECTED TO THE ORIFICE. THE SUPPLY LINE SHALL BE FITTED WITH SUITABLE CONTROL VALVES AND A PRESSURE GAUGE FOR CONTINUALLY MEASURING THE AIR PRESSURE IN THE SEWER. THE PRESSURE GAUGE SHALL HAVE A MINIMUM DIAMETER OF THREE AND ONE-HALF (3-1/2) INCHES AND A RANGE OF 0 - 10 PSIG. THE GAUGE SHALL HAVE MINIMUM DIVISIONS OF 0-10 PSIG AND ACCURACY OF PLUS OR MINUS (+/-) 0.04 PSIG.

THE SEWER SHALL BE PRESSURIZED TO 4 PSIG GREATER THAN THE GREATEST BACK PRESSURE CAUSED BY GROUND WATER OVER THE TOP OF THE SEWER PIPE. AT LEAST TWO (2) MINUTES SHALL BE ALLOWED FOR THE AIR PRESSURE TO STABILIZE BETWEEN THREE AND ONE HALF (3.5) AND FOUR (4) PSIG. IF NECESSARY, AIR SHALL BE ADDED TO THE SEWER TO MAINTAIN A PRESSURE OF 3.5 PSIG OR GREATER.

AFTER THE STABILIZATION PERIOD, THE AIR SUPPLY CONTROL VALVE SHALL BE CLOSED SO THAT NO MORE AIR WILL ENTER THE SEWER. THE SEWER AIR PRESSURE SHALL BE NOTED AND TIMING FOR THE TEST BEGUN. THE TEST SHALL NOT BEGIN IF THE AIR PRESSURE IS LESS THAN THREE AND ONE HALF (3.5) PSIG, OR SUCH OTHER PRESSURE AS IS NECESSARY TO COMPENSATE FOR GROUND WATER LEVEL.

THE TIME REQUIRED FOR THE AIR PRESSURE TO DECREASE ONE (1.0) PSIG DURING THE TEST SHALL NOT BE LESS THAN THE TIME SHOWN IN THE FOLLOWING AIR TEST TABLES. THE CONTRACTOR SHALL USE THE APPROPRIATE TEST TABLE BASED UPON THE SEWER PIPE MATERIAL.

AIR TEST TABLE FOR VITRIFIED CLAY AND CONCRETE PIPE

LENGTH OF LINE, FEET	SPECIFICATION TIME (MIN:SEC) REQUIRED FOR PRESSURE DROP FROM 3-1/2 TO 2-1/2 PSIG														
	WHEN TESTING ONE PIPE DIAMETER ONLY														
	PIPE DIAMETER, INCHES														
	4	6	8	10	12	15	18	21	24	27	30	33	36	39	42
25	0:04	0:10	0:18	0:22	0:27	0:32	0:36	0:45	0:54	1:03	1:12	1:21	1:30	1:39	1:50
50	0:09	0:21	0:36	0:45	0:54	1:03	1:12	1:30	1:48	2:06	2:42	2:42	3:00	3:18	3:39
75	0:14	0:32	0:54	1:08	1:21	1:34	1:48	2:15	2:42	3:09	3:36	4:03	4:30	4:57	5:29
100	0:18	0:42	1:12	1:30	1:48	2:06	2:24	3:00	3:36	4:12	4:48	5:24	6:00	6:36	7:18
125	0:22	0:52	1:30	1:52	2:15	2:38	3:00	3:45	4:30	5:15	6:00	6:45	7:30	8:15	9:08
150	0:27	1:03	1:48	2:15	2:42	3:09	3:36	4:30	5:24	6:18	7:12	8:06	9:00	9:54	10:57
175	0:32	1:14	2:06	2:38	3:09	3:40	4:12	5:15	6:18	7:21	8:24	9:27	10:30	11:33	12:47
200	0:36	1:24	2:24	3:00	3:36	4:12	4:48	6:00	7:12	8:24	9:36	10:48	12:00	13:12	14:36
225	0:40	1:34	2:42	3:22	4:03	4:44	5:24	6:45	8:06	9:27	10:48	12:09	13:30	14:51	16:26
250	0:45	1:45	3:00	3:45	4:30	5:15	6:00	7:30	9:00	10:30	12:00	13:30	15:00	16:30	18:16
275	0:50	1:56	3:18	4:08	4:57	5:46	6:36	8:15	9:54	11:33	13:12	14:51	16:30	18:09	20:06
300	0:54	2:06	3:36	4:30	5:24	6:18	7:12	9:00	10:48	12:36	14:24	16:12	18:00	19:48	21:54
350	1:03	2:27	4:12	5:15	6:18	7:21	8:24	10:30	12:36	14:42	16:48	18:54	21:00	23:06	25:33
400	1:12	2:48	4:48	6:00	7:12	8:24	9:36	12:00	14:24	16:48	19:12	21:36	24:00	26:24	29:12
450	1:21	3:09	5:24	6:45	8:06	9:27	10:48	13:30	16:12	18:54	21:36	24:18	27:00	29:42	32:51
500	1:30	3:30	6:00	7:30	9:00	10:30	12:00	15:00	18:00	21:00	24:00	27:00	30:00	33:00	36:30

NOTE: THIS TABLE IS TAKEN FROM THE NATIONAL CLAY PIPE INSTITUTE (NCP) TABLES WHICH ARE BASED UPON ASTM C828 "TEST METHOD FOR LOW PRESSURE AIR TEST FOR VITRIFIED CLAY PIPE LINES" AND ASTM C924 "STANDARD PRACTICE FOR TESTING CONCRETE PIPE SEWER LINES BY LOW PRESSURE AIR TEST METHOD."

AIR TEST TABLE FOR PVC AND ABS PIPE MINIMUM SPECIFIED TIME REQUIRED FOR A 1.0 PSIG PRESSURE DROP FOR SIZE AND LENGTH OF PIPE INDICATED FOR Q=0.0015*

THE MINIMUM HOLDING TIME FOR THE PRESSURE TO DROP FROM 3.5 TO 2.5 PSIG (GREATER THAN ADDED GROUND WATER PRESSURE) SHALL NOT BE LESS THAN THAT GIVEN IN THE FOLLOWING TABLE FOR EACH TESTED RUN OF SEWER BETWEEN MANHOLES:

NOTE: AIR TESTING SHOULD NOT BE USED IF THE AIR PRESSURE REQUIRED FOR THE TEST EXCEEDS 9 PSIG.

PIPE DIAMETER INCHES	MINIMUM TIME MINUTES	LENGTH FOR MINIMUM TIME, FT.	TIME FOR LONGER LENGTH, S	SPECIFICATION TIME FOR LENGTH (L) SHOWN, MINUTES									
				100 FT	150 FT	200 FT	250 FT	300 FT	350 FT	400 FT	450 FT		
4	3:46	597	0.380 L	3:46	3:46	3:46	3:46	3:46	3:46	3:46	3:46	3:46	3:46
6	5:40	398	0.854 L	5:40	5:40	5:40	5:40	5:40	5:40	5:40	5:42	6:24	
8	7:34	298	1.520 L	7:34	7:34	7:34	7:34	7:34	7:36	8:52	10:08	11:24	
10	9:26	239	2.374 L	9:26	9:26	9:26	9:53	11:52	13:51	15:49	17:48		
12	11:20	199	3.418 L	11:20	11:20	11:24	14:15	17:05	19:56	22:47	25:38		
15	14:10	159	5.342 L	14:10	14:10	17:48	22:15	26:42	31:09	35:36	40:04		
18	17:00	133	7.692 L	17:00	19:13	25:38	32:03	38:27	44:52	51:16	57:41		
21	19:50	114	10.470 L	19:50	26:10	34:54	43:37	52:21	61:00	69:48	78:31		
24	22:40	99	13.674 L	22:47	34:11	45:34	56:58	68:22	79:46	91:10	102:33		
27	25:30	88	17.306 L	28:51	43:16	57:41	72:07	86:32					
30	28:20	80	21.366 L	35:37	53:25	71:13	89:02	106:50					
33	31:10	72	25.852 L	43:05	64:38	86:10	107:43	129:16					
36	34:00	66	30.768 L	51:17	76:55	102:34	128:12	153:50					

NOTE: THIS TABLE IS TAKEN FROM ASTM F1417 "STANDARD TEST METHOD FOR INSTALLATION AND ACCEPTANCE OF PLASTIC GRAVITY SEWER LINES USING LOW PRESSURE AIR TEST." ASTM F1417 CONFORMS TO UNI-BELL "RECOMMENDED PRACTICE FOR LOW PRESSURE AIR TESTING OF INSTALLED SEWER PIPE" (UNI-B-6-98)

* Q IS THE ALLOWABLE LEAKAGE RATE IN CUBIC FEET/MINUTE/SQUARE FOOT OF INSIDE SURFACE AREA OF PIPE

EXFILTRATION TEST

EXFILTRATION OR LEAKAGE FROM THE SEWER LINE CAN BE MEASURED BY RECORDING THE WATER LEVEL DROP OVER A GIVEN PERIOD OF TIME IN A STANDPIPE PLACED AND CONNECTED IN THE UPSTREAM MANHOLE. THE MEASURED DROP IN THE TIME PERIOD CAN BE CONVERTED BY CALCULATIONS TO THE LEAKAGE RATE IN TERMS OF GALLONS PER INCH OF PIPE DIAMETER PER MILE PER DAY.

EXFILTRATION TESTS MAY BE SUBSTITUTED FOR LOW PRESSURE AIR TESTS WHERE APPROVED BY THE TOWNSHIP ENGINEER. EXFILTRATION TESTS WILL NOT BE ALLOWED WHERE THE EXTERNAL WATER PRESSURE EXCEEDS FOUR (4) FEET.

FOR THE PURPOSE OF EXFILTRATION TESTING, THE INTERNAL WATER LEVEL SHALL BE EQUAL TO THE EXTERNAL WATER LEVEL PLUS FOUR (4) FEET AS MEASURED FROM THE TOP OF THE HIGHEST PIPE IN THE SYSTEM BEING TESTED. THIS COULD BE EITHER A HOUSE LEAD OR A LATERAL. HOWEVER, THE MAXIMUM TOTAL HEIGHT OF WATER ABOVE THE INVERT OF THE PIPE AT THE LOWER END SHALL NOT EXCEED SIXTEEN (16) FEET. A PROSPECTIVE TEST THAT WOULD EXCEED THIS SIXTEEN (16) FOOT LIMIT SHOULD NOT BE TAKEN. THE LINE UNDER CONSTRUCTION CAN BE BROKEN DOWN INTO SMALLER SECTIONS SUCH THAT THE MAXIMUM HEAD OF SIXTEEN (16) FEET WILL NOT BE EXCEEDED.

THE MAXIMUM EXFILTRATION RATE SHALL BE THE SAME AS THAT PERMITTED FOR THE INFILTRATION TEST. THE EXFILTRATION TEST PROCEDURE IS SUMMARIZED AS FOLLOWS:

- 1) ALL SERVICE LATERALS, STUBS AND FITTINGS INTO THE SEWER LINE(S) BEING TESTED SHOULD BE PROPERLY CAPPED OR PLUGGED, AND CAREFULLY BRACED TO RESIST THE THRUST ACTIONS DEVELOPED BY THE INTERNAL WATER PRESSURE. IN PREPARING THE BLOCKING OF PLUGS OR END CAPS, IT IS EXTREMELY IMPORTANT TO RECOGNIZE THAT THE FIVE (5) TO TEN (10) FEET OF HEAD IN THE STANDPIPE WILL EXERT CONSIDERABLE THRUST AGAINST THE PLUGS OR CAPS.
- 2) A PLUG IS INSERTED AND TIGHTENED IN THE INLET PIPE OF THE DOWNSTREAM MANHOLE TO WHICH THE WATER SUPPLY CONNECTION IS MADE FOR FILLING THE PIPE.
- 3) THE UPPER MANHOLE IS PLUGGED AND SECURELY TIGHTENED FOR CONNECTION TO THE STANDPIPE. THE STANDPIPE IS THEN PLACED IN THIS MANHOLE AND CONNECTED TO THE TAPPED PLUG. THE STANDPIPE MUST BE CAPABLE OF HANDLING FROM FIVE (5) TO TEN (10) FEET OF WATER HEAD TO DETERMINE THE TIGHTNESS AND SOUNDNESS OF THE SEWER LINE, AS SPECIFIED AND DIRECTED BY THE ENGINEER.
- 4) WATER IS INTRODUCED INTO THE LINE AT THE DOWNSTREAM (LOWER) MANHOLE UNTIL THE STANDPIPE IN THE UPSTREAM MANHOLE HAS BEEN COMPLETELY FILLED, BY FILLING THE LINE FROM THE LOWEST LEVEL, THE AIR IN THE LINE IS EASILY PUSHED AHEAD AND, FINALLY DISPELLED THROUGH THE STANDPIPE AT THE UPPER END OF THE TEST SECTION. CARE SHOULD BE TAKEN TO MINIMIZE ENTRAPPED AIR THAT WILL GIVE DISTORTED TEST RESULTS. THE RATE OF DROP IN THE STANDPIPE MAY BE QUITE RAPID UNTIL THE AIR HAS BEEN EXPELLED.
- 5) AFTER FILLING WITH WATER, THE LINE MUST BE ALLOWED TO STAND FOR AT LEAST FOUR (4) HOURS BEFORE BEGINNING THE TEST. DURING THIS TIME SOME WATER ABSORPTION INTO THE MANHOLE STRUCTURES AND SEWER PIPE WILL TAKE PLACE. AFTER THE WATER ABSORPTION HAS STABILIZED, THE WATER LEVEL IN THE STANDPIPE IS CHECKED AND WATER ADDED IF NECESSARY.
- 6) THE TEST IS NOW READY TO BEGIN. THE DROP IN THE STANDPIPE IS MEASURED AND RECORDED OVER A FIFTEEN (15) MINUTE PERIOD. TO VERIFY THE FIRST RESULTS, A SECOND FIFTEEN (15) MINUTE TEST IS SUGGESTED. THIS WILL ALSO VERIFY WHETHER A STABLE CONDITION EXISTS IN THE LINE.
- 7) THE MEASURED DROPS IN THE STANDPIPE ARE CONVERTED TO LEAKAGE IN TERMS OF GALLONS PER INCH DIAMETER PER MILE PER DAY.
- 8) ANOTHER COMMONLY USED METHOD OF CONDUCTING WATER EXFILTRATION TESTING IS TO UTILIZE THE MANHOLE IN LIEU OF A STANDPIPE. THE TEST PROCEDURE IS EXACTLY AS OUTLINED FOR USING THE STANDPIPE. HOWEVER, SINCE THE MANHOLE IS LARGER IN DIAMETER THAN THE STANDPIPE, THIS METHOD NORMALLY REQUIRES A MINIMUM TWO (2) HOUR TEST PERIOD IN ORDER TO BE ABLE TO RECORD A MEASURABLE WATER LEVEL DROP. MANHOLE LEAKAGE MUST ALSO BE CONSIDERED IN THE LEAKAGE RATE AND TEST RESULTS.
- 9) CAUTION SHOULD BE TAKEN ABOUT CONDUCTING EXFILTRATION TESTS ON SEWER LINES LAID ON STEEP GRADES. CONSIDERATION MUST BE GIVEN TO THE DOWNSTREAM PORTION OF THE SYSTEM TO PREVENT EXCESSIVE PRESSURES IN THESE LOWER LINES. FOR THESE INSTALLATIONS AND WHERE THE UPSTREAM MANHOLES ARE VERY DEEP, IT IS NOT ADVISABLE TO FILL THE STANDPIPE OR MANHOLE TO THE TOP WHEN PERFORMING THE TEST.

DEFLECTION TEST FOR PLASTIC PIPE

DEFLECTION GAUGE (MANDREL): MANDREL TESTING SHALL TAKE PLACE TO ENSURE THE FLEXIBLE PIPE HAS BEEN PROPERLY BEDDED AND BACK-FILLED. THE DEFLECTION TEST MUST BE CONDUCTED NO LESS THAN 30 DAYS AFTER INSTALLATION OF THE FINAL BACKFILL. THE MAXIMUM ALLOWABLE DEFLECTION IS 5 PERCENT. INSTALLATION SHALL CONFORM TO ASTM 2321-89. A NINE-ARM (POINT) MANDREL SHALL BE USED. CHERNE FIXED STEEL DEFLECTION OR APPROVED EQUAL.

VIDEOTAPING

AS A MEANS OF INSURING THAT PIPE LAYING WAS PROPERLY DONE AND THAT ALL JOINTS ARE IN A "HOME" POSITION, THE CONTRACTOR SHALL PROVIDE VIDEOTAPING OF ALL OF THE PIPE LAID THAT IS THIRTY-SIX (36) INCHES IN DIAMETER AND SMALLER. THIS VIDEOTAPING SHALL BE DONE NO SOONER THAN THIRTY (30) DAYS AFTER COMPLETION OF BACKFILL. THE CONTRACTOR SHALL PROVIDE FORTY-EIGHT (48) HOURS NOTICE TO THE TOWNSHIP PRIOR TO VIDEOTAPING SO THAT A REPRESENTATIVE MAY BE PRESENT. A SATISFACTORY REVIEW OF THE VIDEOTAPE BY THE TOWNSHIP SHALL BE A CONDITION FOR SEWER ACCEPTANCE BY THE TOWNSHIP. TYPICAL ITEMS TO BE REVIEWED ON THE VIDEOTAPE WILL INCLUDE PIPE DEFLECTION, PIPE SETTLEMENT, LEAD CONNECTIONS, JOINTS AND PIPE CLEANLINESS. IF THE VIDEOTAPE REVIEW REVEALS UNSATISFACTORY CONDITIONS, THE CONTRACTOR SHALL CORRECT THE CONDITIONS AND SHALL RE-VIDEOTAPE THE AFFECTED PIPE SECTIONS FOR REVIEW BY THE TOWNSHIP.



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34000 Plymouth Road | Livonia, MI 48150 | P (734) 522-6711 | F (734) 522-6427

DATE: OCT 2003

CHARTER TOWNSHIP OF SUPERIOR
STANDARD SANITARY DETAILS

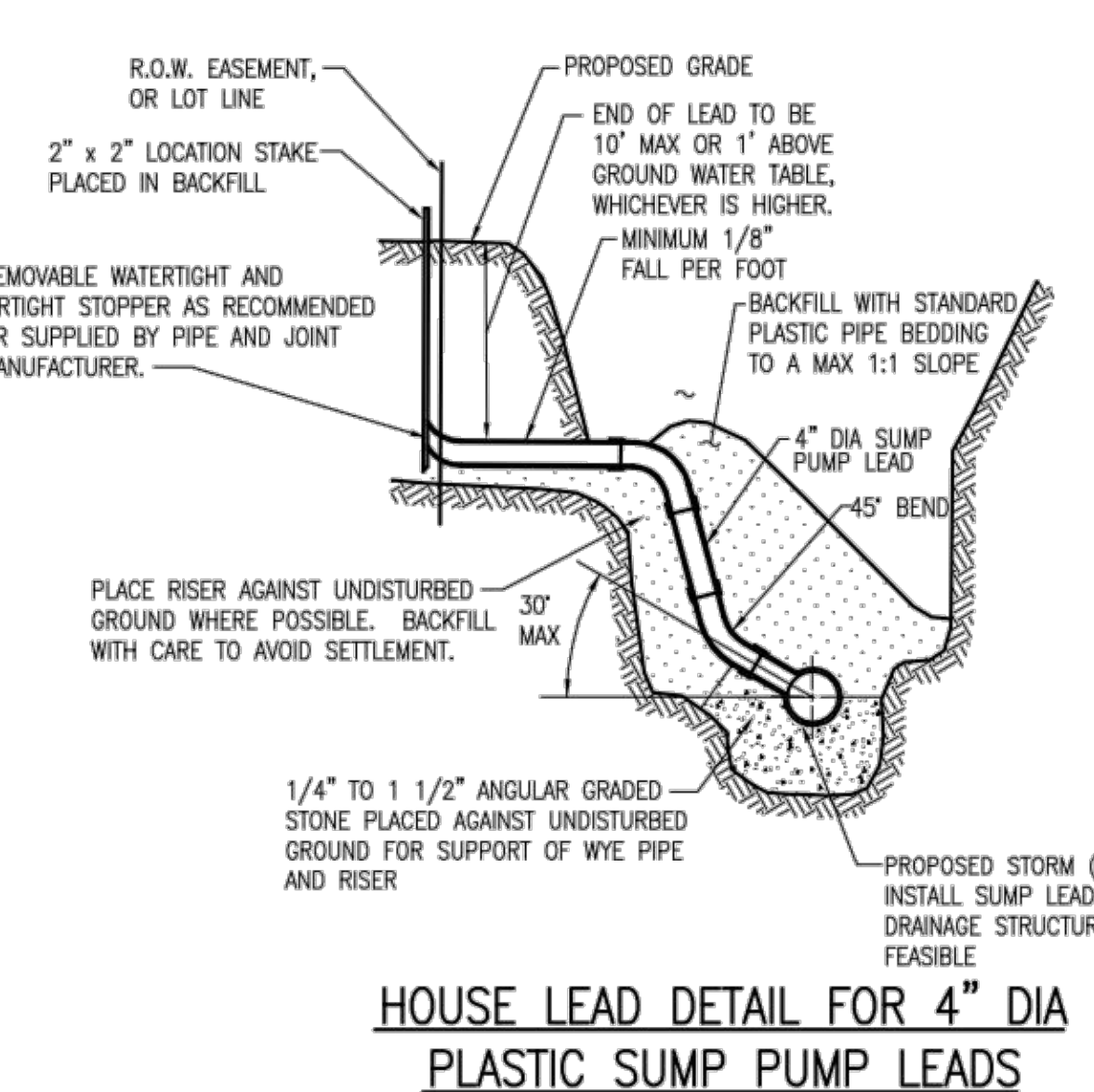
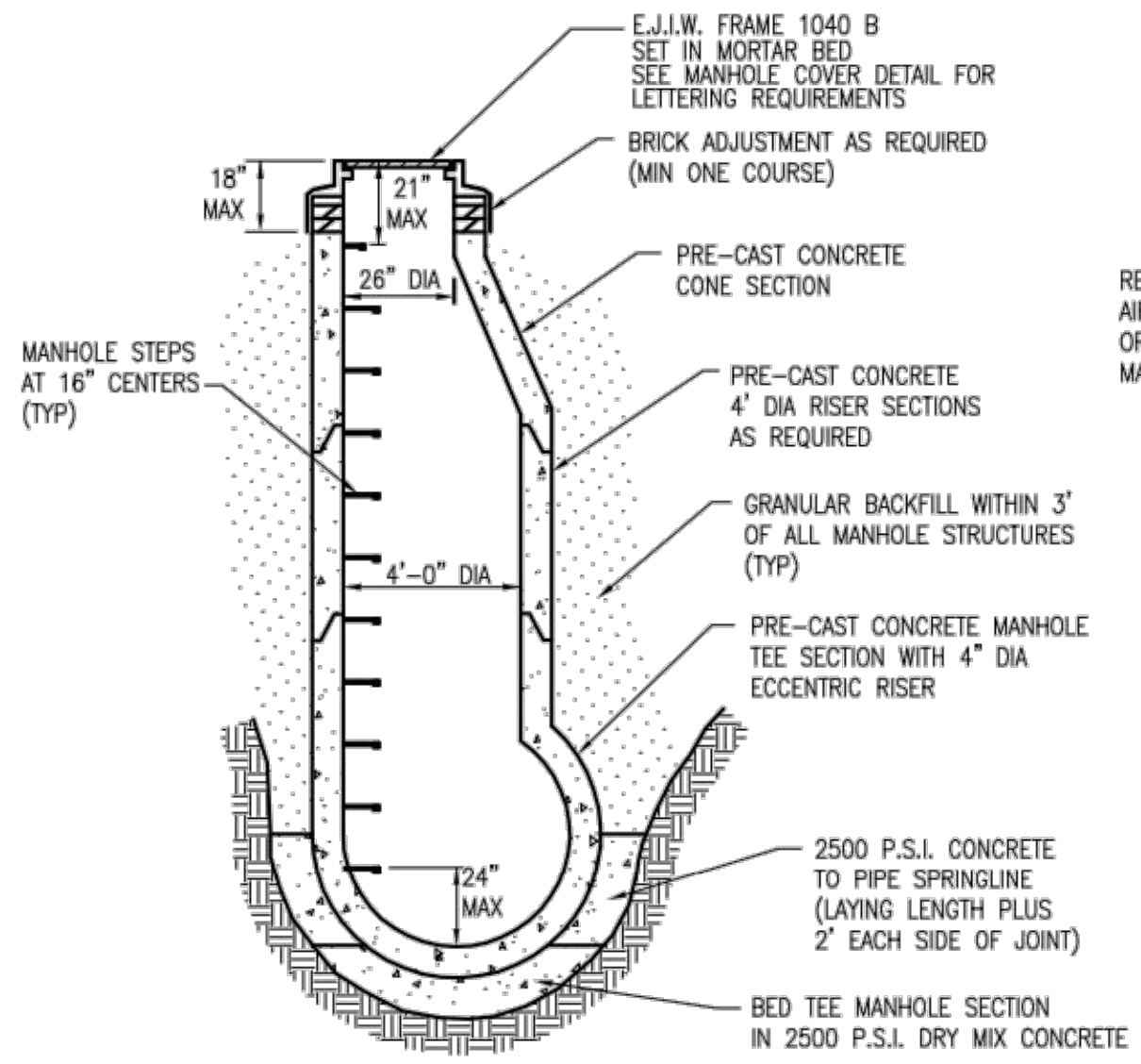
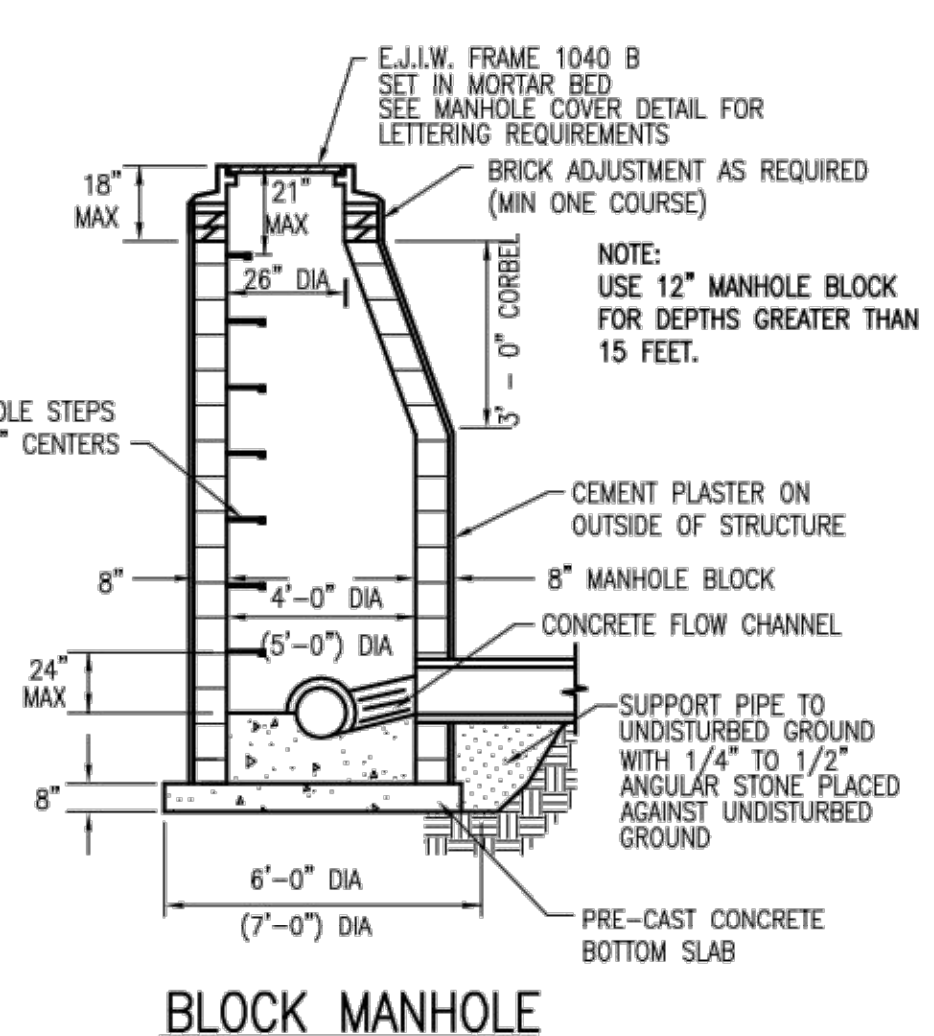
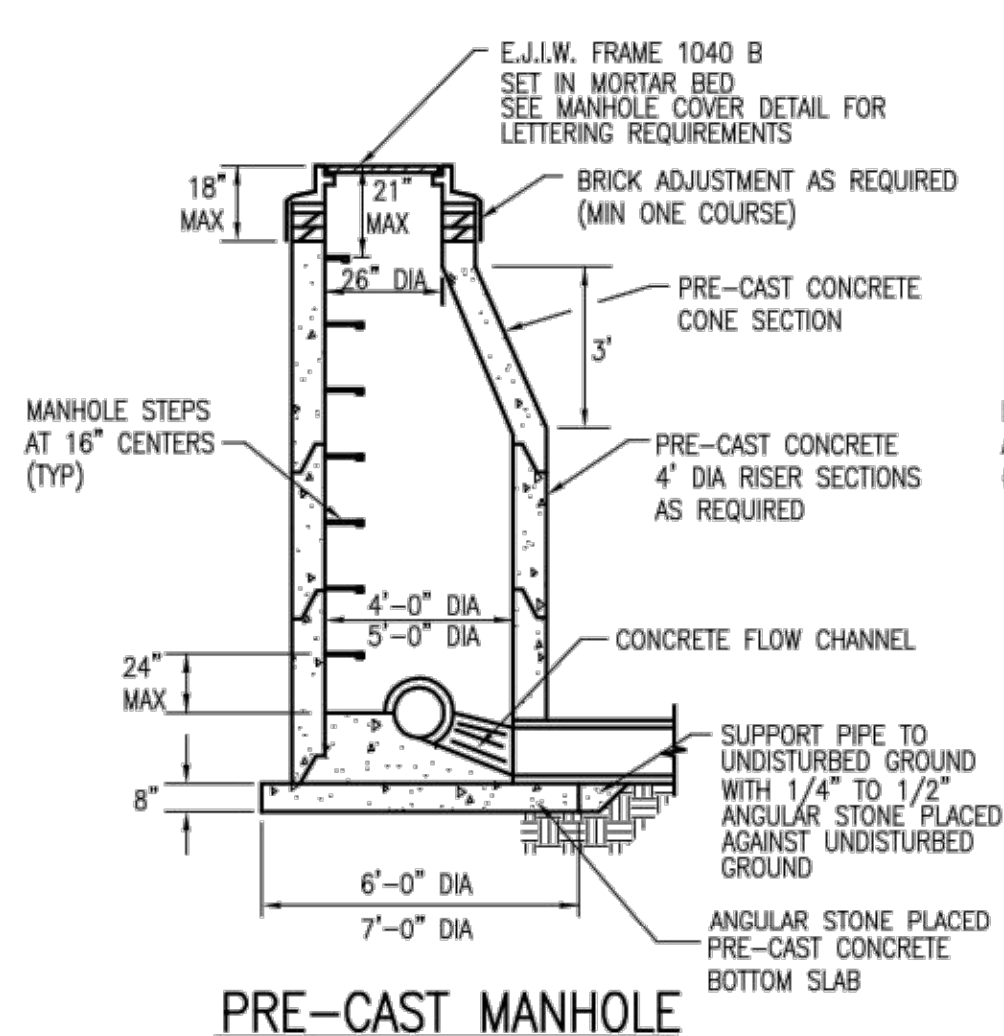
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GENERAL NOTES FOR STORM SEWER CONSTRUCTION

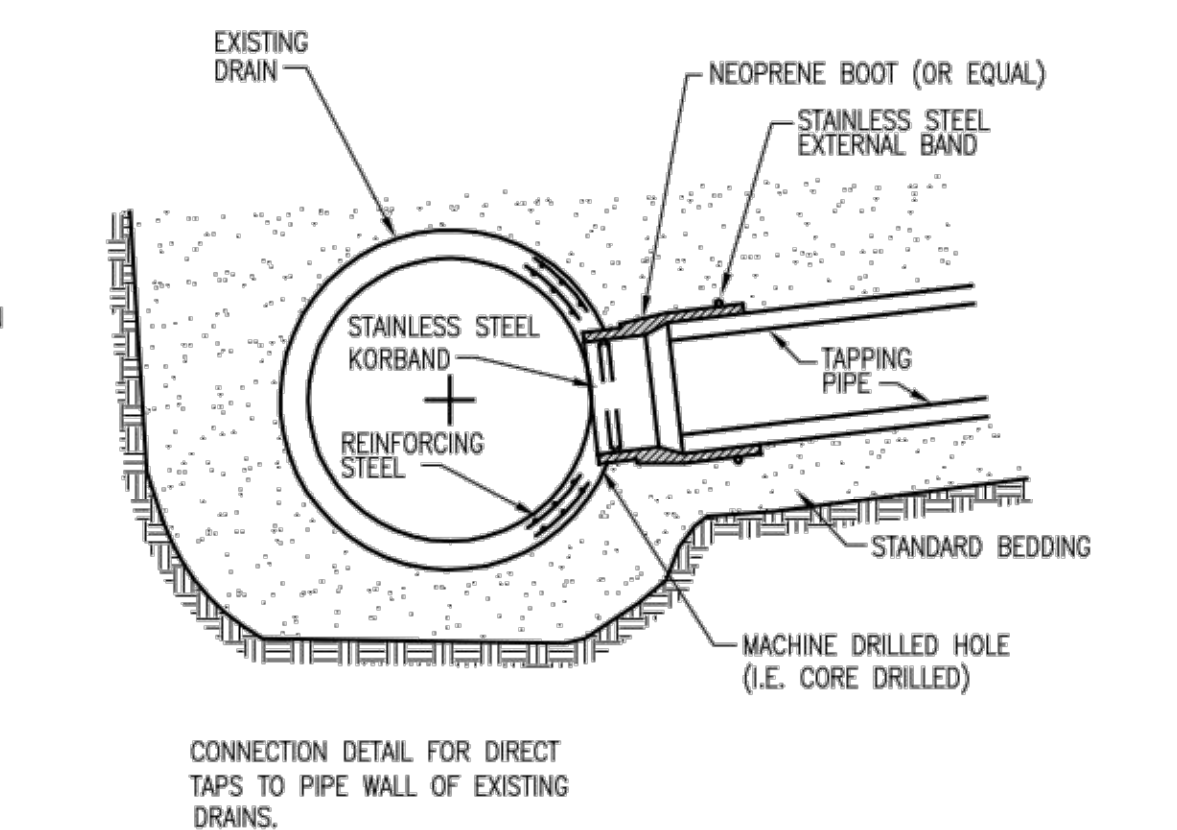
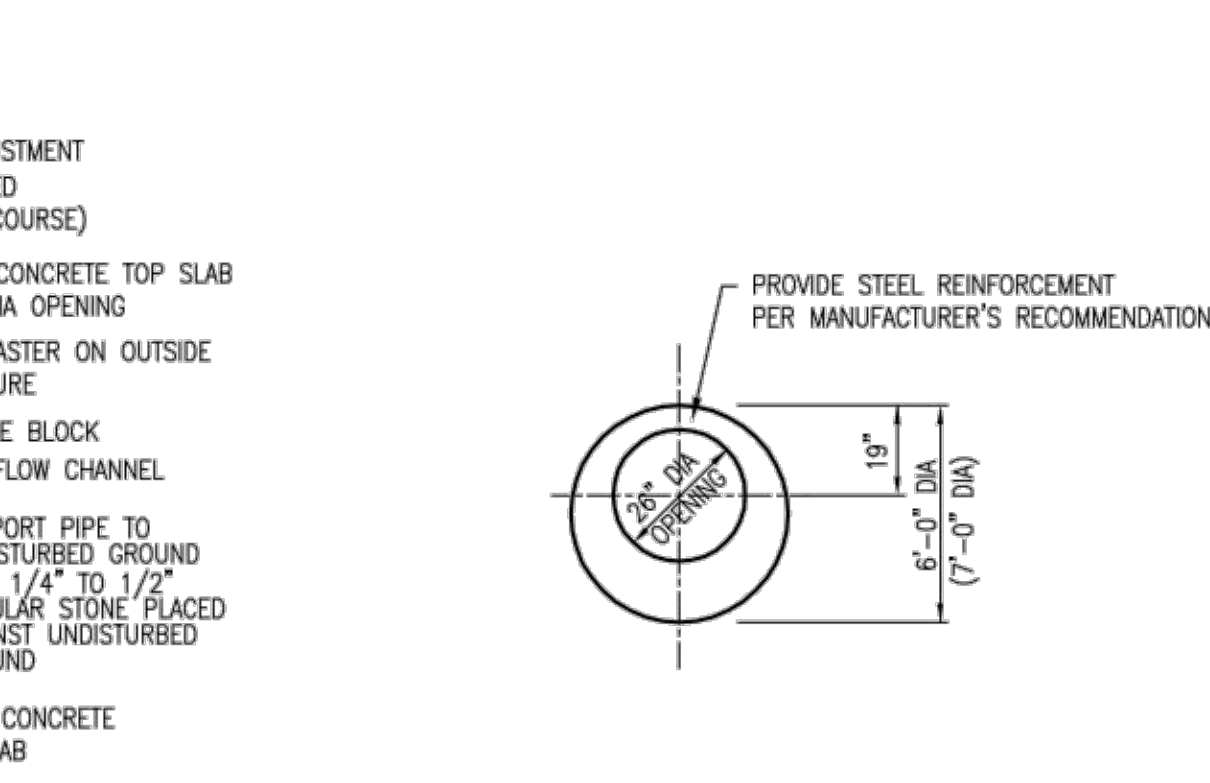
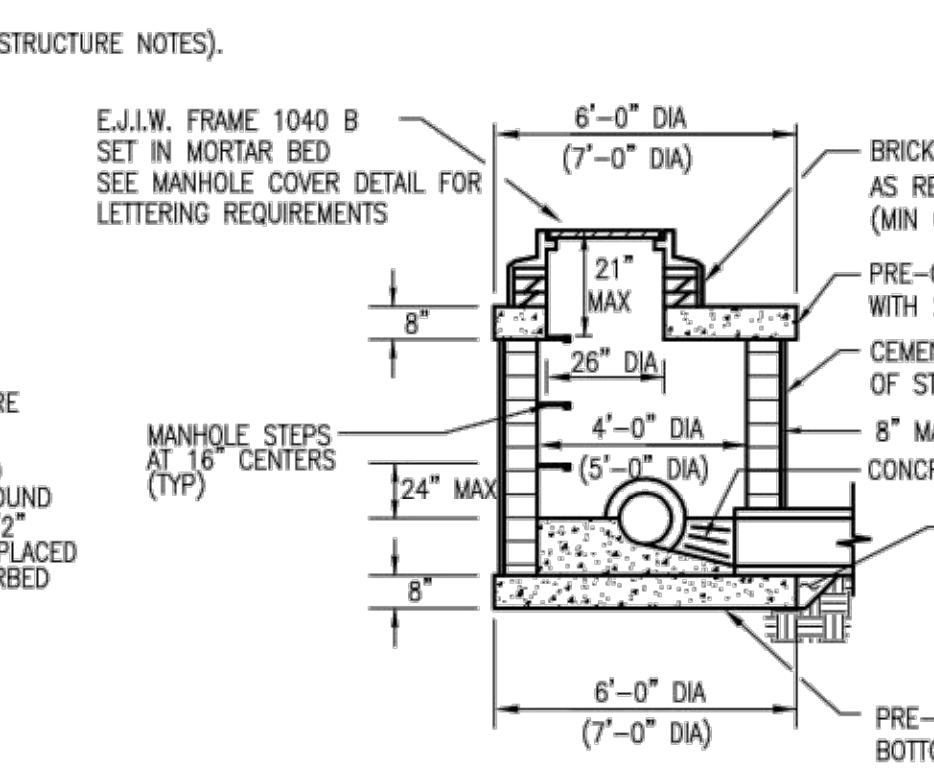
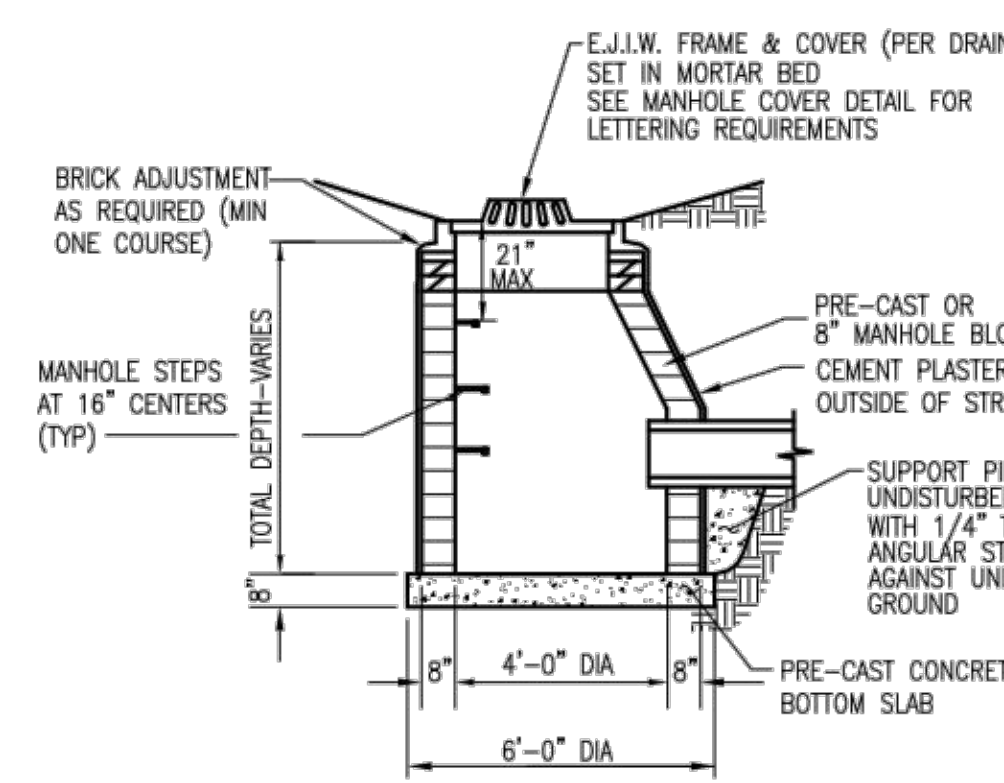
- ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE STANDARDS AND SPECIFICATIONS OF CHARTER TOWNSHIP OF SUPERIOR.
 - TYPE AND CLASS OF PIPE SHALL BE AS SPECIFIED ON PLANS.
 - SAND BEDDING SHALL BE USED THROUGHOUT, UNLESS OTHERWISE SPECIFIED ON THE PLAN.
 - ALL END SECTIONS 18" AND LARGER SHALL BE PROVIDED WITH A GALVANIZED BAR SCREEN.
- CONTRACTOR SHALL CONSTRUCT MANHOLES WITH PRECAST REINFORCED CONCRETE IN LIEU OF CONCRETE, BRICK AND BLOCK MANHOLES IN ACCORDANCE WITH THE FOLLOWING CONDITIONS:
 - NO OPENINGS SHALL BE MADE IN PRECAST UNITS WHICH WOULD LEAVE LESS THAN 12" OF UNDISTURBED PRECAST STRUCTURE WALL BETWEEN PIPES (AS MEASURED BETWEEN OUTSIDE PIPE WALLS) OR WOULD REMOVE MORE THAN 40% OF THE CIRCUMFERENCE ALONG ANY HORIZONTAL PLANE.
 - STRUCTURES FOR SEWERS LARGER THAN 18", OR THOSE NOT MEETING THE OPENING REQUIREMENTS, MAY BE BUILT OF BLOCK OR BRICK UP TO A MINIMUM OF 8" ABOVE THE TOP OF SEWER, WITH PRECAST UNITS BEING USED ABOVE THIS POINT. WHERE PRECAST UNITS REST ON THE BLOCK OR BRICK, THE GROOVE IN THE PRECAST UNIT SHALL BE FILLED WITH MORTAR.
 - OPENINGS FOR THE OUTLET SEWER SHALL BE PRECAST WITH A DIAMETER OF 3 INCHES LARGER THAN THE OUTSIDE DIAMETER OF THE OUTLET PIPE. ALL OTHER OPENINGS SHALL BE MADE IN THE FIELD AFTER THE MANHOLE HAS BEEN CONSTRUCTED.
- ALL VERTICAL OPENINGS IN CONCRETE BLOCK STRUCTURE WALLS SHALL BE COMPLETELY FILLED WITH MORTAR. ALL VERTICAL WALL JOINTS SHALL BE CEMENT POINTED.
 - CONCRETE PIPE REQUIREMENTS:
 - THE CONTRACTOR SHALL PROVIDE REINFORCED CONCRETE PIPE AS SPECIFIED ON THE PLANS.
 - ALL ROUND REINFORCED CONCRETE PIPE SHALL HAVE MODIFIED GROOVE TONGUE JOINTS WITH O-RING TYPE RUBBER GASKET, PER A.S.T.M. SPECIFICATIONS C443. ALL ELLIPTICAL CONCRETE PIPE SHALL HAVE GROOVE TONGUE JOINTS WITH BITUMINOUS (DEWITT #10) JOINT MATERIAL AND INSIDE MORTAR POINTING. ELLIPTICAL CONCRETE PIPE JOINTS SHALL ALSO BE WRAPPED PER A.S.T.M. SPECIFICATION C877 FOR EXTERNAL SEALING BANDS FOR NON-CIRCULAR CONCRETE PIPE.
 - THE INSIDE JOINT OF PIPE SIZES OVER 27" DIAMETER SHALL BE POINTED UP WITH MORTAR UPON COMPLETION OF BACKFILLING OPERATIONS.
 - WHERE UNSTABLE GROUND CONDITIONS ARE ENCOUNTERED, STONE BEDDING SHALL BE USED AS DIRECTED BY THE ENGINEER IN ORDER TO PROVIDE A STABLE FOUNDATION FOR PIPE AND MANHOLES.
 - ALL PIPES ENTERING OR LEAVING A MANHOLE SHALL BE ADEQUATELY SUPPORTED WITH 1/4" TO 1/2" ANGULAR GRADED STONE FILL FROM UNDISTURBED EARTH TO SPRINGLINE OR WITH APPROVED CRUSHED AGGREGATE.

DRAINAGE STRUCTURE REQUIREMENTS:

- ALL STRUCTURE(S) SHALL BE 4' IN DIAMETER UNLESS OTHERWISE INDICATED ON CONSTRUCTION DRAWINGS. 2' DIAMETER CATCH BASINS AND INLETS SHALL BE USED ONLY WITH PRIOR TOWNSHIP APPROVAL.
- MANHOLE STEPS SHALL BE STEEL, ENCASED WITH POLYPROPYLENE PLASTIC OR APPROVED EQUIVALENT TO M.A. INDUSTRIES, INC., PS-1 FOR BRICK, OR PS-1B FOR BLOCK, EAST JORDAN IRON WORKS 8503 (OR APPROVED EQUAL). MANHOLE STEPS AT 16" CENTERS.
- CATCH BASIN STEPS SHALL BE EAST JORDAN IRON WORKS 8502 PLASTIC COATED (OR APPROVED EQUAL).
- MANHOLE COVERS AND FRAMES SHALL BE EAST JORDAN IRON WORKS 1040, TYPE "B" COVER OR AS PER CONSTRUCTION DRAWINGS.
 - CATCH BASIN AND INLET FRAME AND COVER SHALL BE:
 - EAST JORDAN IRON WORKS 5080, TYPE "M1" COVER WITH STRAIGHT FACE CURB AND GUTTER (OR AS APPROVED EQUAL).
 - EAST JORDAN IRON WORKS 5080, TYPE "M1" COVER WITH MOUNTABLE CURB AND GUTTER AND INTEGRAL CURB AND GUTTER (OR AS APPROVED EQUAL).
 - EAST JORDAN IRON WORKS 1040, TYPE "O2" COVER (BEEHIVE) TO BE USED ON OPEN DITCHES AND SWALES, REAR YARD CATCH BASIN (OR AS APPROVED EQUAL). IF WITHIN 8' OF ROAD, TYPE "N" COVER (LOW BEEHIVE) SHALL BE USED.
 - FRAMES SHALL BE SET IN FULL BED OF MORTAR AND THE SIDE SHALL BE OVERLAPPED TO PREVENT LEAKAGE.
- A PROPER CHANNEL SHALL BE CONSTRUCTED WITHIN THE EXISTING MANHOLE OR OTHER STRUCTURE AT WHICH THE CONNECTION IS TO BE MADE TO DIRECT THE FLOW TO THE EXISTING OUTLET IN A MANNER WHICH WILL TEND TO CREATE THE LEAST AMOUNT OF TURBULENCE. THE CHANNEL SHALL BE CONSTRUCTED TO THE SAME SIZE AS THE INSIDE DIAMETER OF THE EXISTING PIPES, AND SHALL BE BUILT TO HEIGHT OF 1/3 THE EXISTING PIPE DIAMETER WITH A MINIMUM OF 2% SLOPE ON THE BENCHES.
- STANDARD BRICK ADJUSTMENT: MINIMUM OF ONE COURSE AND A MAXIMUM OF 5 COURSES OF BRICK.
 - ALL BRICKS AND BLOCKS USED FOR ADJUSTMENT SHALL BE CONCRETE.
 - BLOCK USED FOR STANDARD CATCH BASINS AND MANHOLES SHALL BE 8" (FOR 0'-15" DEEP) AND 12" (FOR 15'-25" DEEP). BLOCK USED FOR 2' DIAMETER INLETS AND CATCH BASINS SHALL BE 6".
 - PRECAST REINFORCED CONCRETE SECTION AS MINIMUM SHALL CONFORM TO A.S.T.M. C-478.
 - CONCRETE BASE FOR MANHOLE, CATCH BASIN, AND INLET SHALL BE MDOT GRADE 30P (MIN), 8" THICK, 3000 P.SI.
- PLASTER ALL OUTSIDE MASONRY SURFACES WITH 1:2 1/2 MASONRY CEMENT (TYPE II) 1/2" THICK.
- WHEN TAPPING INTO AN EXISTING STRUCTURE A BRICK COLLAR SHALL BE PLACED 12" THICK AROUND THE PIPE AND EXTENDED 12" BEYOND THE OPENING. IF PRE-CAST SECTION IS TAPPED, BEND MESH AND USE AS REINFORCEMENT WITH BRICK COLLAR.
- ALL PRECAST RISER(S) SHALL BE PLACED IN A FULL BED OR MORTAR. ALL JOINTS & LIFTHOLES SHALL BE POINTED UP WITH MORTAR ON THE OUTSIDE AND INSIDE.
 - ALL VERTICAL AND HORIZONTAL BARS SHALL BE TACK-WELDED TO THE ANGLE FRAME.
 - THE BAR GRATE SCREEN SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION IS COMPLETE.



STANDARD MANHOLE DETAILS

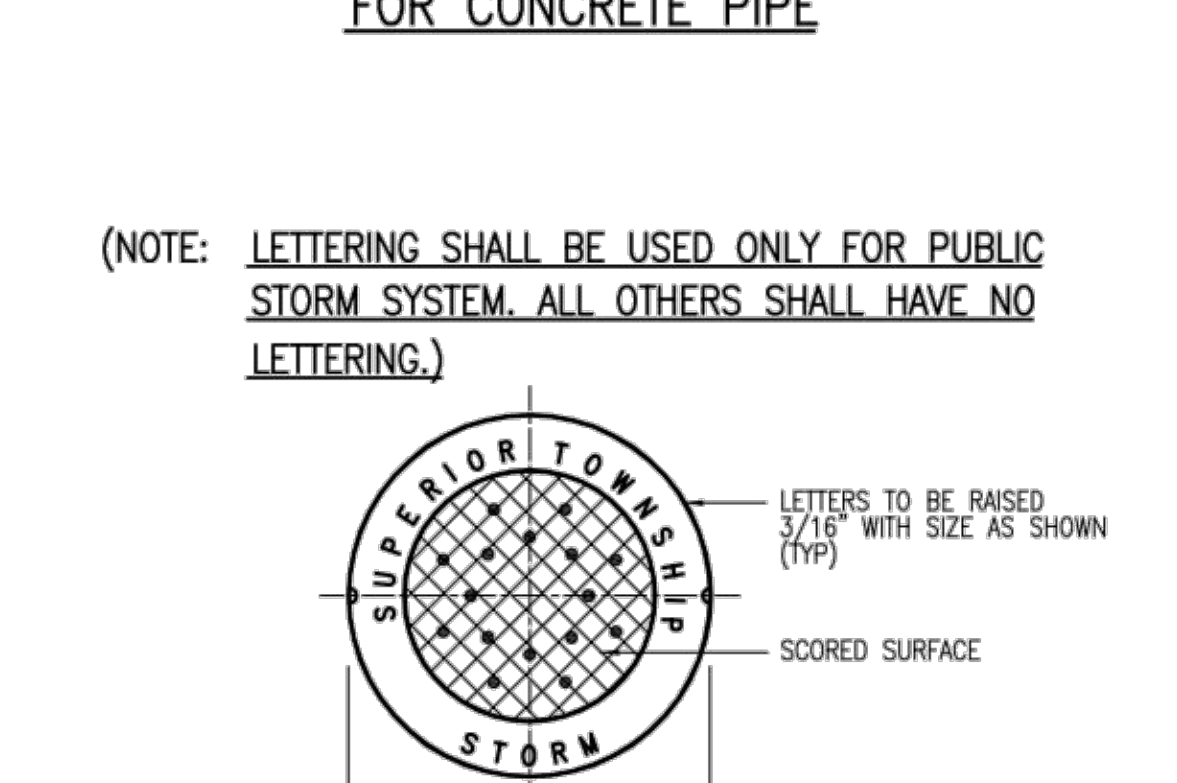
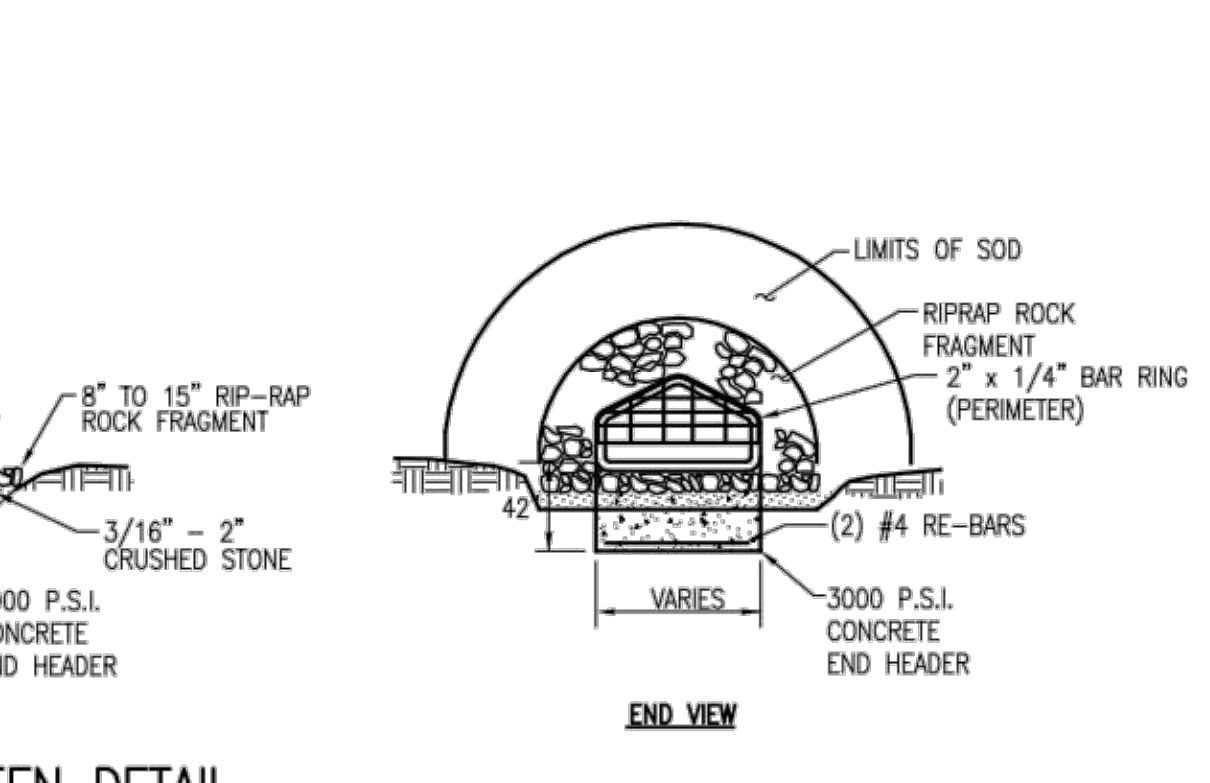
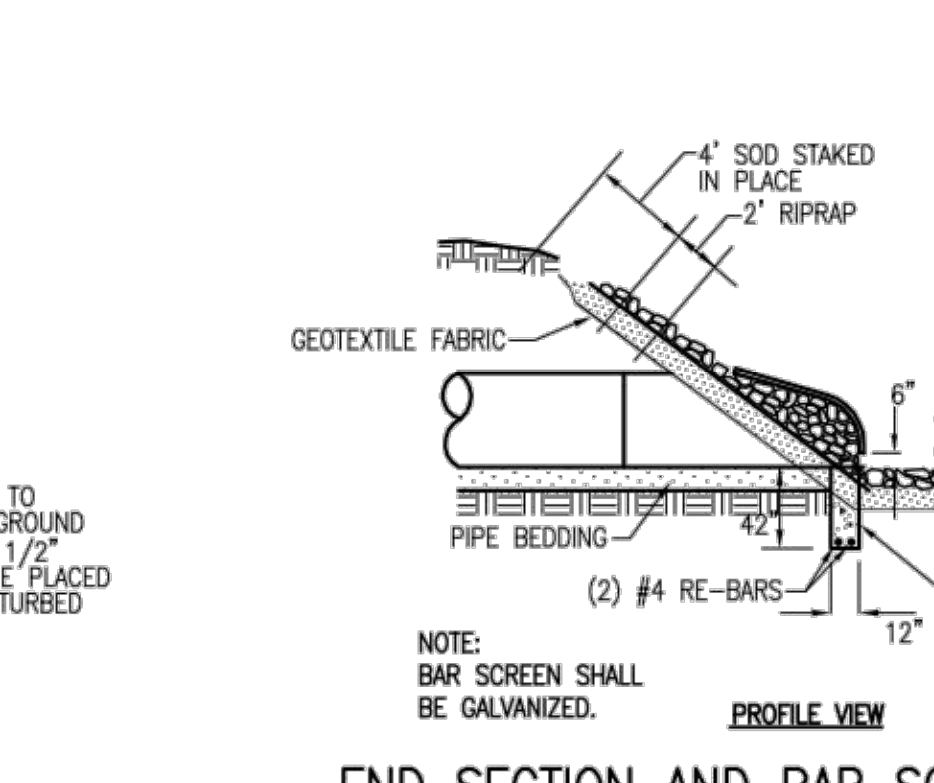
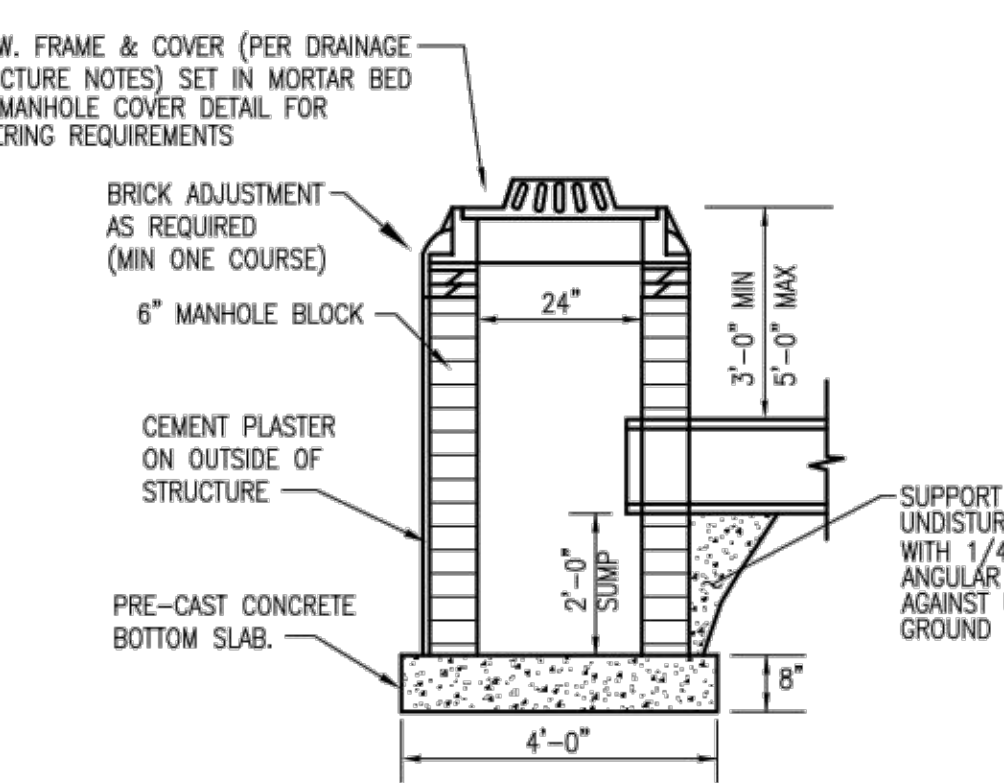


CATCH BASIN DETAIL

LOW HEAD MANHOLE AND CATCH BASIN DETAIL

PRE-CAST CONCRETE TOP SLAB DETAIL, 8" THICK

KOR-N-TEE TAP FOR CONCRETE PIPE



2' DIA CATCH BASIN DETAIL

END SECTION AND BAR SCREEN DETAIL

2' DIA INLET DETAIL

CAST IRON MANHOLE COVER E.J.I.W. 1040

NOT TO SCALE
(NOTE: 2' DIA CATCH BASIN SHALL NOT BE USED UNLESS PRIOR APPROVAL IS OBTAINED FROM TOWNSHIP ENGINEER.)

RIP RAP SCHEDULE

PIPE DIA	SY
12"	4
15"	4
18"	4
21"	5
24"	6
27"	7
30"	8
36"	10
42"	12
48"	14

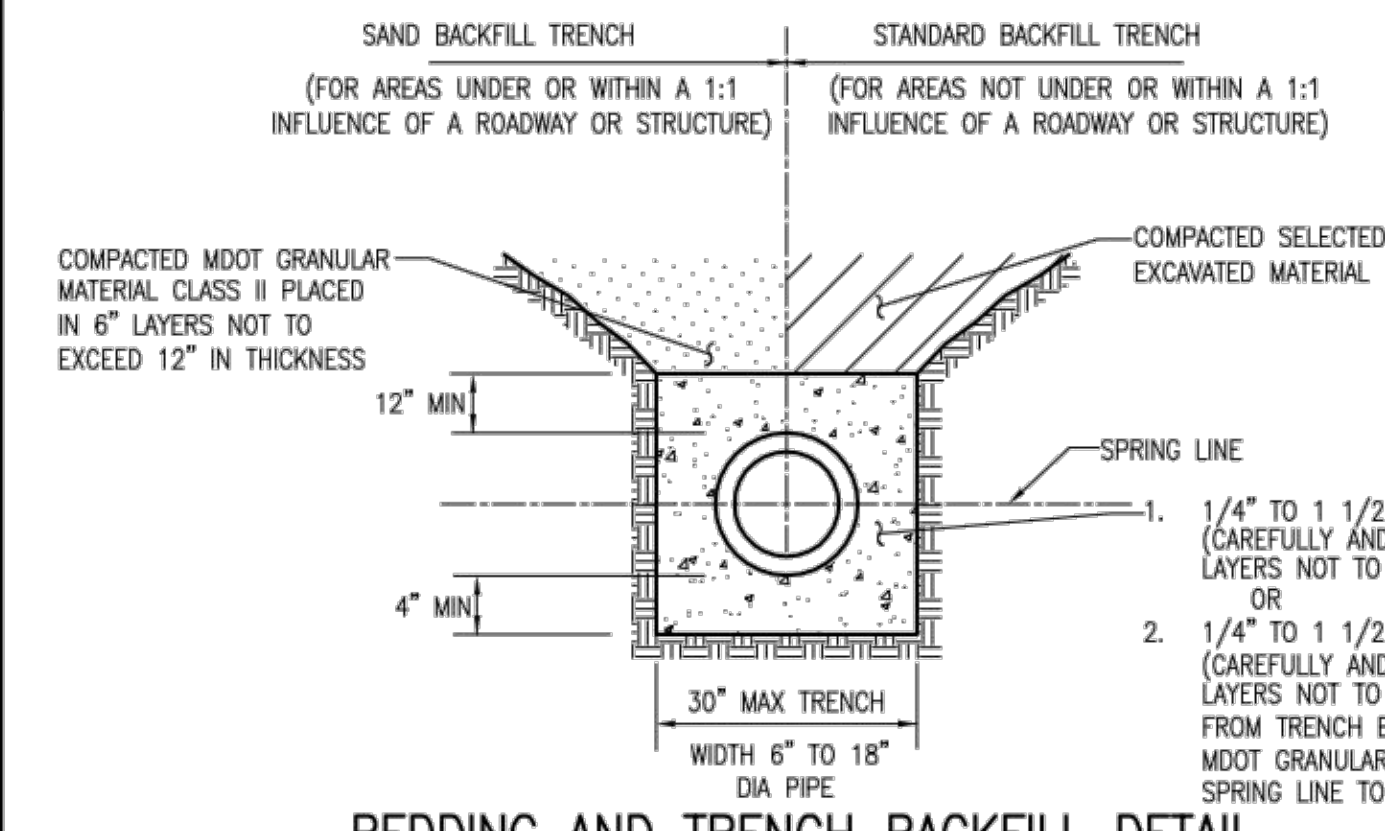
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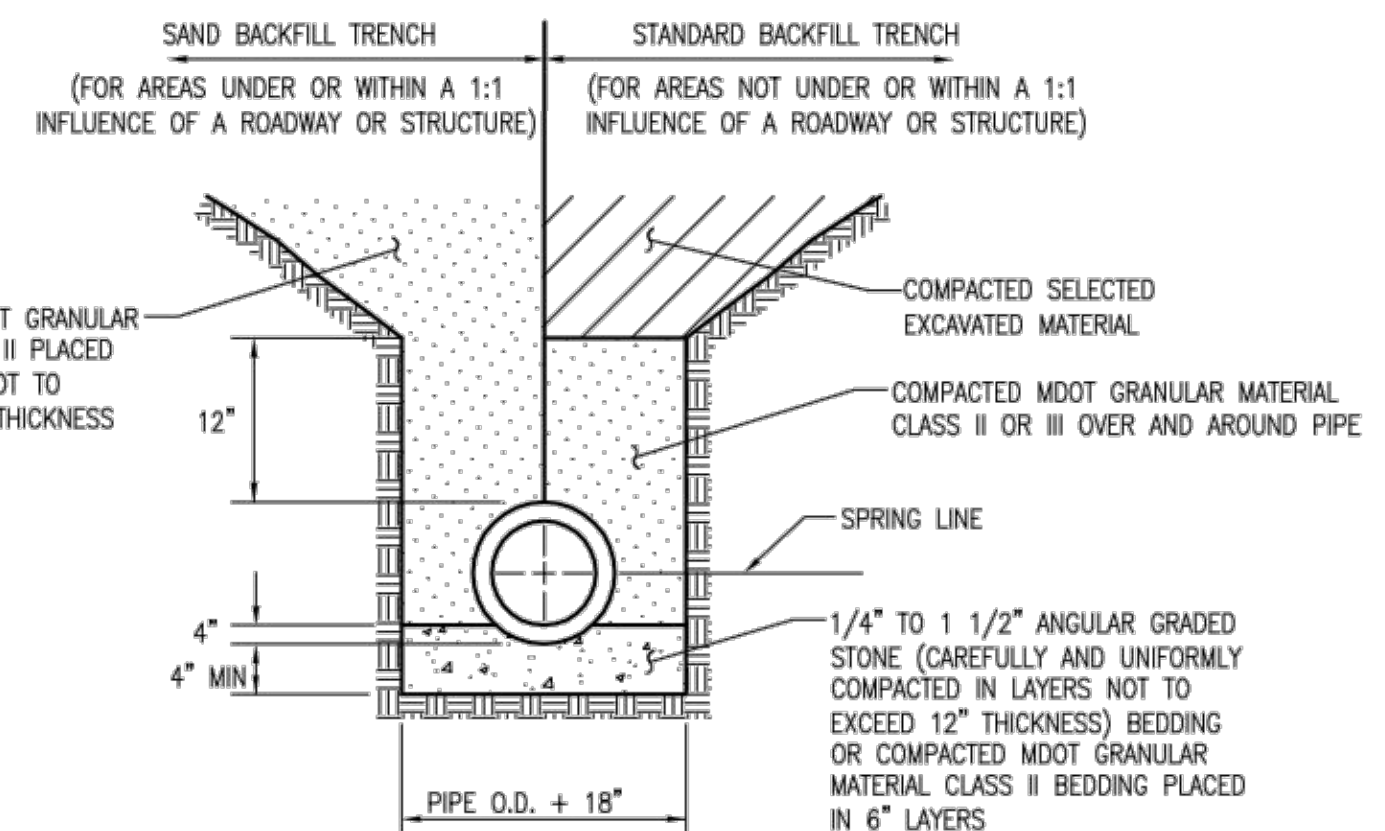
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DATE	OCT 2003	DATE	
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SECTION		SECTION	
TOWN		TOWN	
RANGE		RANGE	
COUNTY	WASHTENAW	COUNTY	WASHTENAW
CITY/TOWNSHIP	CHARTER TOWNSHIP OF SUPERIOR	CITY/TOWNSHIP	CHARTER TOWNSHIP OF SUPERIOR
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V. N/A		V. N/A	
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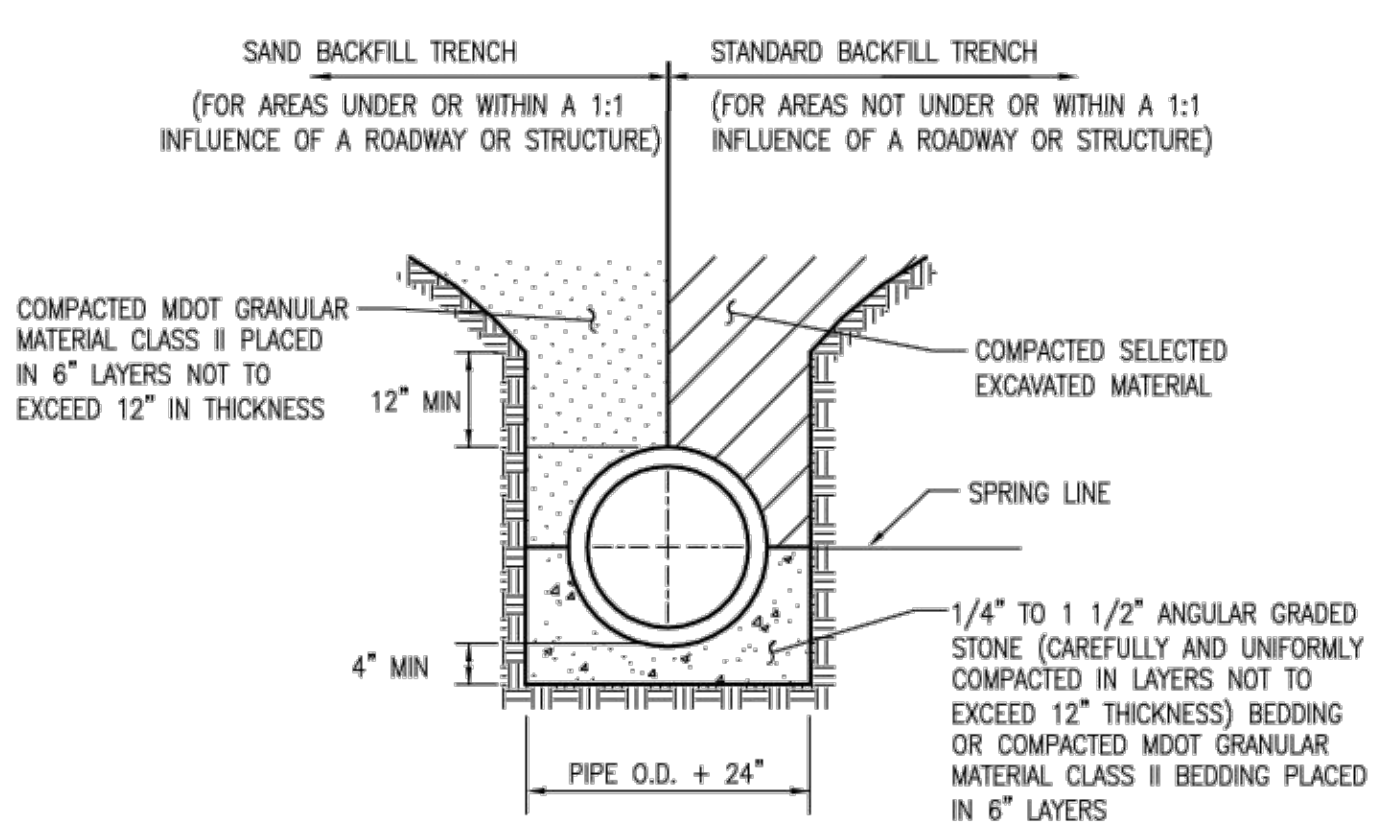
CHARTER TOWNSHIP OF SUPERIOR
STANDARD STORM SEWER DETAILS



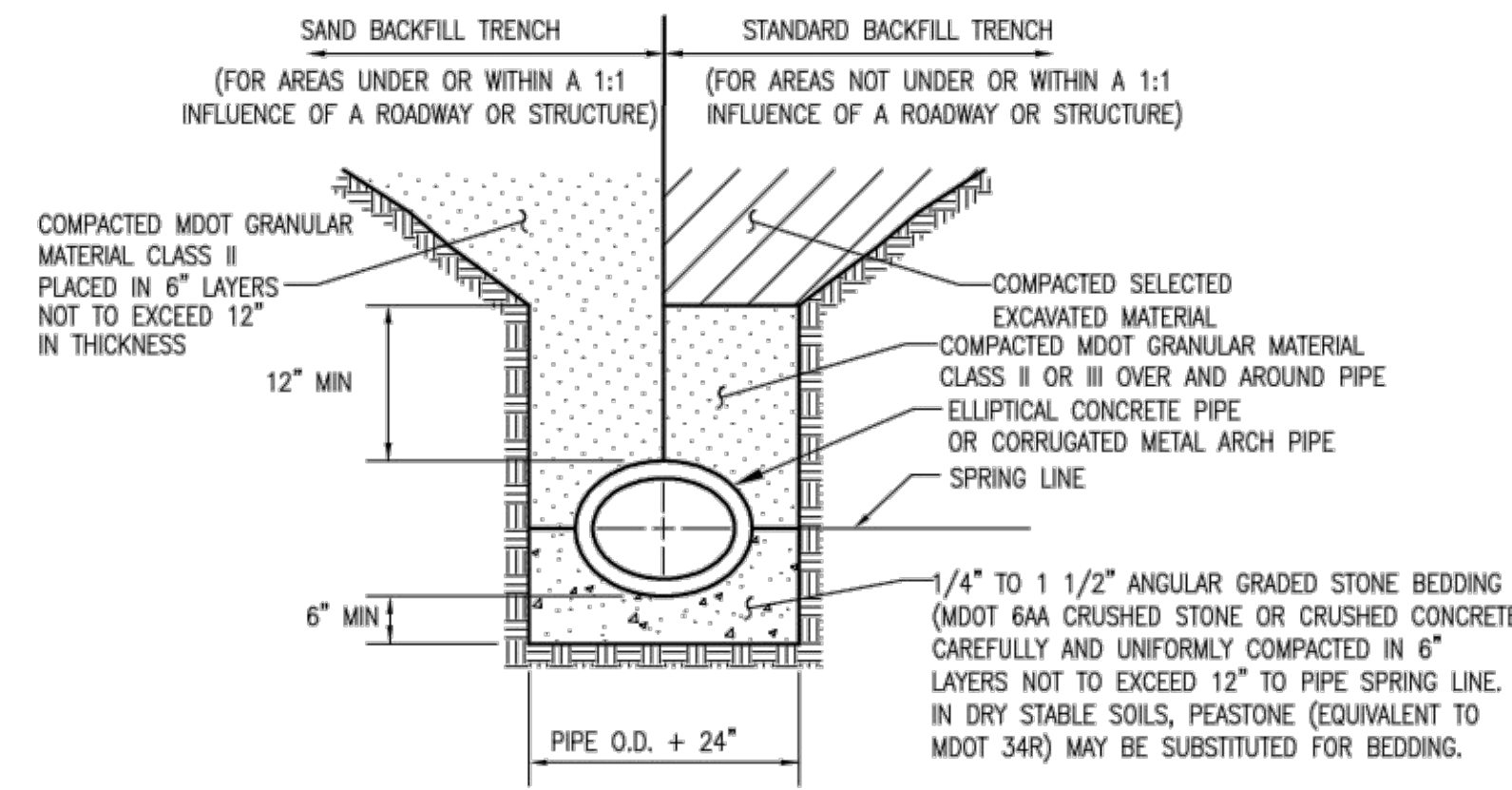
BEDDING AND TRENCH BACKFILL DETAIL FOR 18" DIAMETER AND SMALLER PIPE (PVC SOLID WALL, AND TRUSS PIPE)



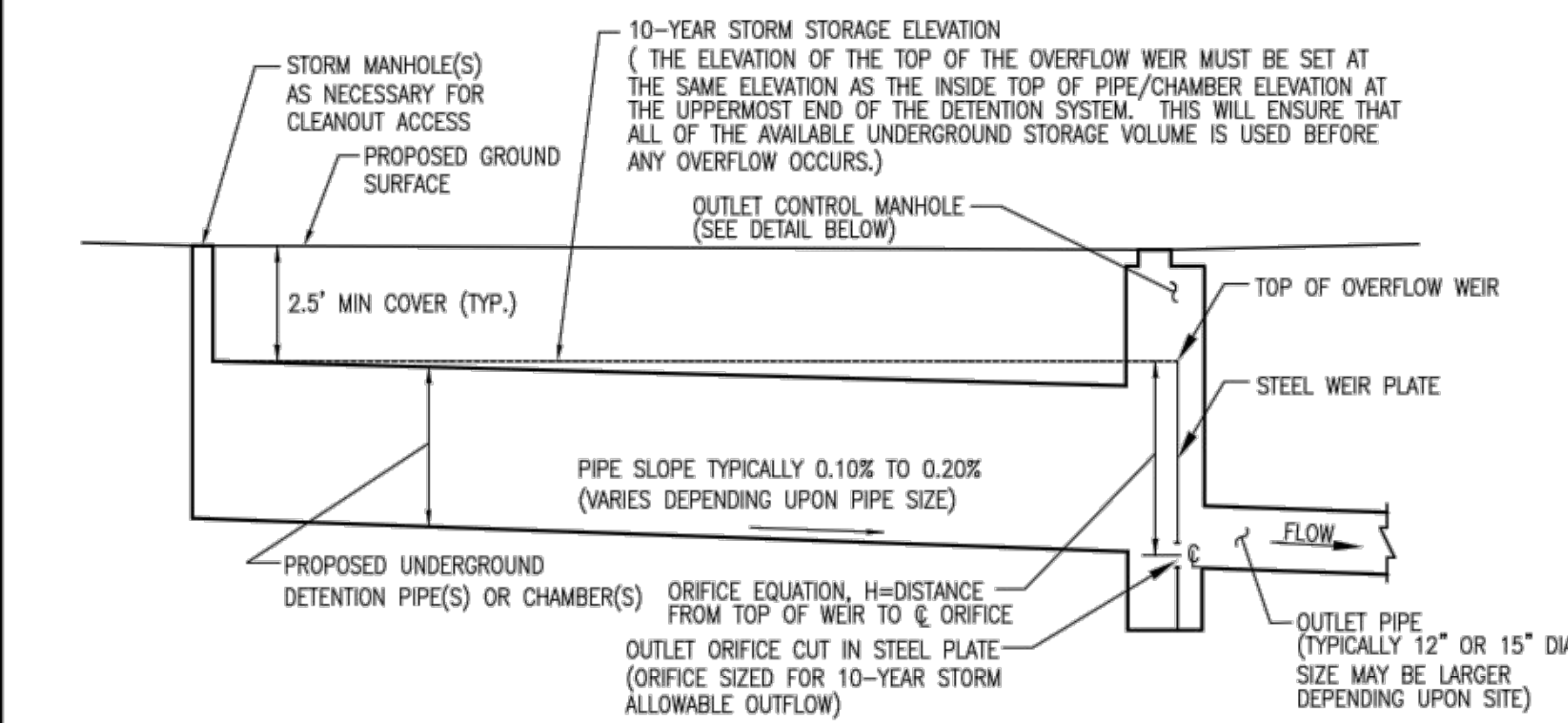
BEDDING AND TRENCH BACKFILL DETAIL FOR 24" DIAMETER AND SMALLER PIPE



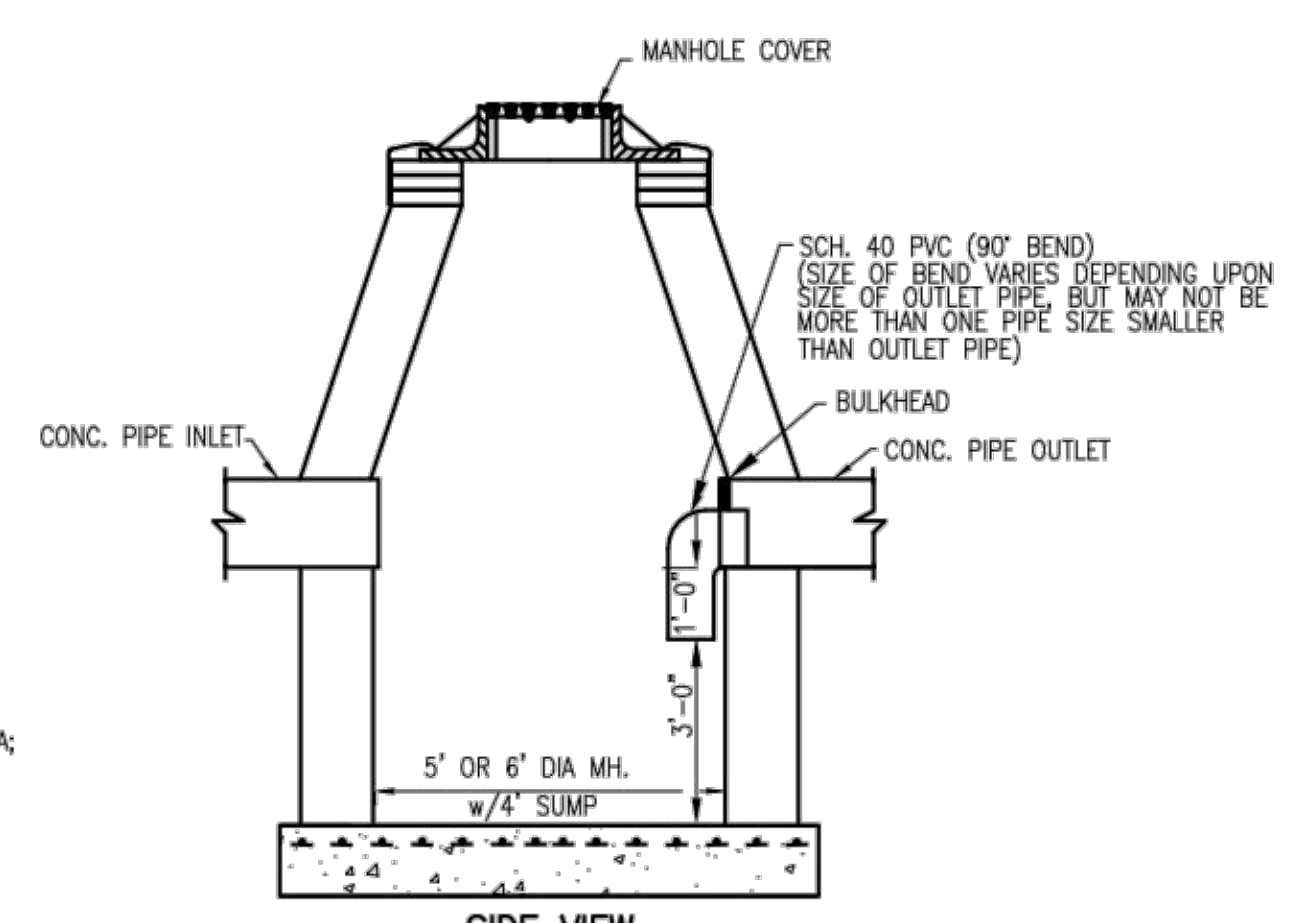
BEDDING AND TRENCH BACKFILL DETAIL FOR 27" DIAMETER AND LARGER PIPE



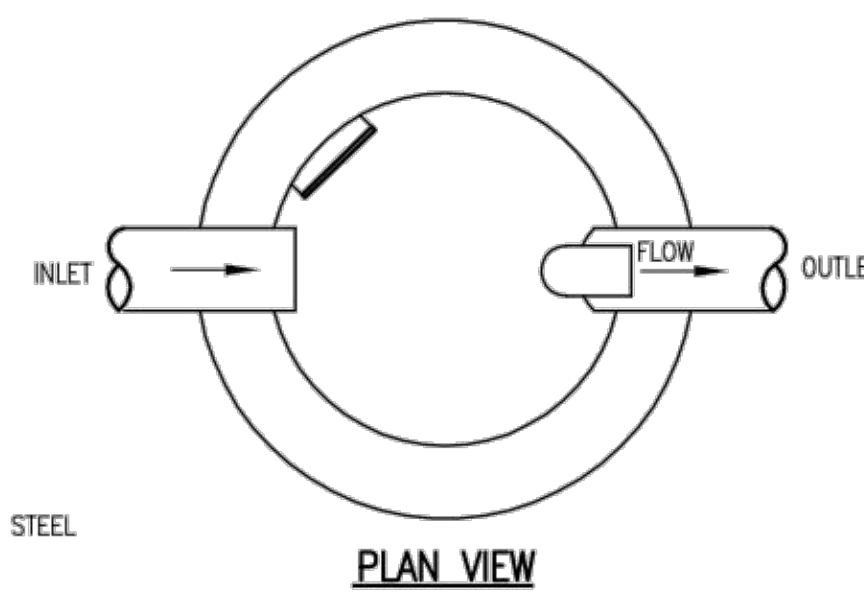
BEDDING AND TRENCH BACKFILL DETAIL FOR ELLIPTICAL CONCRETE PIPE OR CORRUGATED METAL ARCH PIPE



DETENTION SYSTEM PROFILE

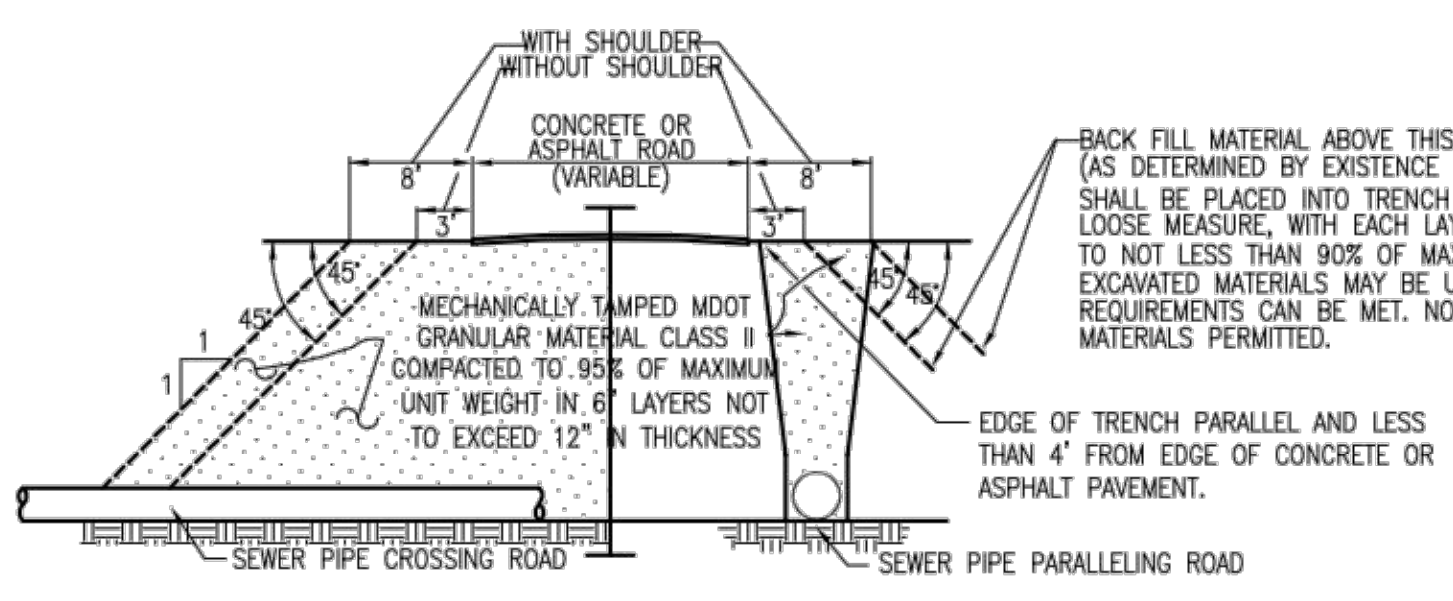


SIDE VIEW

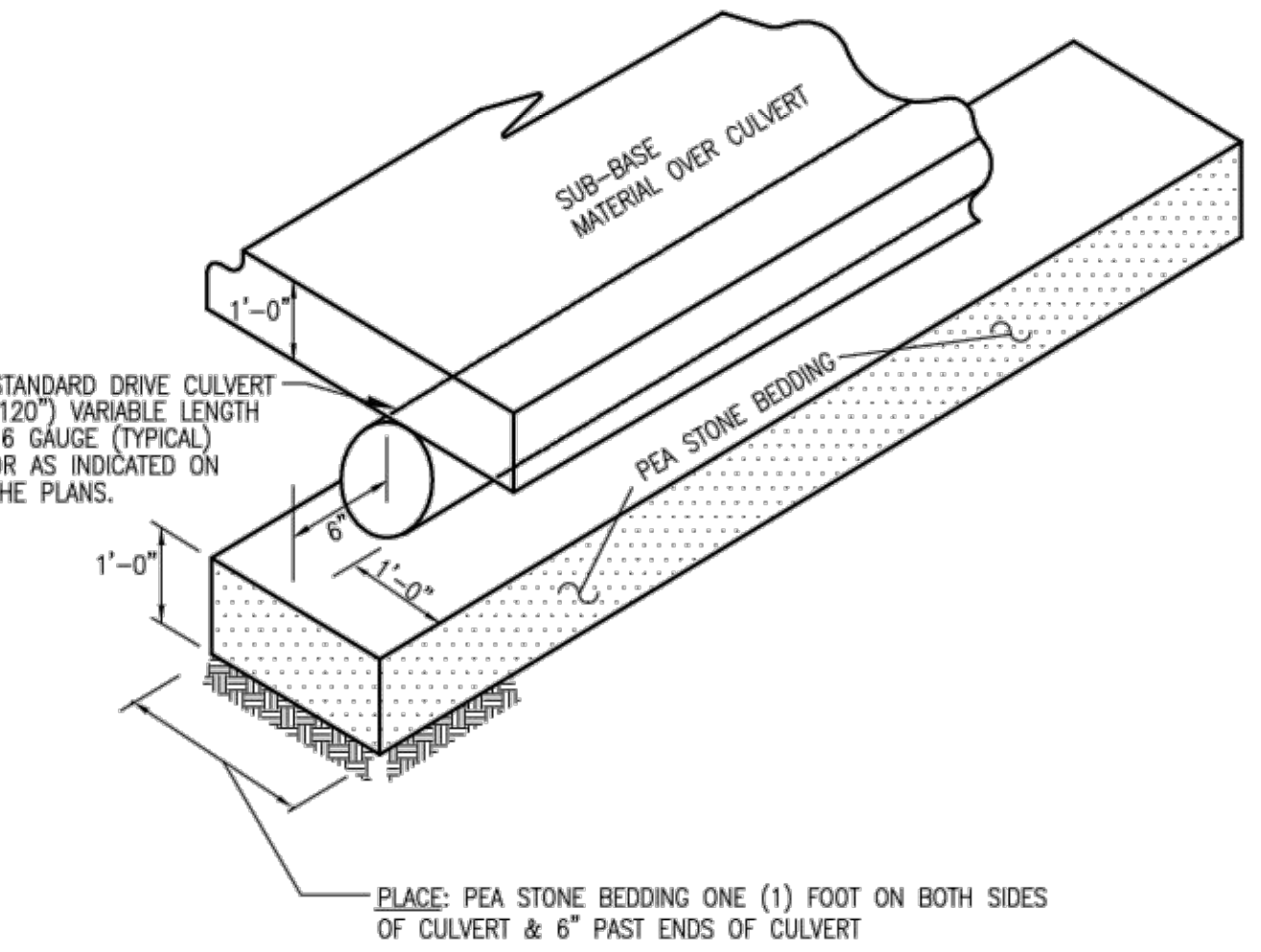


PLAN VIEW

OIL/GAS SEPARATOR PLACEMENT DETAIL FOR 18" DIAMETER AND SMALLER OUTLET PIPE (FOR OUTLET PIPES LARGER THAN 18" IN DIAMETER, AN ALTERNATE DESIGN MUST BE APPROVED BY THE TOWNSHIP ENGINEER)



SAND OR GRAVEL BACKFILL DETAILS FOR SEWERS UNDER CONCRETE OR ASPHALT PAVEMENTS, SIDEWALKS, DRIVEWAYS AND PARKING AREAS



DRIVE CULVERT BEDDING DETAIL

GENERAL NOTES FOR STORM SEWER CONSTRUCTION

- PIPE BEDDING AND BACKFILLING:**

BEDDING SHALL EXTEND A MINIMUM OF 4" BELOW PIPE, UNLESS OTHERWISE NOTED ON CONSTRUCTION PLANS. BEDDING SHALL BE UNIFORM GRADE. HOWEVER, IF THE EXISTING NATIVE SOILS MEET THE REQUIREMENTS FOR MDOT GRANULAR MATERIAL CLASS II (MINIMUM 4" THICK), THEN STORM STORM SEWER MAY BE LAID DIRECTLY ON COMPACTED NATIVE SUBGRADE SOILS.

BACKFILL SHALL BE COMPACTED ABOVE PIPE OR AS INDICATED ON CONSTRUCTION DRAWINGS. TRENCH BACKFILL SHALL BE OF A SUITABLE MATERIAL AND SHALL BE FREE OF ANY ORGANIC MATERIALS AND ROCKS LARGER THAN 3" IN SIZE. BACKFILL SHALL BE RAMMED INTO TRENCH AND COMPACTED WITH A SMALL DOZER OR OTHER APPROVED METHODS. WHERE TRENCH IS WITHIN A 1:1 INFLUENCE OF STREETS, ALLEYS, SIDEWALKS, DRIVEWAYS AND PARKING AREAS, SAND BACKFILL SHALL BE USED WHICH SHALL CONSIST OF MDOT GRANULAR MATERIAL CLASS II OR III COMPACTED IN 6" LAYERS NOT TO EXCEED 12" TO A DENSITY OF 95% AS DETERMINED BY AASHTO T99. ALL BACKFILL PLACED WITHIN A 1:1 INFLUENCE OF STRUCTURES SHALL BE APPROVED SAND, PLACED IN 1" LAYERS AND COMPACTED. NO FROZEN MATERIAL SHALL BE BURIED MORE THAN 4' BELOW THE FINAL ELEVATION OF THE GROUND.

TRENCHES WHICH ARE TO BE LEFT OPEN OVERNIGHT SHALL BE ENCLOSED WITH SUITABLE FENCING AND LIGHTED BARRICADES, UNLESS OTHERWISE APPROVED BY THE TOWNSHIP.
- SUMP PUMP LEAD REQUIREMENTS:**

ALL SUMP PUMP LEADS CONNECTED TO A DRAIN SHALL BE PRE-MANUFACTURED.

SUMP PUMP MAINS AND LEADS SHALL BE A SDR 35, NON-PERFORATED, SOLID WALL, PVC, ARMO CO TRUSS PIPE, OR APPROVED EQUAL, WITH PREMIUM JOINTS.

TAPS TO 12" STORM SEWER SHALL BE MADE WITH A FERROD EZ TAP OR APPROVED EQUAL. TAPS TO OTHER SIZE STORM SEWER SHALL BE MADE WITH A ROMAC SADDLE, KOR-N-TEE LATERAL CONNECTOR FOR CONCRETE PIPE, OR APPROVED EQUAL.

ENDS OF ALL 4" SUMP PUMP LEADS SHALL BE TEMPORARILY CAPPED AND THEIR LOCATION STAKED, WITNESSED AND RECORDED.

ALL SUMP PUMP LEADS TO BE TAKEN TO THE PROPERTY LINE, EASEMENT LINE OR AS INDICATED ON THE PLAN.

SUMP PUMP CLEANOUTS SHALL BE A MINIMUM INSIDE DIAMETER OF 24" AND BE CONSTRUCTED AT CHANGES OF ALIGNMENT, ENDS OF SUMP PUMP MAINS OR AS INDICATED ON THE PLAN.
- RESTORATION REQUIREMENTS:**

ALL DISTURBED AREAS WITHIN THE RIGHT-OF-WAY SHALL BE RESTORED AS FOLLOWS, UNLESS OTHERWISE NOTED ON CONSTRUCTION DRAWINGS:

FINISH GRADE

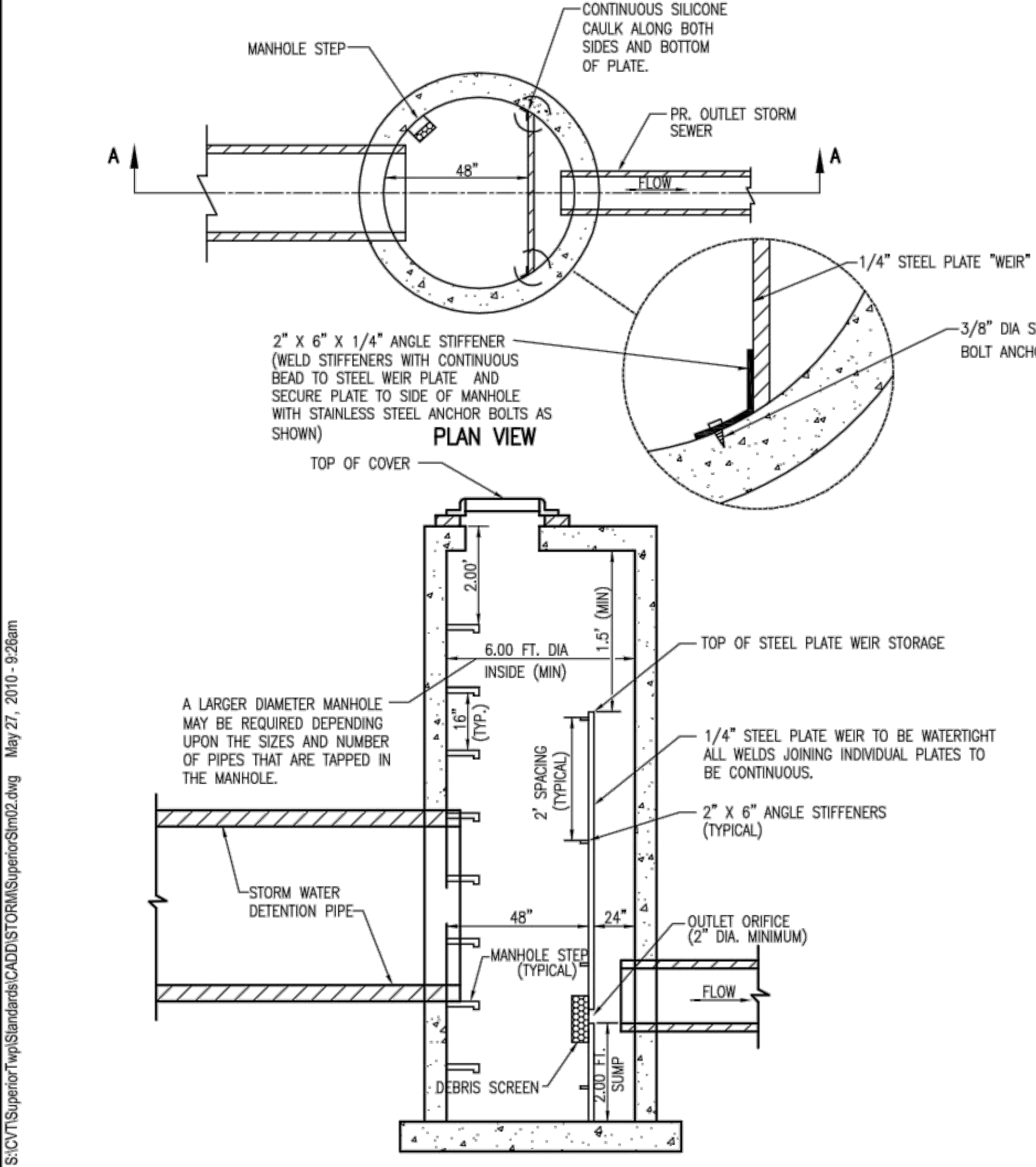
PLACE 3" THICKNESS OF "QUALITY" TOPSOIL ACCEPTABLE TO THE ENGINEER.

APPLY SOD OR SEED AND FERTILIZER AS FOLLOWS:

LOCATION	SODDING/ SEEDING REQUIREMENTS	FERTILIZER REQUIREMENT
SLOPES & DITCH BANKS, ETC.	MDOT "ROADSIDE" MIX (50% PERENNIAL RYE, 15% KENTUCKY BLUE, 35% RED FESCUE) APPLIED AT 100 LBS/ACRE	240 LBS/ACRE OF CHEMICAL FERTILIZER NUTRIENTS IN EQUAL PROPORTIONS OF NITROGEN, PHOSPHORIC ACID AND POTASH. (MUST BE A SLOW-RELEASE FERTILIZATION)
OTHER AREAS	MDOT "CLASS A" MIX (30% PERENNIAL RYE, 30% KENTUCKY BLUE, 40% RED FESCUE) APPLIED AT 100 LBS/ACRE	240 LBS/ACRE OF CHEMICAL FERTILIZER NUTRIENTS IN EQUAL PROPORTIONS OF NITROGEN, PHOSPHORIC ACID AND POTASH. (MUST BE A SLOW-RELEASE FERTILIZATION)
DITCH BOTTOMS, SLOPES EXCEEDING 3:1, AND AT STRUCTURES	3" TOPSOIL WITH CLASS A SOD	

APPLY STRAW MULCH AT THE RATE OF 2-3 BALES/1000 SQUARE FEET.

THE CONTRACTOR SHALL BE RESPONSIBLE TO INSURE THE GROWTH OF ALL SEEDED AREAS, AND SHALL RE-SEED AS NECESSARY TO ACCOMPLISH THIS.



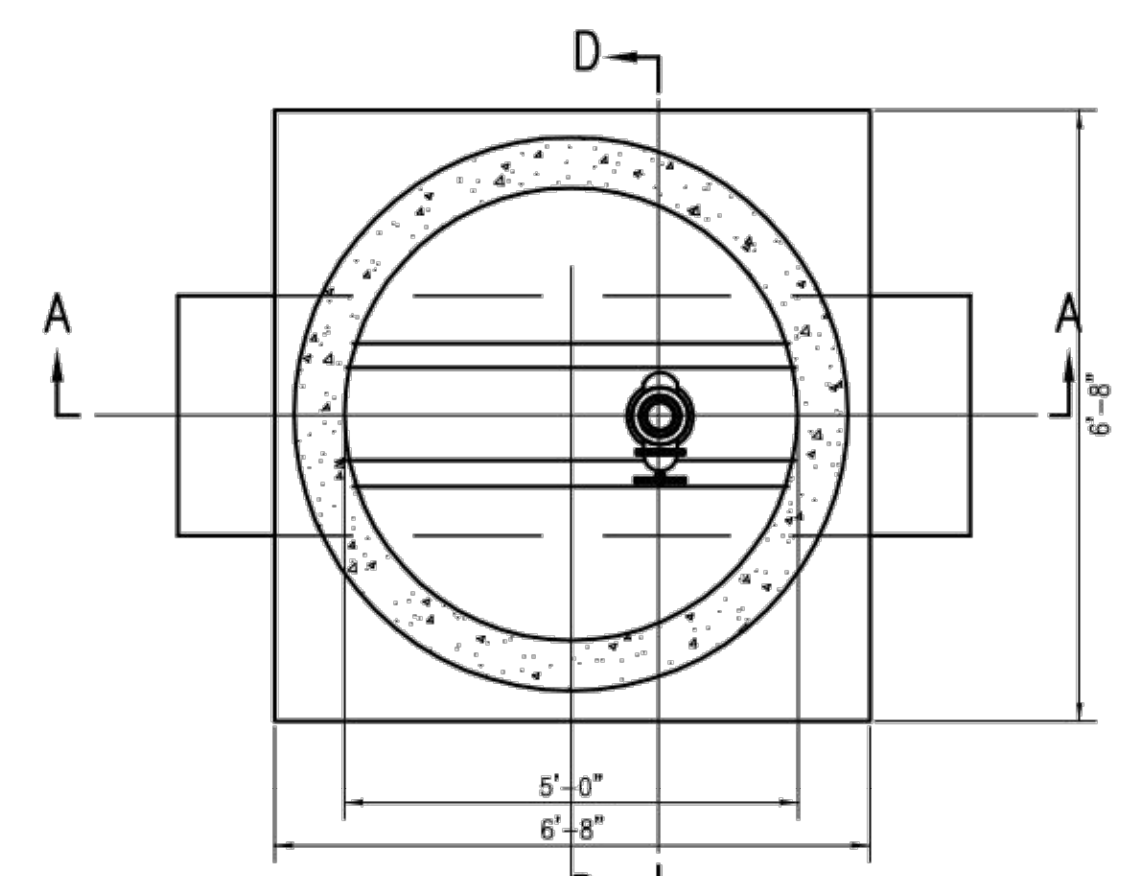
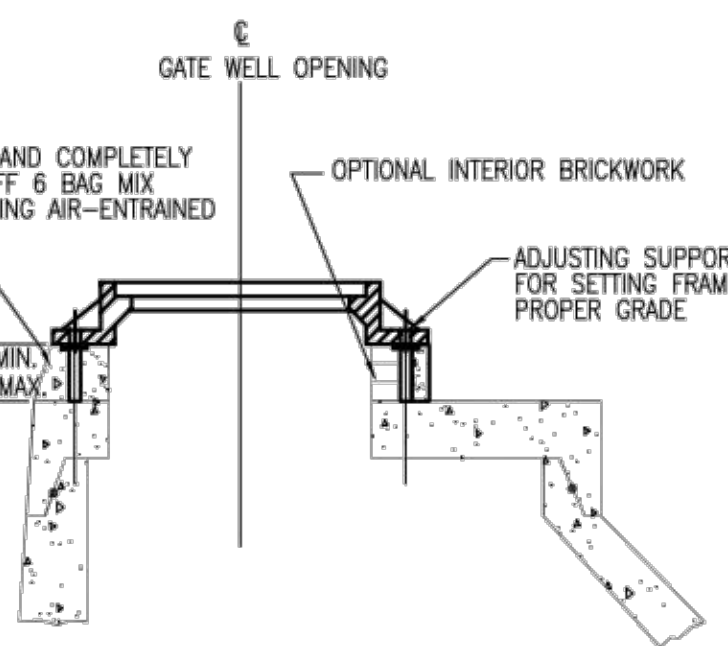
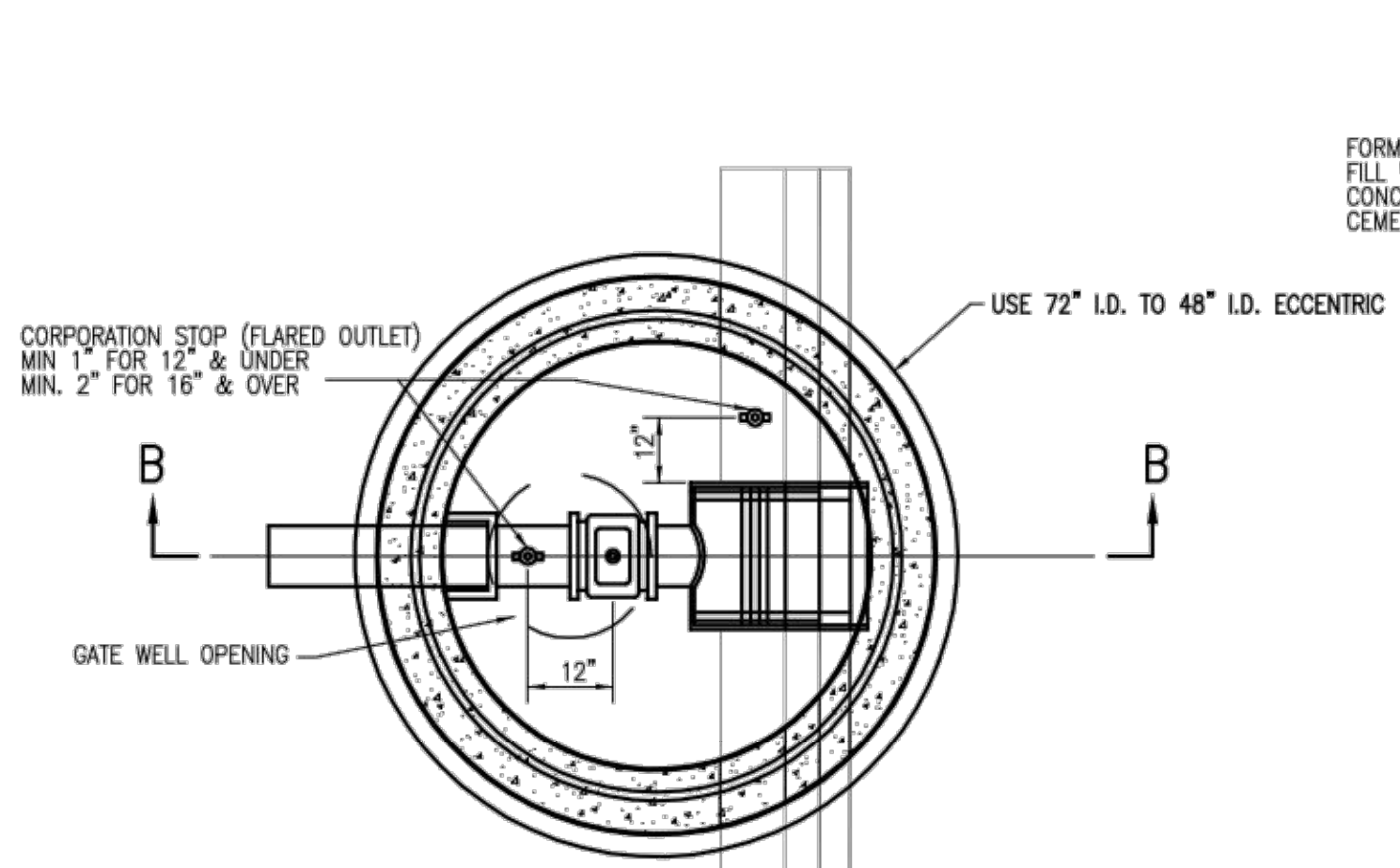
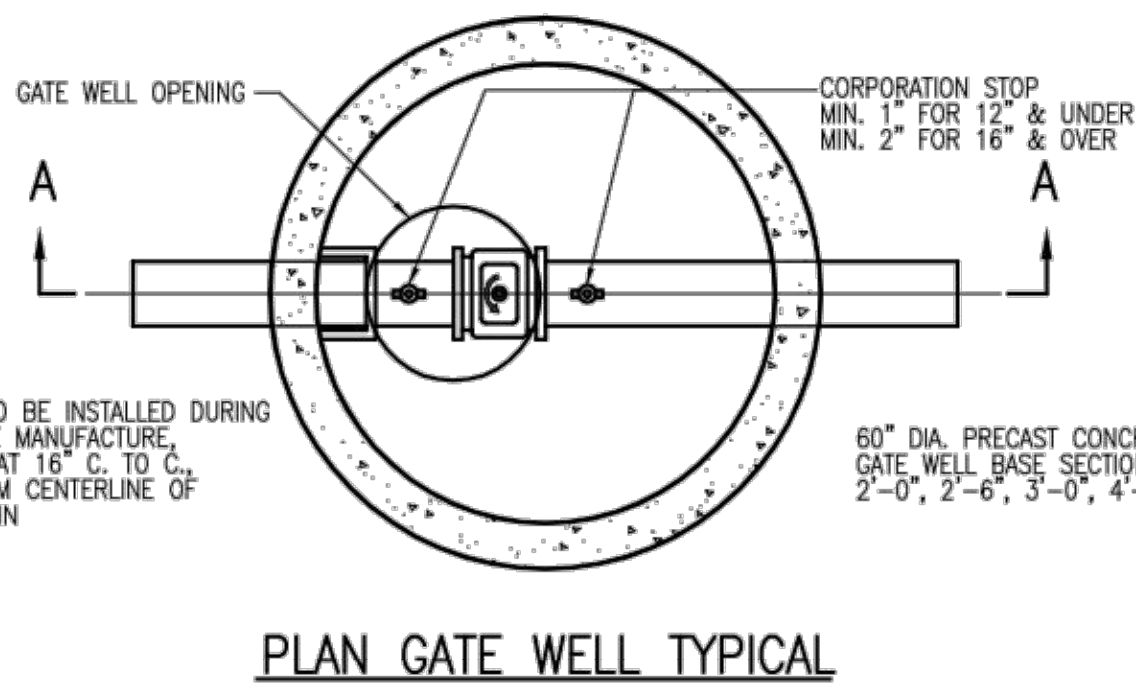
**SECTION A-A
6 FT. DIA OUTLET MANHOLE**

TYPICAL UNDERGROUND DETENTION AND OUTLET MANHOLE DETAILS

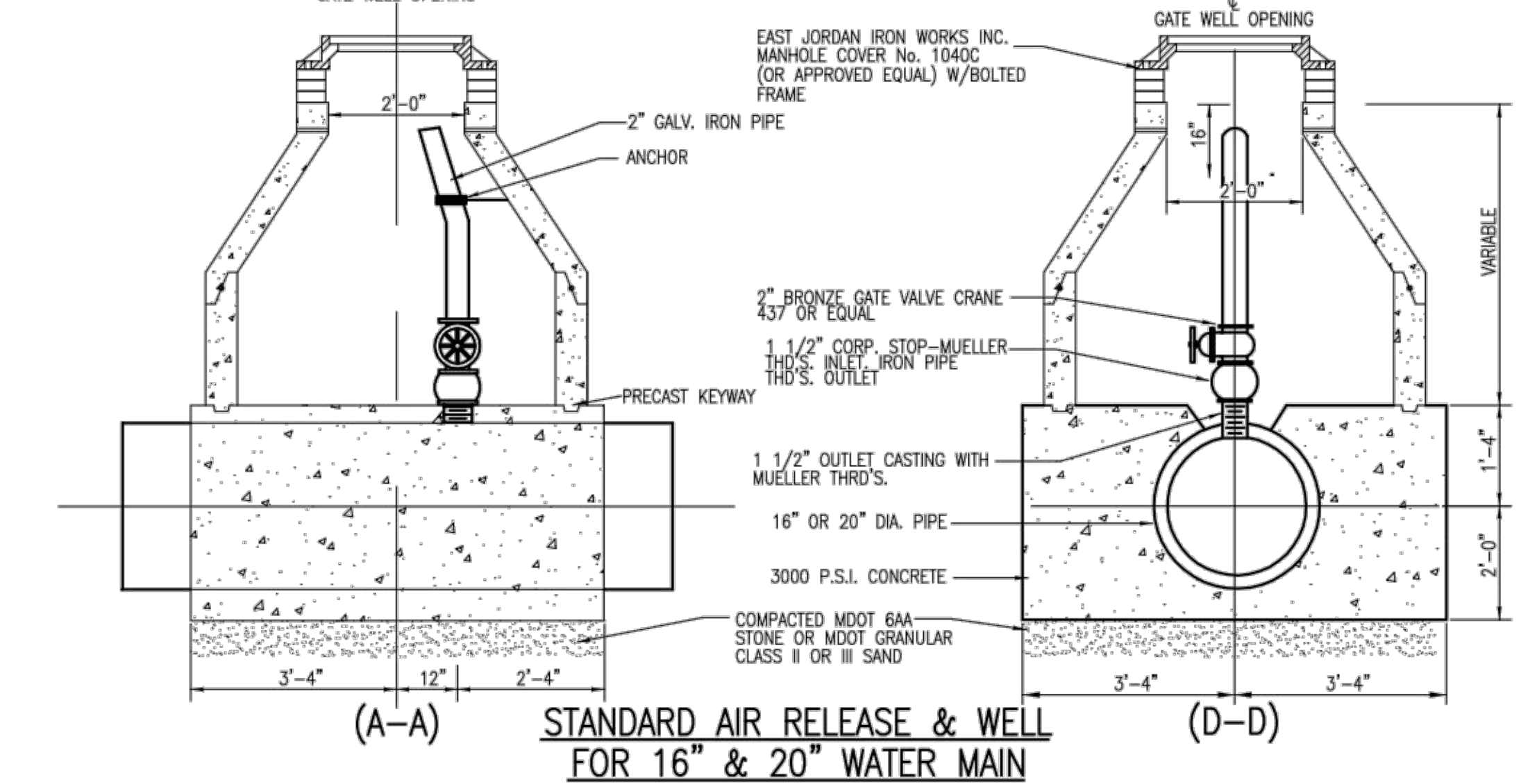
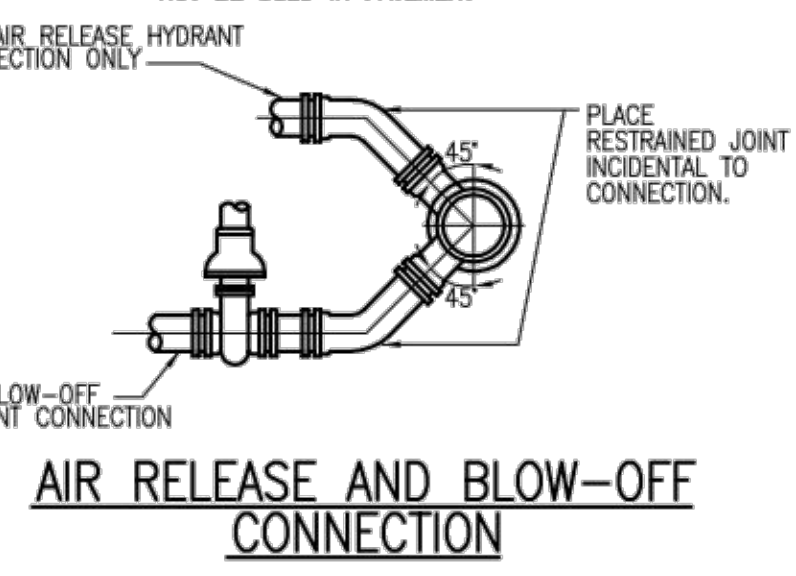
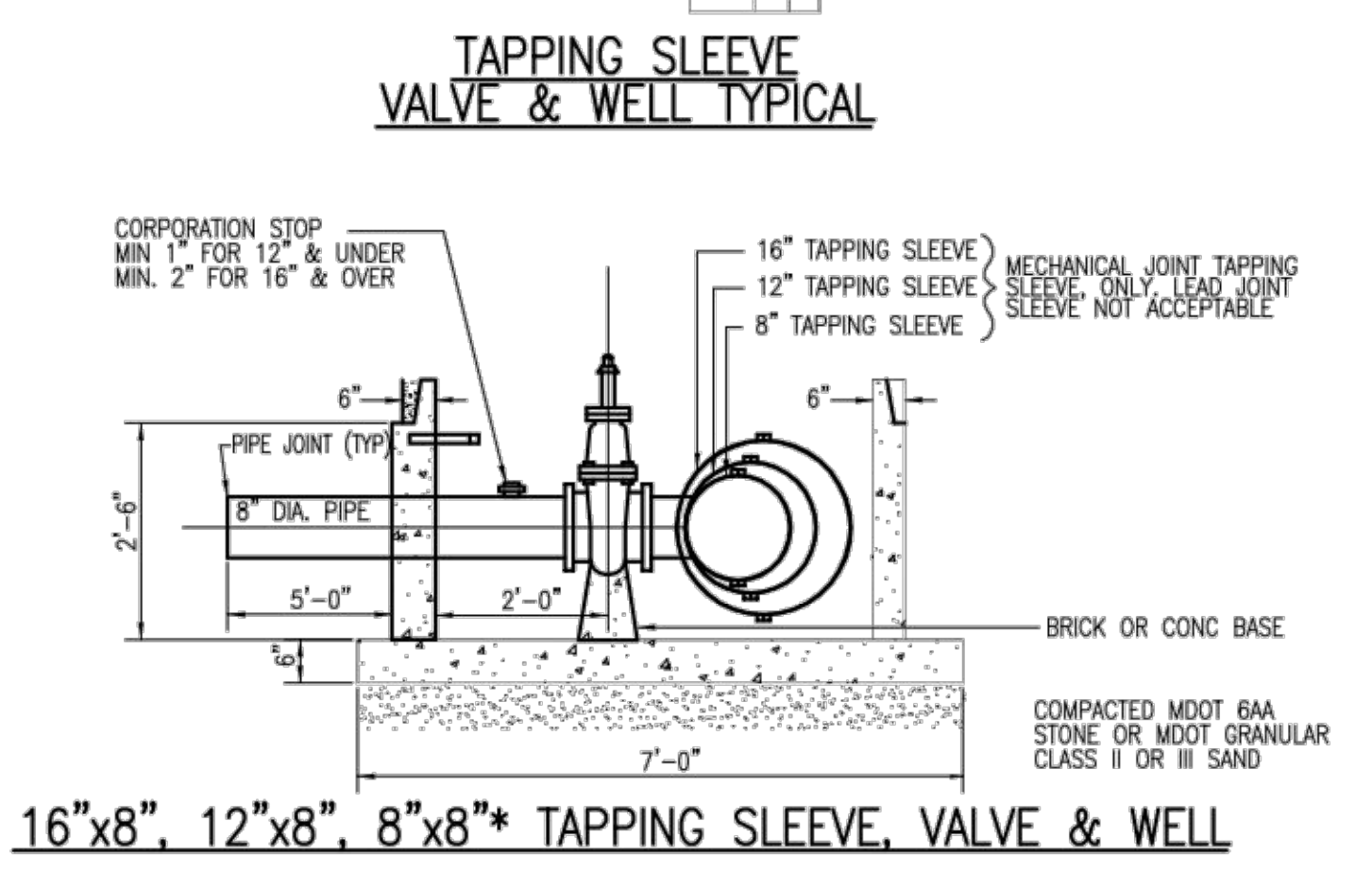
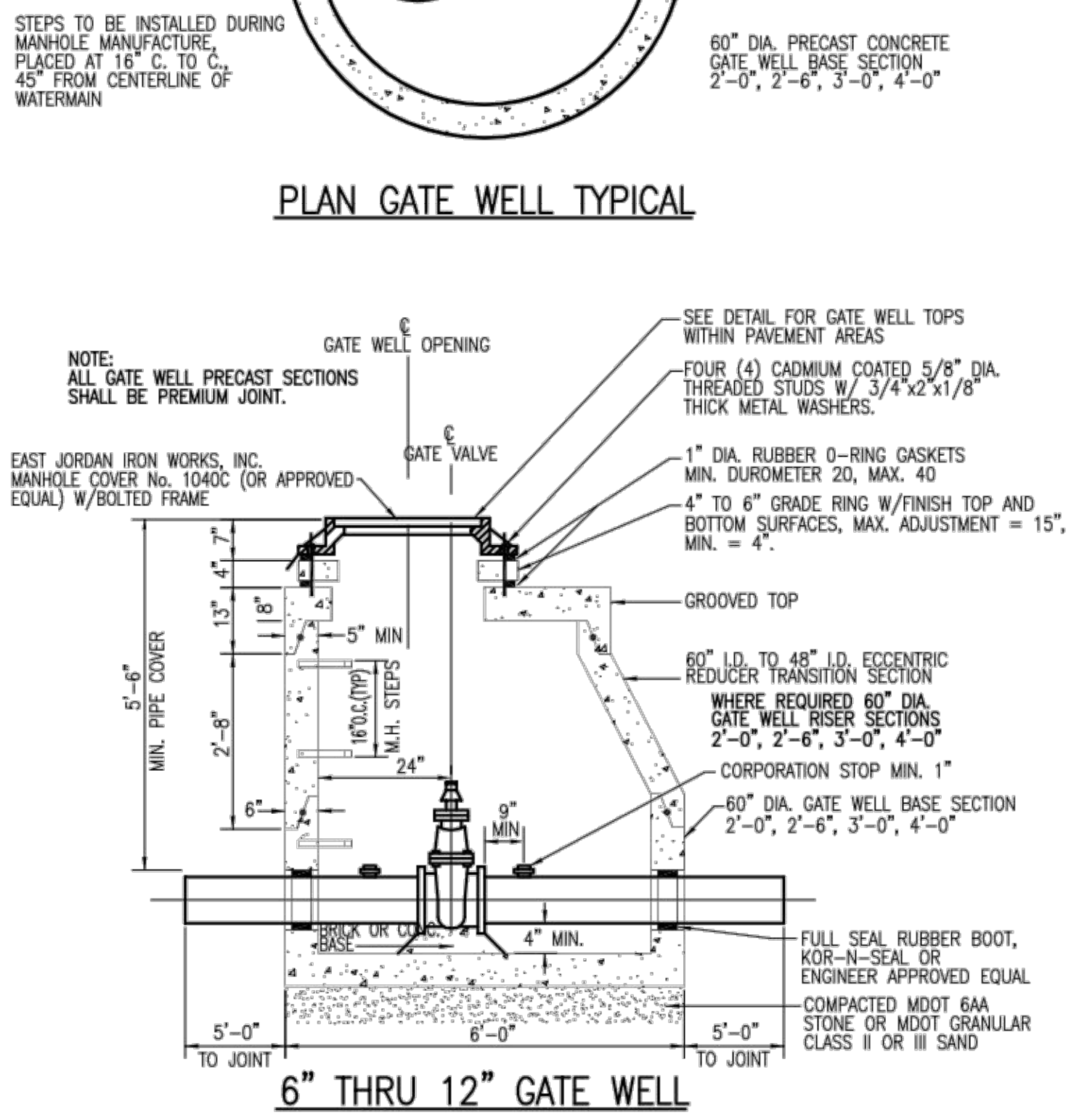
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DATE	OCT 2003	DESIGNED BY	DATE	OCT 2003
CAD	DK	DRAWN BY	DATE	OCT 2003
PROJ. NO.		CHECKED BY	DATE	OCT 2003
SECTION		CLIENT	CHARTER TOWNSHIP OF SUPERIOR	
TOWN		RANGE	CHARTER TOWNSHIP OF SUPERIOR	
COUNTY	WASHTENAW	CITY/TOWNSHIP	CHARTER TOWNSHIP OF SUPERIOR	
SCALE	V. N/A	H. NTS	000-00-0000	
VERTICALLY	N/A			

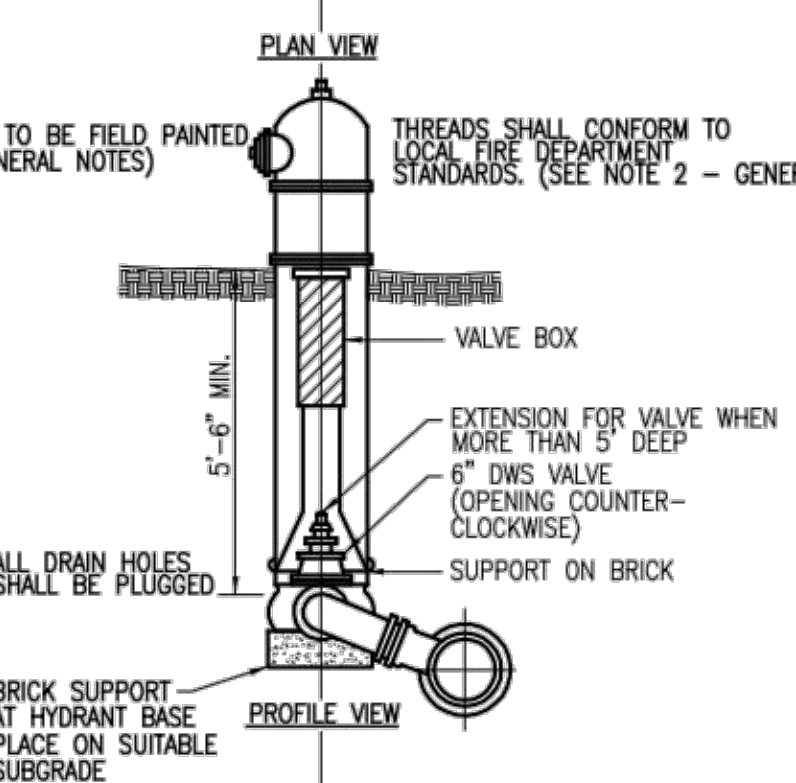
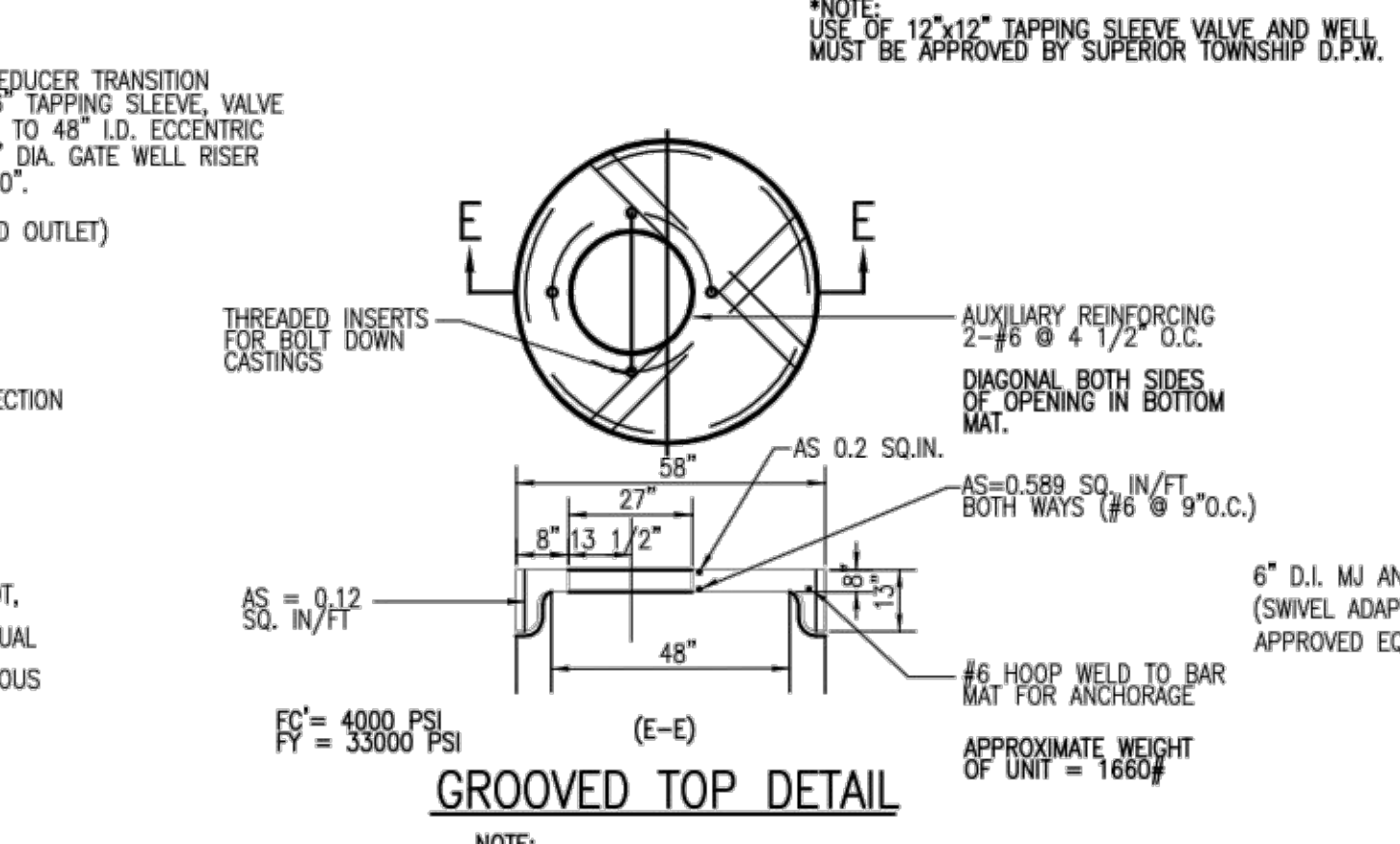
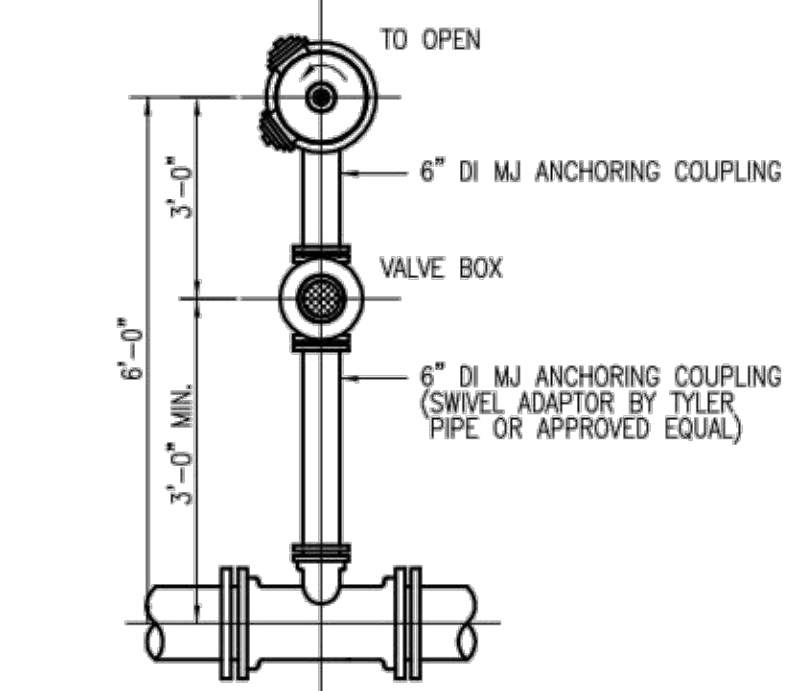
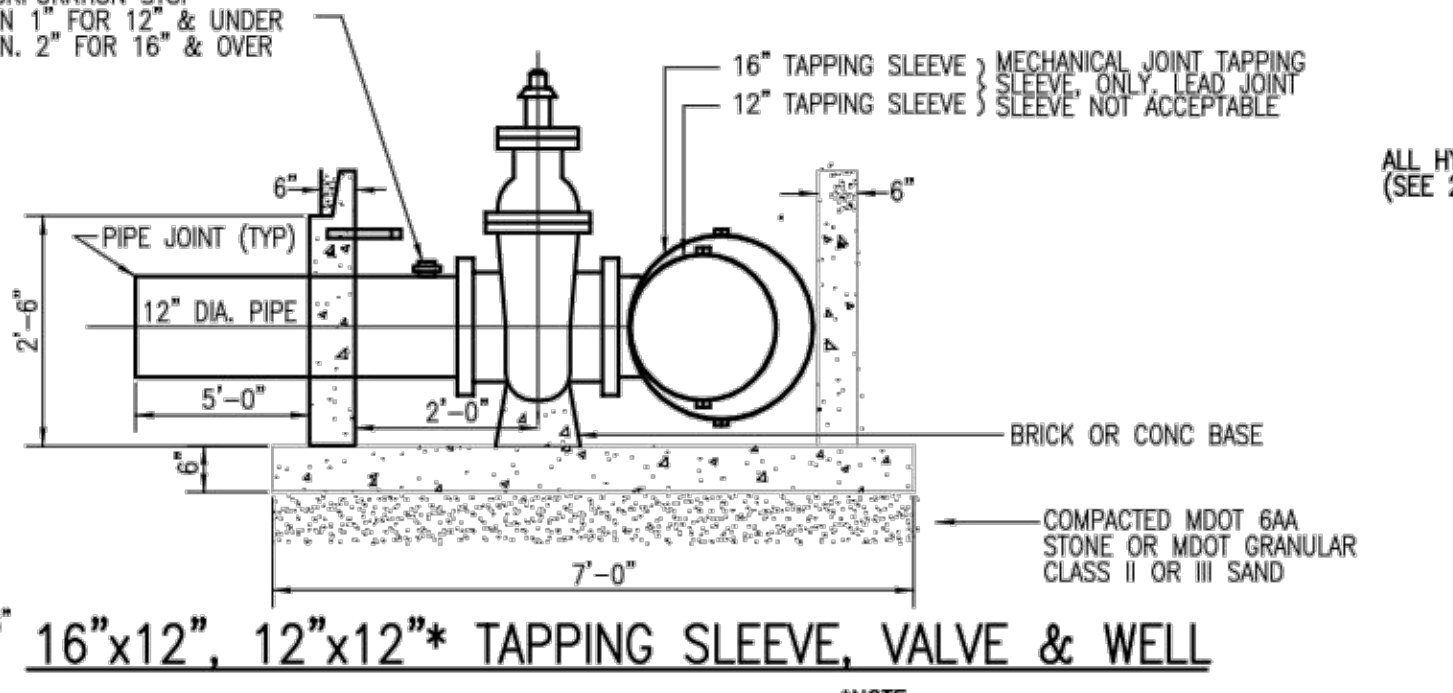
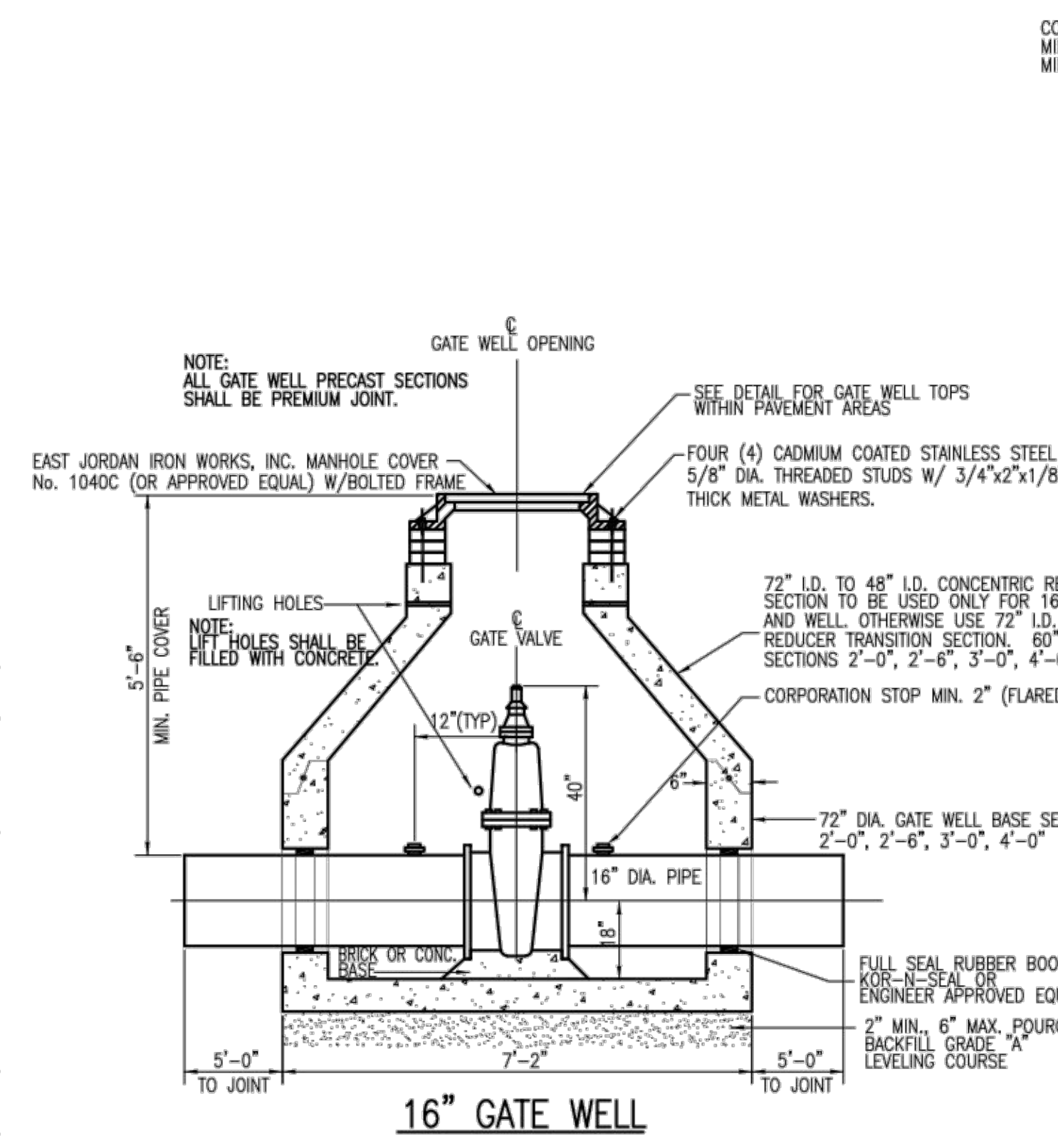


NOTES:
ALL AIR RELEASE WELLS SHALL BE PRECAST STRUCTURES.
ALL AIR RELEASE VALVES SHALL BE AUTOMATIC.



WATER MAIN NOTES

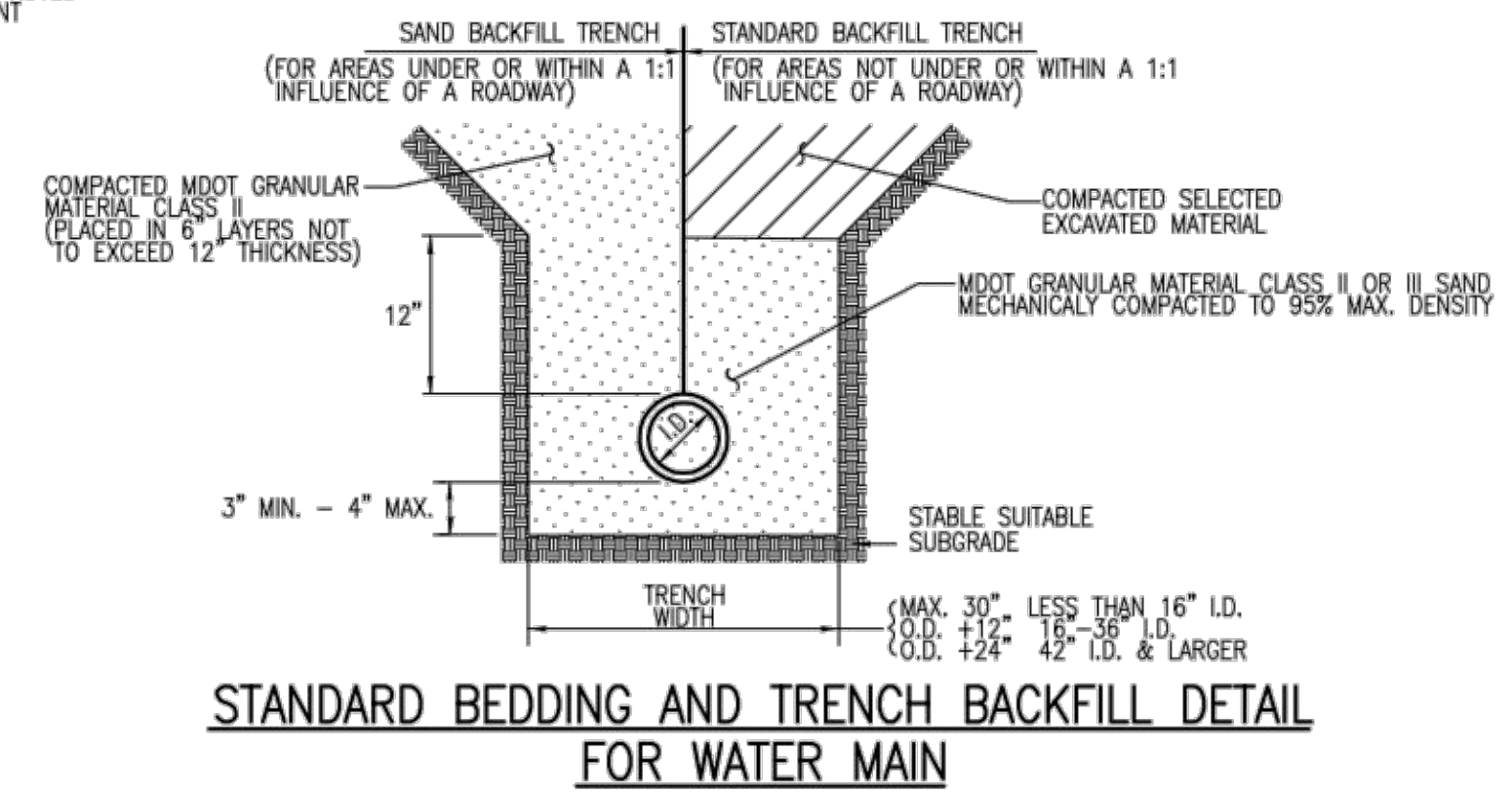
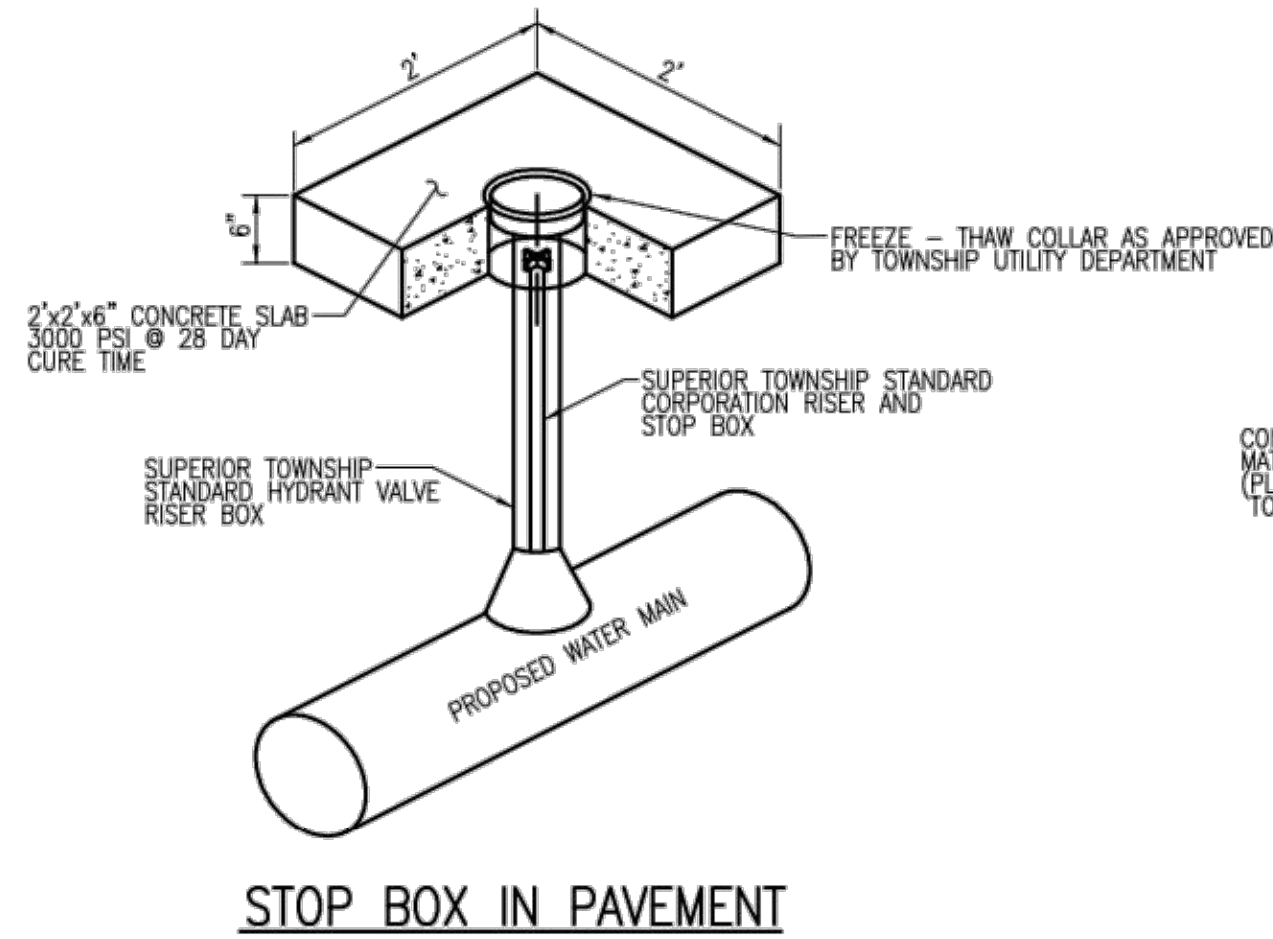
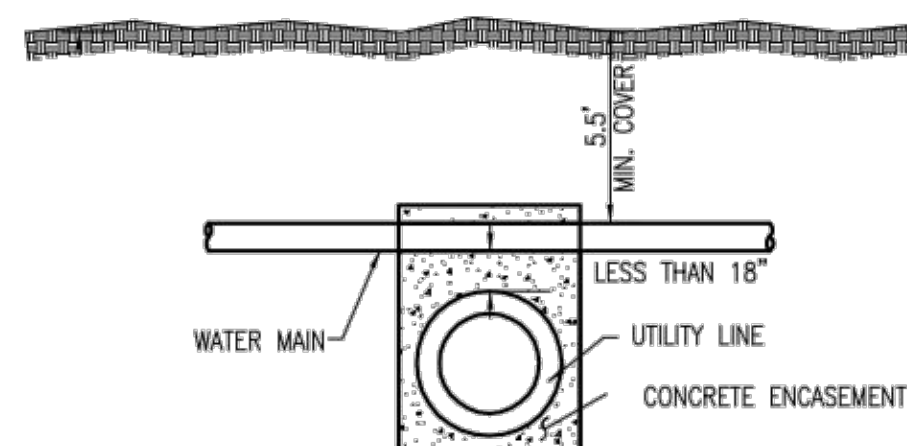
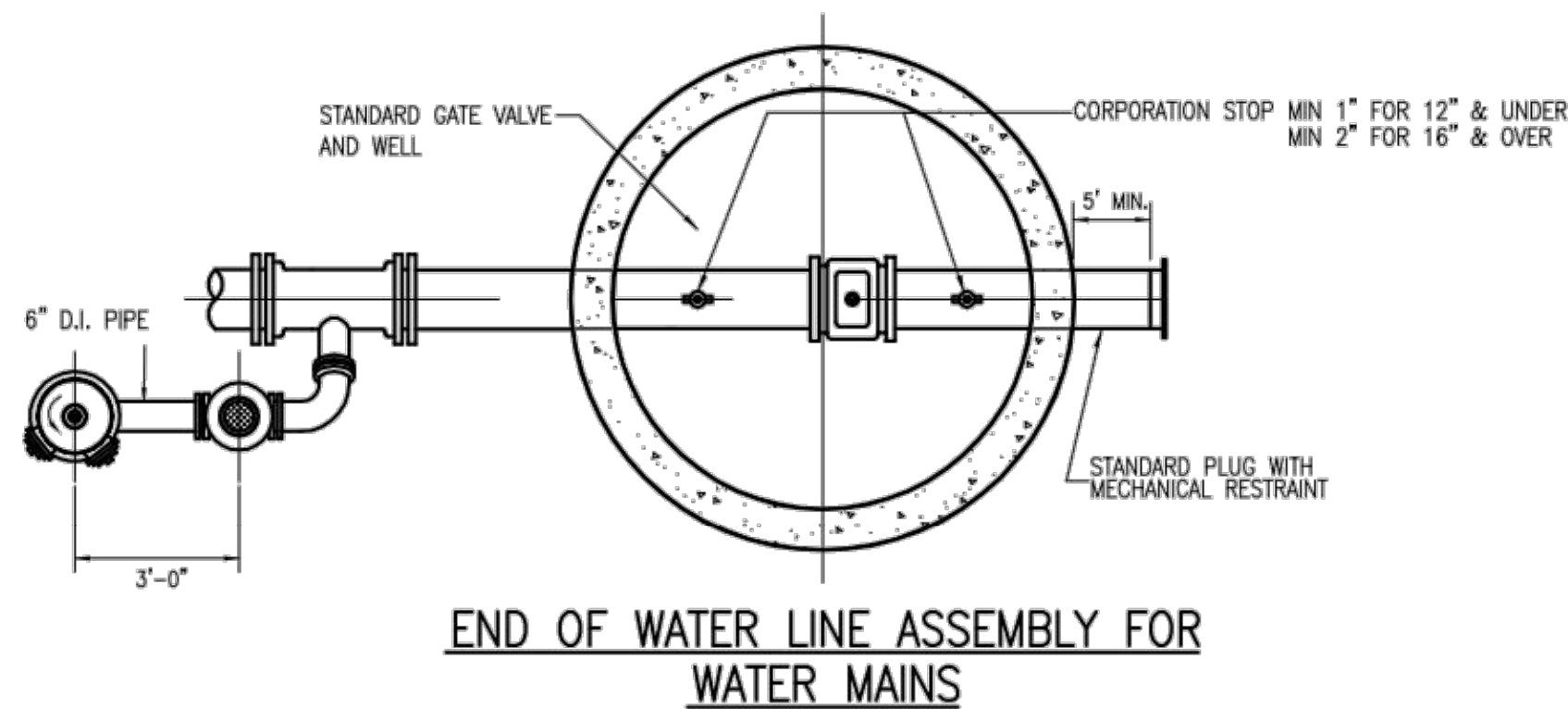
- All construction procedures and materials used on all water main projects shall conform to Ypsilanti Community Utilities Authority (Y.C.U.A.), Detroit Water System Department (D.W.S.D.), and the Charter Township of Superior current Standards and Specifications.
- All hydrants shall be Mueller A-425-Super-Centurion 250 or E.J.L.W. 5-BR Water Master and shall conform to AWWA current standards and shall have a 5 1/4" valve opening which closes with the water pressure. Hydrants shall be traffic style with breakable flange and coupling. Hydrants shall have a swivel flange to allow bonnet to be turned 360 degrees without removing the bonnet, and barrel flanges shall be integrally cast with the barrel. Inlet connection shall be 6" mechanical joint, ASA-A21-11. Stem threads shall be sealed with double 'O' rings and shall be permanently lubricated with all weather grease. Hose connections: Two (2) 4 1/2" pumper nozzles facing the centerline of the road unless otherwise directed by the Township. Pumper connections shall have: National Standard Threads on one pumper connection (4 1/2" I.D., 4 threads per inch, 5.389 minor dia., 5.761 major dia., 5.588 pitch dia.) and Slorz Fitting on the other pumper connection (R/S Manufacturing). Operating Nut: (1) 1 1/2" P-F pentagon, open left. Drain holes in hydrant shall be plugged and watertight. Hydrants shall be painted red above the ground and black below. All hydrants shall be constructed with companion valve in a three piece piece adjustable cast iron valve box.
- All water mains shall be ductile iron as per the following specification. Ductile iron pipe shall be ANSI 1-A21.51 (AWWA-C151) std. wall thickness, cement lined with bituminous seal coat Class 54 for sizes 6" through 16" Class 55 for 24" pipe, pipe sizes larger than 24" in nominal diameter shall meet all the requirements of the current AWWA C100 for ductile iron pipe.
- Gate Valves shall be iron body, fully bronze mounted, E.J.L.W. resilient wedge, non-rising stem, opening counterclockwise with 2" square operating nut conforming to Y.C.U.A. and D.W.S.D. specifications. All gate valves shall be provided with an extension stem. The length of the extension shall be such that it will be within 5' of ground surface when an extension stem is used. It shall be held in place by two extension stem guide assemblies. Each assembly shall be comprised of a "J" bracket and "L" bracket supplied by E.J.L.W. The stem guides shall be located opposite from each other, and shall be suitably fastened to the wall of the gate well. In addition, a "stop" shall be welded to the extension stem in a location that will prevent the extension stem from slipping off the operating nut. Details of extension stem and method of installation shall be approved by the engineer prior to installation. All precast concrete gate well sections shall be manufactured to conform with ASTM C478, standard specifications for precast reinforced concrete manhole sections, except wall thickness, shall be shown on these details. All joints for precast concrete gate well sections shall be "modified grooved tongue" with gasket manufactured to conform with ASTM C443, standard specification for joints for circular concrete sewer and culvert pipe rubber gaskets. All gate well covers shall be E.J.L.W. #1040C with bolted frame and have words "Superior Township Water Main and Logo" in raised letters on the frame cover, or approved equal.
- Tapping sleeve shall be mechanical joint with DWS Mechanical Joint Tapping Gate Valve. Lead joint sleeves shall not be used.
- No installation of water main is to be attempted without Township's inspector being present. Unless otherwise specified on plans, top of all water mains shall be 5.5 ft. below existing or proposed road centerline, or 5.5 ft. below existing or proposed ground, whichever results in lower elevation. An 18" minimum vertical clearance between storm or sanitary sewer shall be maintained.
- Three (3) working days before you dig, dial MISS DIG at 1-800-482-7171.
- All required cross connection devices shall be installed as required by the local plumbing inspector and in accordance with the standards of the Michigan Department of Public Health.
- The design engineer shall furnish Superior Township with mylar "Record" water main plans along with a computer disk using the most recent release of AutoCAD, upon job completion. Plans shall locate all water mains, hydrants and gate valves and wells.
- Where work is to be performed in the vicinity of a Superior Township main, contractor shall notify the Superior Township Utility Department at (734) 480-5500 at least 3 working days prior to start of construction as well as township engineer to schedule inspection.
- All pipe and all pipe fittings shall be made in the U.S.A.
- All bolts on all flanged and mechanical joint fittings shall be domestic origin high strength, low alloy COR-BLUE steel bolts or approved equal. These bolts shall meet the current provisions of American National Standard ANSI/AWWA C111/A21.11-90 for rubber gasket joints for ductile iron pressure pipes and fittings. Bolt manufacturer's certificate of compliance must accompany each shipment.
- BACKFILL NOTE:** Under road surfaces, pavement, sidewalk, curb, driveways and where the edge of the trench is within 3 (three) feet of the pavement or as called for on the plans, the trench depth shall be 4 (four) inches lower than the proposed water main elevation. The trench width shall be the outside diameter plus 16 (sixteen) inches for pipe diameters up to 36 (thirty-six) inches and larger. The trench shall be backfilled by placing granular material by the "controlled Density Method" or other means having approval of the engineer and compacting it to 95 (ninety-five) percent of its maximum unit weight.
- "Mega Lugs," "Field Lock Gaskets," or a mechanical joint restraint system shall be used instead of concrete thrust blocks for joint restraint.
- All bolts used in securing fittings to the water main shall be "COR-BLUE" bolts or approved equal.
- All buried bolts, nuts and washers shall be "COR-BLUE" or equivalent and poly-wrapped.
- Like size to like size tapping sleeves shall not be used with transit pipe.
- Where water main is located under pavement, the Township shall not be responsible for repairing pavement within the easement in the event that maintenance or repairs to the water main become necessary.
- Mechanical and slip-on joints for ductile iron shall be in accordance with AWWA C111 (ANSI A21.11)
- All fire hydrant joints shall be totally restrained by use of "Mega Lugs" or other approved restrained joint.
- Fittings for ductile iron pipe shall be ductile iron or cast iron and shall meet requirements of AWWA C10 (ANSI A21.10) or AWWA C153 (ANSI A21.53). Ductile iron fittings shall be rated for 350 psi, pipe 24" diameter and less and 250 psi for pipe sizes over 24" diameter, except that ductile iron flanged fitting shall be rated for 250 psi for all pipe diameters.
- Manhole steps shall be steel encased with polypropylene plastic or approved equivalent to M.A. Industries, Inc., PS-1 for brick, or PS-18 for block, East Jordan Iron Works 8502 (or approved equal). Manhole steps at 16" centers.
- Maximum joint deflection and water main radius shall not exceed manufacturer recommendations (e.g. 4"-36" WM - 5" per 20' or 206' radius).



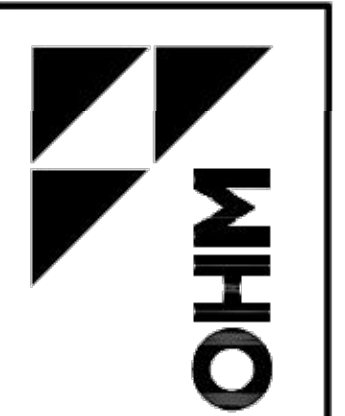
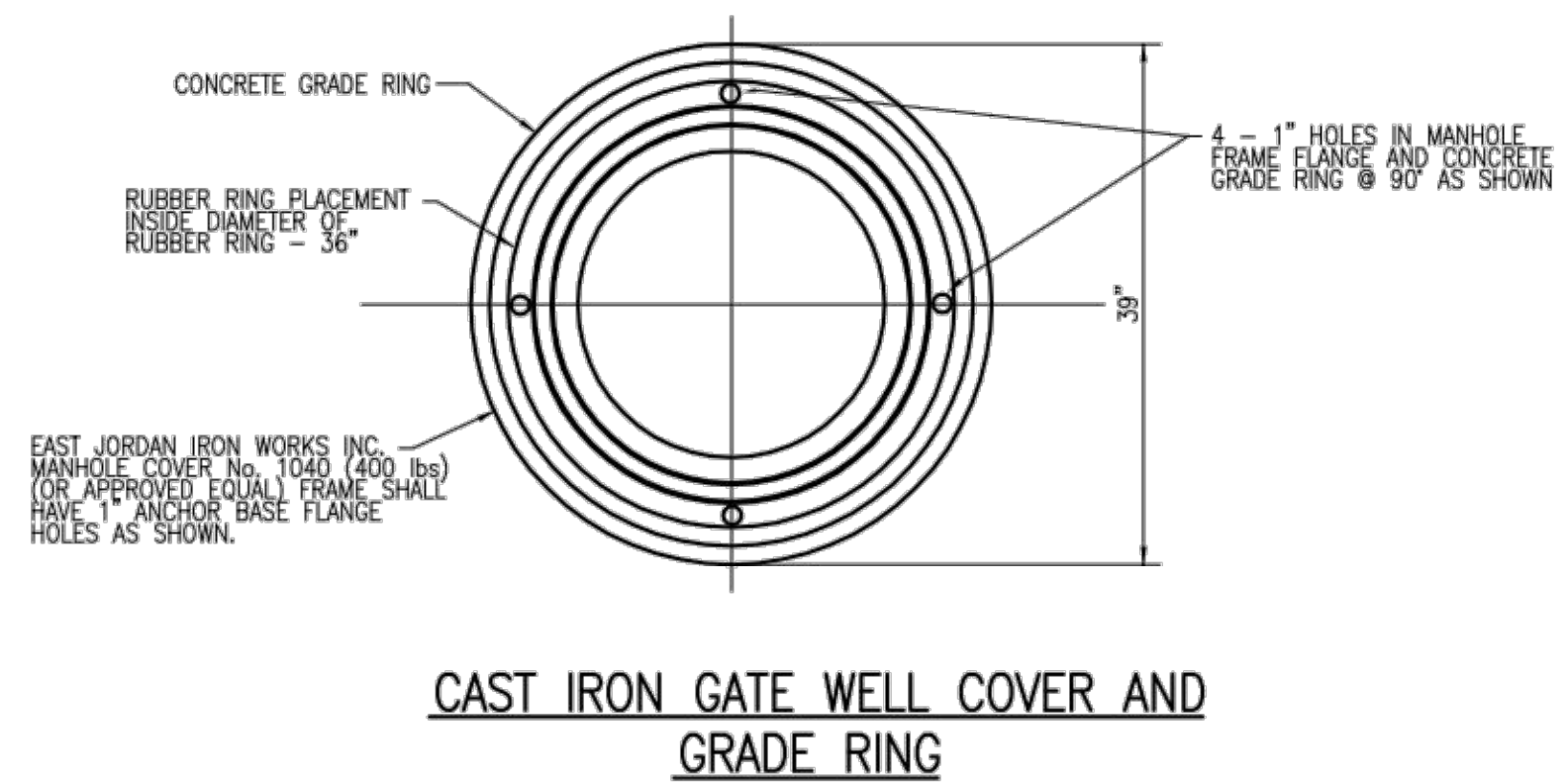
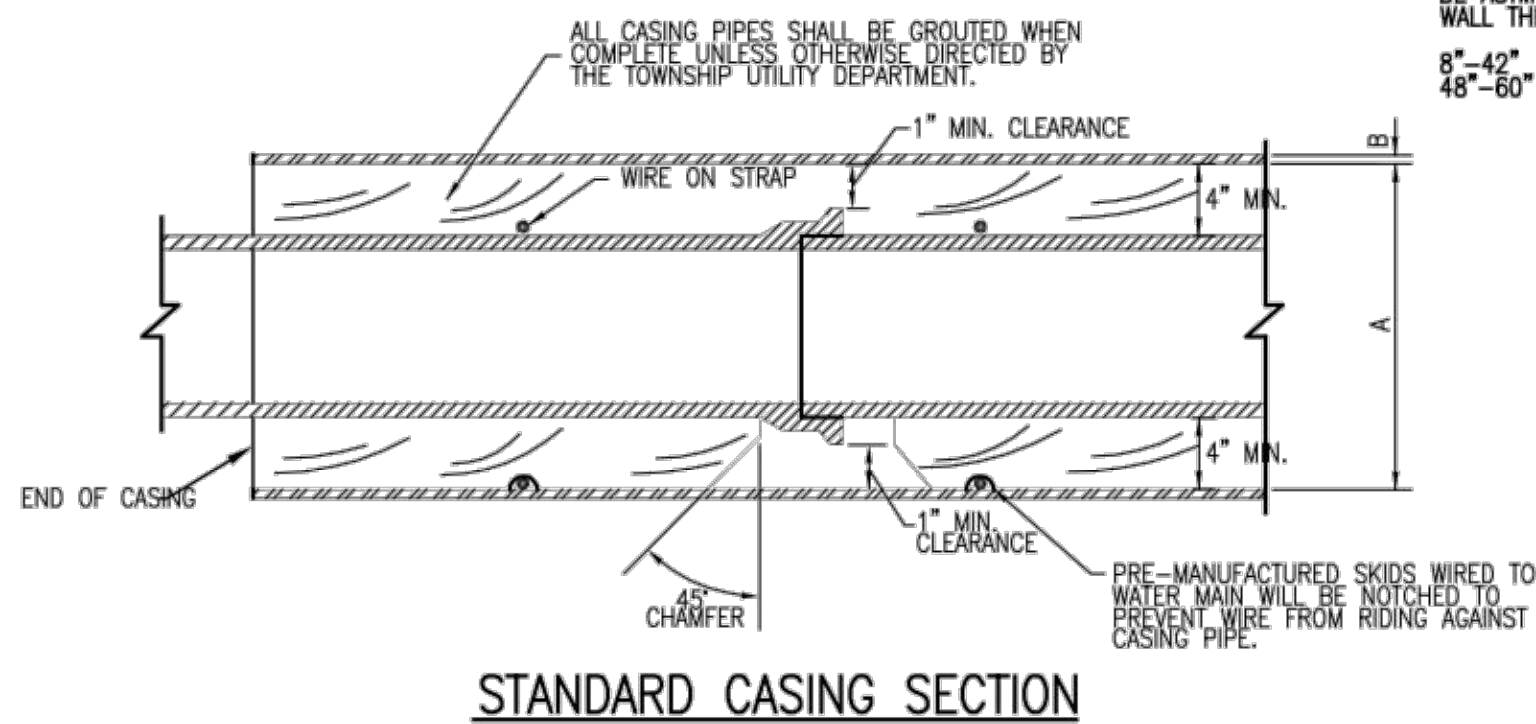
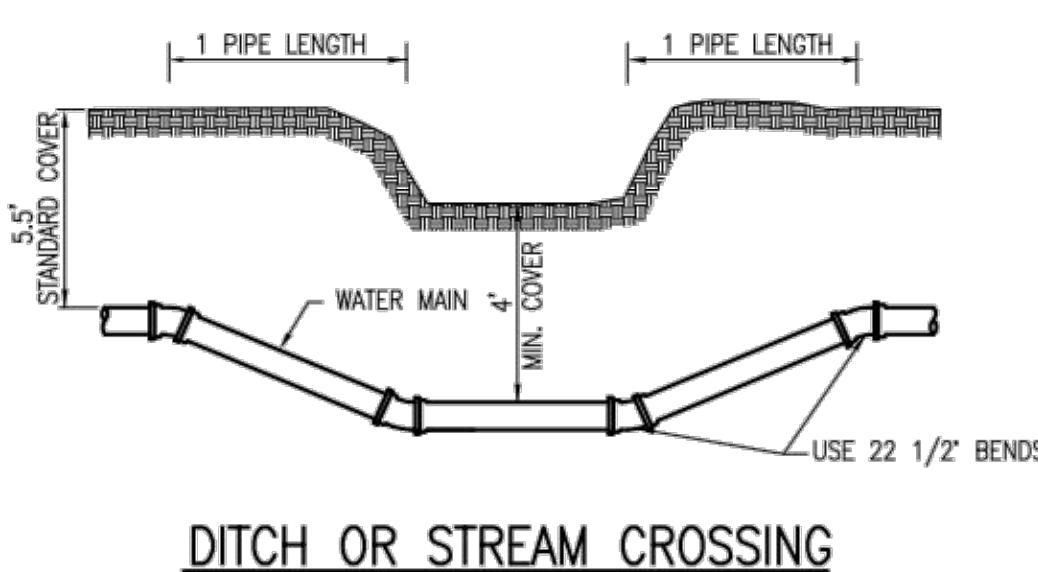
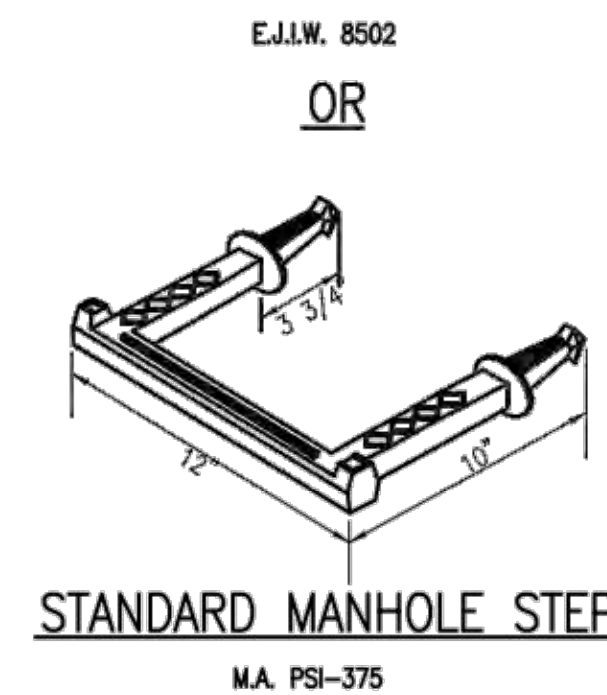
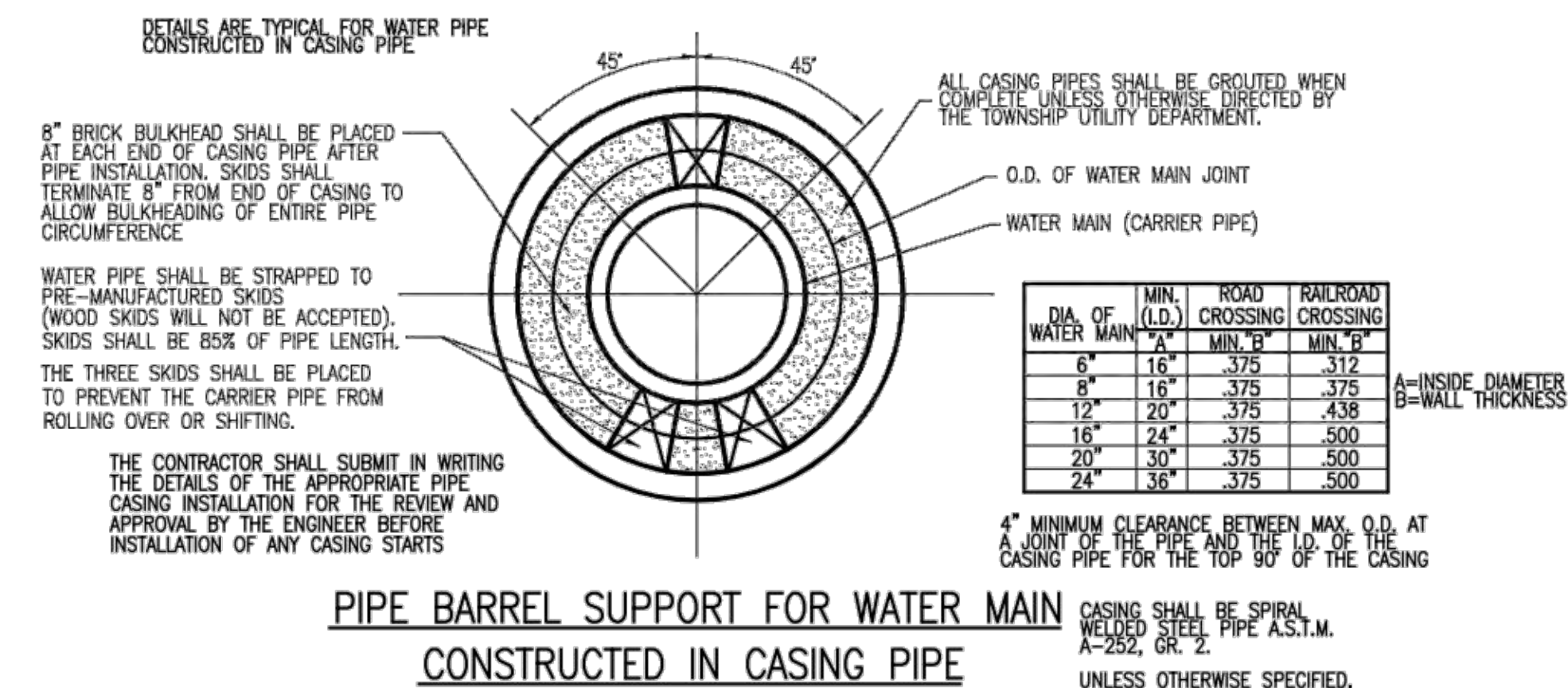
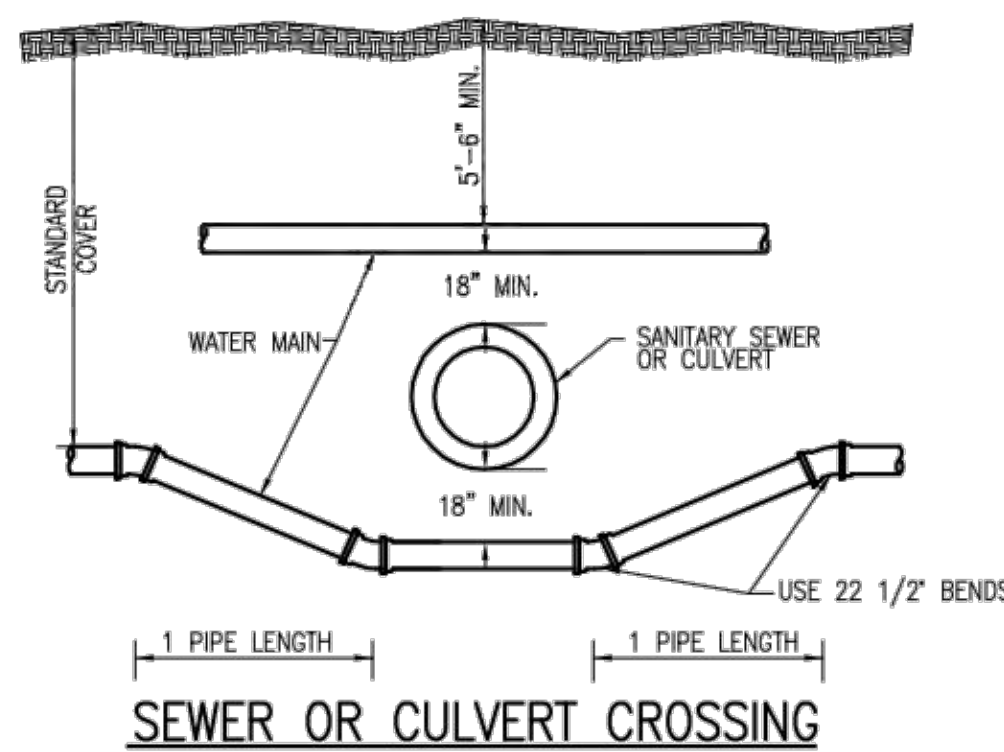
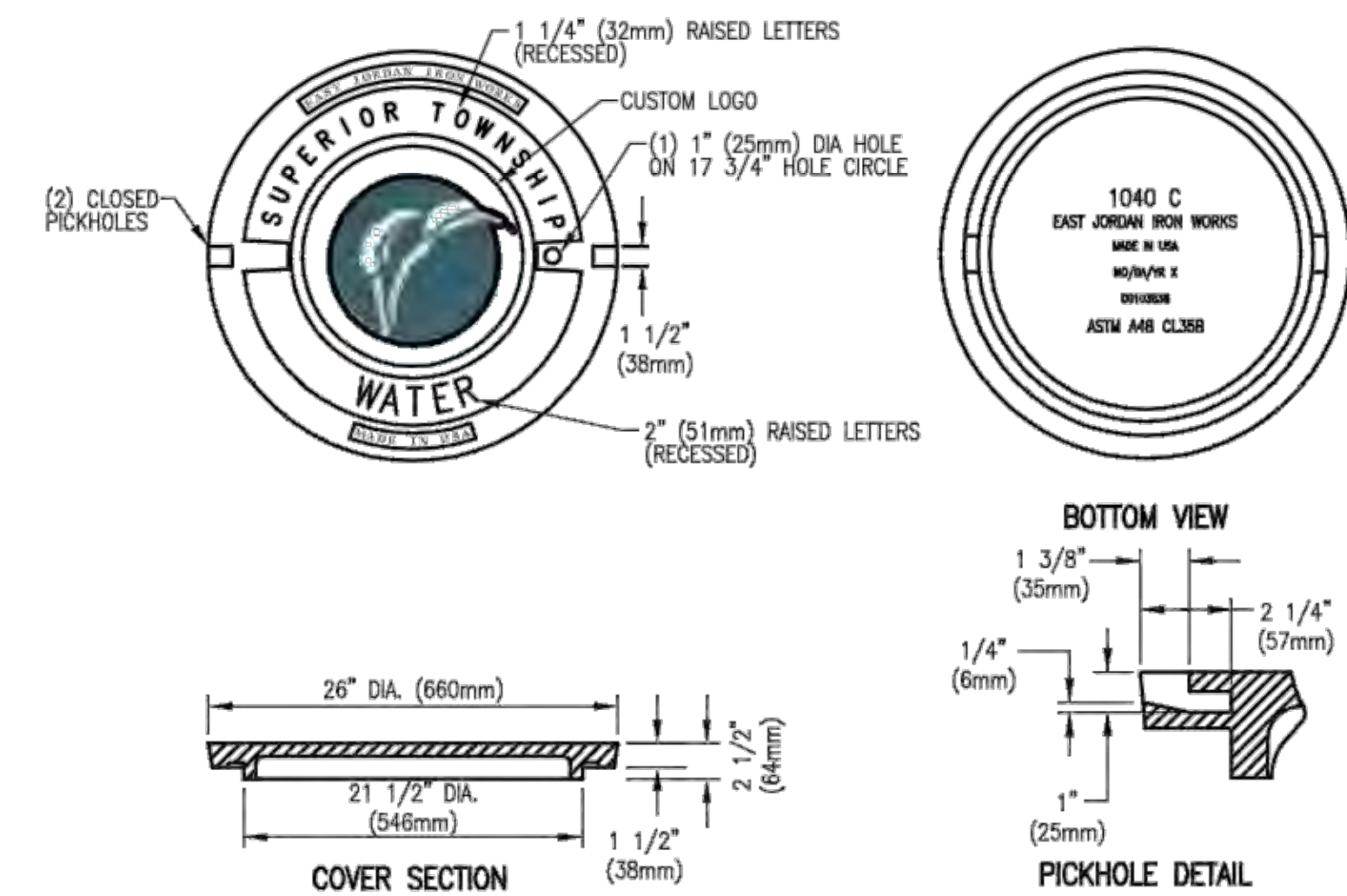
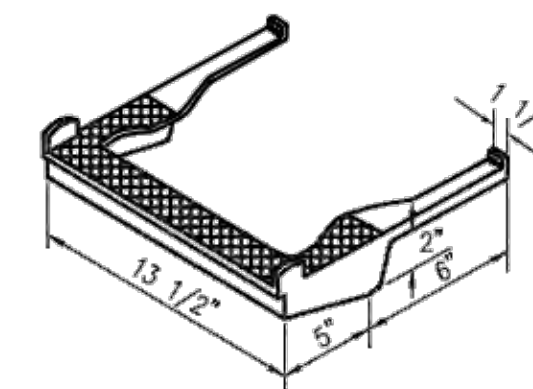
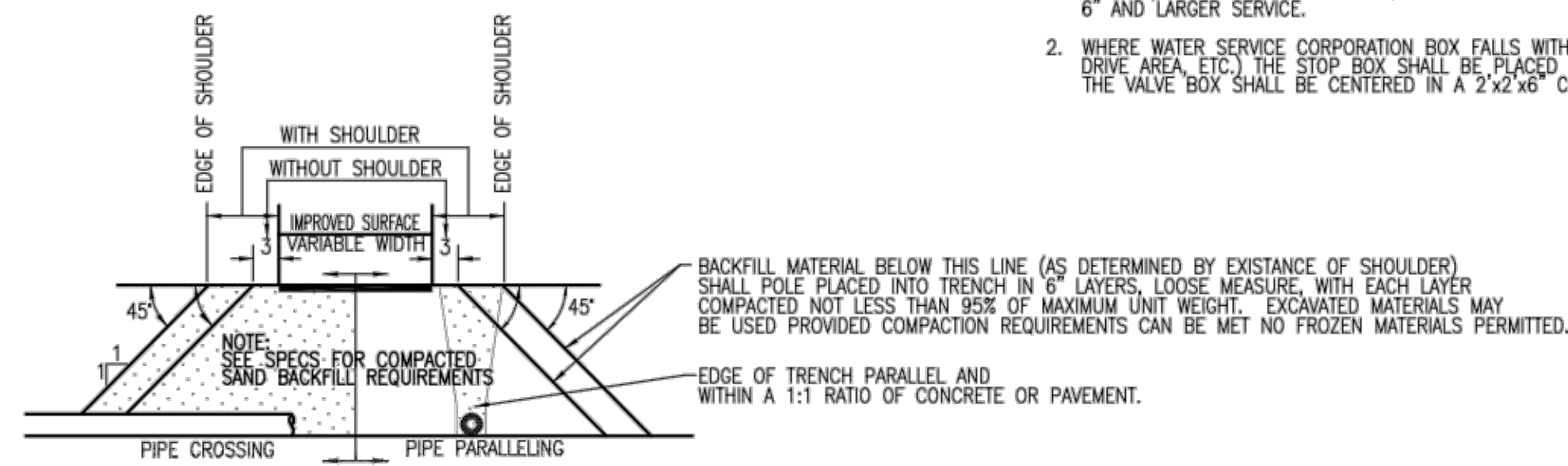
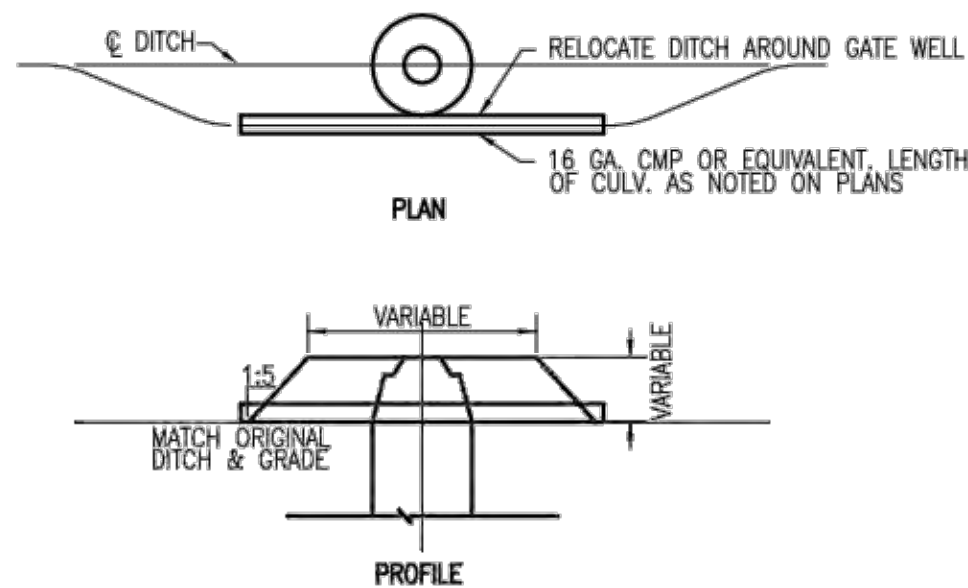
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NOTE: ALL GATE WELL PRECAST SECTIONS SHALL BE PREMIUM JOINT.
EAST JORDAN IRON WORKS, INC. MANHOLE COVER No. 1040C (OR APPROVED EQUAL) W/BOLTED FRAME
SEE DETAIL FOR GATE WELL TOPS WITHIN PAVEMENT AREAS
FOUR (4) CADMIUM COATED 5/8" DIA. THREADED STUDS W/ 3/4"x2"x1/8" THICK METAL WASHERS.
1" DIA. RUBBER O-RING GASKETS MIN. DUROMETER 20, MAX. 40
4" TO 8" GRADE RING W/FINISH TOP AND BOTTOM SURFACES, MAX. ADJUSTMENT = 15", MIN. = 4"
GROOVED TOP
60" I.D. TO 48" I.D. ECCENTRIC REDUCER TRANSITION SECTION
WHERE REQUIRED 60" DIA. GATE WELL RISER SECTIONS 2'-0", 2'-6", 3'-0", 4'-0"
CORPORATION STOP MIN. 1"
60" DIA. GATE WELL BASE SECTION 2'-0", 2'-6", 3'-0", 4'-0"
BRICK OR CONC.
4" MIN.
FULL SEAL RUBBER BOOT, KOR-N-SEAL OR ENGINEER APPROVED EQUAL
COMPACTED MDOT 6AA STONE OR MDOT GRANULAR CLASS II OR III SAND
5'-0" TO JOINT

NOTE: TAPPING SLEEVE VALVE AND WELL FOR WATER MAIN LARGER THAN 16" DIAMETER SHALL BE PER Y.C.U.A. AND D.W.S.D. STANDARDS, AND MUST BE SHOWN ON THE PLANS.
*USE OF 8"x8" TAPPING SLEEVE VALVE AND WELL MUST BE APPROVED BY SUPERIOR TOWNSHIP
OPENING IN PRECAST BASE TO BE FILLED WITH BRICK.
NOTE: USE OF 12"x12" TAPPING SLEEVE VALVE AND WELL MUST BE APPROVED BY SUPERIOR TOWNSHIP D.P.W.
NOTE: ALL HYDRANTS TO BE FULLY RESTRAINED BY MECHANICAL JOINTS APPROVED BY TOWNSHIP ENGINEER.



- WATER STOP BOX REQUIREMENTS**
- TWO HOLE ARCH PATTERN (ERIE PATTERN) TYPE WITH 1" UPPER AND 5' EXTENDED LENGTH WITH 42" ROD (FORD STOP BOX PART NO. EAT-50-40-42R OR APPROVED EQUAL); SMALL BOX REQUIRED FOR 3/4" AND 1" SERVICE; LARGE BOX REQUIRED FOR 1-1/2" AND 2" SERVICE; HYDRANT VALVE BOX REQUIRED FOR 3" AND 4" SERVICE; GATE WELL REQUIRED FOR 6" AND LARGER SERVICE.
 - WHERE WATER SERVICE CORPORATION BOX FALLS WITHIN A PAVED AREA (PARKING LOT, SERVICE DRIVE AREA, ETC.) THE STOP BOX SHALL BE PLACED IN A STANDARD HYDRANT VALVE BOX. THE VALVE BOX SHALL BE CENTERED IN A 2'x2'x6" CONCRETE SLAB.



DATE	DESIGNER	DRAWN	CHECKED	SECTION	TOWN	RANGE	COUNTY	CITY/TOWNSHIP	SCALE	H. NTS	V. GRA	VERT. DATA
OCT 2003	DK						WASHINGTON	CHARTER TOWNSHIP OF SUPERIOR				NA
REVISIONS												
000-00-0000												
WWW.OHM-ADVISORS.COM												
34000 Plymouth Road Livonia, MI 48150 P (734) 522-6711 F (734) 522-6427												
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CHARTER TOWNSHIP OF SUPERIOR
STANDARD WATER MAIN DETAILS

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